

SECTION 00 31 32 - GEOTECHNICAL DATA

PART 1 - GENERAL

1.01 INFORMATION AVAILABLE TO BIDDERS – GEOTECHNICAL DATA

- A. This Document and its referenced specifications and drawings are included in the Procurement and Contracting Requirements for Project as they provide Owner's information for Bidders' convenience and are intended to supplement rather than serve in lieu of Bidders' own investigations. They are made available for Bidders' convenience and information and are not a warranty of existing conditions. All Bidders will receive the following list of reference documents:
1. Geotechnical Engineering Reports, including all Appendices, for the Pottsgrove HS Facility & Athletics Improvement Project (the "Project") prepared by David Blackmore Associates dated May 15, 2026, is included as Specification Section 00 31 33.
 2. Infiltration Testing Plan and Logs, including all Appendices, for the Pottsgrove HS Facility & Athletics Improvement Project (the "Project") prepared by K&W Engineers May 26, 2026, is included as Specification Section 00 31 34.
- B. Geotechnical Data provided as part of this Section and its referenced documents shall not constitute a Contract Document and shall be considered as "general information" only. The data indicated for subsurface conditions is not a representation or guarantee of the continuity of such conditions including variations in kind, depth, quantity, and conditions of soil. The Owner, Construction Manager, and Architect will not be responsible for interpretations or conclusions drawn from the Geotechnical Report(s) by the Bidder. The Owner, Construction Manager, and Architect are not responsible for the accuracy or completeness of the information given in the Geotechnical Report(s).
- C. Bidders may rely on the technical data contained in such reports and information presented herein, but not upon non-technical data, interpretations, recommendations, or opinions contained therein or for the completeness thereof for Bidder's purposes. Bidders shall determine the adequacy of the included geotechnical data for use in preparing the Bid. Should the data contained in the above referenced document(s) not be adequate for the Bidder's purposes, the Bidder may make, prior to bidding, his own exploration, tests, and analyses.
- D. Bidders should visit the site to familiarize themselves with existing conditions. No claims will be honored for inadequacy of data contained within the provided geotechnical reports. Should Bidder determine that additional information is required, the Bidder shall make, prior to bidding, his own exploration, tests, and analyses.
- E. Contact CMTA, Inc., Construction Manager (CM), Scott Kennedy at (267) 718-0148 to schedule additional exploration or tests. Bidders shall provide insurance certificates as per General Conditions Article 11 and other information required prior to performing additional exploration or tests.

PART 2 - PRODUCTS (Not Used)

PART 3 - EXECUTION (Not Used)

END OF SECTION 00 31 32

*Providing Innovative Solutions to
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REPORT OF
GEOTECHNICAL EXPLORATION

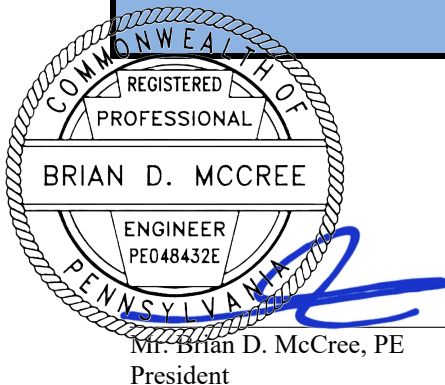
Pottsgrove High School Stadium Renovation Project
1301 Kauffman Road
Pottstown Borough, Montgomery County, PA

PREPARED FOR

Pottsgrove School District
c/o CMTA, Inc.
1400 N. Providence Road,
Building #2, Suite 4025
Media, PA 19063

PROJECT 5813G1R1
May 14, 2026

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EXECUTIVE SUMMARY

Purpose

This exploration was conducted to evaluate subsurface conditions and their effects on the proposed site development. This exploration focused on the proposed stadium improvements including new bleachers, storage buildings, restrooms, and concession areas within the existing stadium located at the Pottsgrove High School Campus at 1301 Kauffman Road, in Pottsgrove, PA

Scope

A total of ten (10) borings were drilled at the project site to a maximum depth of 21 feet or to auger refusal on bedrock. These borings were located within accessible portions of the planned building footprint areas as indicated on the attached preliminary site plan. The test borings were drilled by our subcontractor, The Corcoran Drilling Company, under the direction of DBA personnel.

Findings

The results of our exploration indicate the presence of the following subsurface conditions:

- Existing fill soils
- Shallow depth to bedrock
- Shallow depth to groundwater

The drilling program encountered a 6 inch to 4-foot-thick deposit of existing fill (Stratum IF) in three of the test borings completed (B2, B6, B9). This fill consists of orange-brown and reddish-brown fine sand and clayey silt with shale fragments, and organic silt. This stratum is variable in strength and content, with SPR values ranging from 4 blows per foot (B/F) to 10 B/F. The native soil profile includes a layer of reddish-brown fine sand and silt with shale fragments (Stratum II). This layer then transitions to red weathered shale (Stratum III) at depths ranging from 2.67 feet to 8 feet below the existing grade.

Shale bedrock was encountered in nine of the borings completed. Depths to bedrock range from 6.58 feet to 16.33 feet below the existing grade at elevations from 198.41 feet to 209.64 feet. Dense weathered shale, as shown by augering difficulty, was also encountered in nine of the test borings completed at depths ranging from 4 to 14.74 feet below the existing grade at elevations of 199.39 feet to 213.84 feet. See Table IIA- Approximate Rock Elevation for information at each boring location.

Groundwater was encountered in four of the test borings completed at depths ranging from 9.75 feet to 18.50 feet below the existing grade at elevations from 196.92 feet to 204.99 feet. See Table IIB – Approximate Groundwater Elevations for information at each boring location.

Recommendations:

The site conditions are suitable for the support of the proposed structure. A shallow foundation system with slab-on-grade construction is feasible, provided that the recommendations included herein are followed. DBA recommends a removal and replacement of Stratum IF as noted in section 7.1A – Soil Replacement.

1. INTRODUCTION

David Blackmore and Associates, Inc. (DBA) has completed the geotechnical exploration of the subject site in accordance with our Proposal 5813G1P1, dated February 18, 2026. This exploration was completed to evaluate the existing subsurface conditions and their effect upon the proposed site development. Specifically, DBA has provided recommendations regarding the following:

- Foundation support of the structure and slabs, including soil bearing pressures, bearing elevations, foundation design recommendations, and anticipated settlement for shallow foundations,
- Alternate foundation systems(if required),
- Depth to material requiring rock excavation methods for removal, if encountered,
- Depth to and management of groundwater for design of structures and pavements, if encountered
- Relative elevations of surface and subsurface features,
- Fill and compaction criteria,
- Pavements and floor slabs,
- Permeability of subgrade soils at proposed seepage basin areas,
- Lateral earth pressures for retaining walls, and
- General geotechnical related construction procedures.

The following section (2. PROPOSED CONSTRUCTION) summarizes the information available to DBA regarding the proposed site development. This report has been prepared based on the proposed construction. Changes to the proposed construction may require alterations to this report or additional investigative work. DBA should be notified of significant changes to the proposed construction.

2. PROPOSED CONSTRUCTION

The proposed construction includes new bleachers, support buildings, a synthetic turf field, and a running track. The new support buildings are to include a buildings and grounds maintenance and storage building; concession stand and associated patio area with restroom facilities on the home side of the stadium and a restroom and concession building on the visitor's side of the stadium. These buildings are anticipated to be single story structures using a shallow concrete foundation system with structural steel framing, slab-on-grade construction, and masonry interior and exterior walls. The information in this report is preliminary and subject to revision during plan development.

3. GEOTECHNICAL EXPLORATION

A total of ten (10) borings were drilled at the project site to a maximum depth of 21 feet or to auger refusal on bedrock. These borings were located within accessible portions of the planned building footprint areas as indicated on the attached preliminary site plan.

The test borings were drilled by our subcontractor, **The Corcoran Drilling Company**, under the direction of DBA personnel.

All test boring logs and a test boring location plan are included in the appendix of this report.

4. GEOTECHNICAL BACKGROUND

4.1 SITE DESCRIPTION

The subject site is the existing football stadium at the Pottsgrove High School Campus located at 1301 Kauffman Road. The site is currently used as the school's athletic fields, with existing fields, bleachers, a facilities building, and a garage. The site slopes downward in the westerly direction from an elevation of

approximately 220 feet along school lane to approximately 210 feet on the western side of the existing home bleachers.

A copy of the USGS Topographical Map, Sassamansville Quadrangle, indicating the site is included as Figure I.

4.2 GEOLOGY

Available geological sources indicate the site is underlain by the Brunswick Formation (TrB). This formation consists of reddish-brown shale, mudstone, and siltstone with argillite and shale interbedding. The formation is moderately well bedded; thin to flaggy, joints are moderately developed and abundant; closely spaced; steeply dipping; mostly open and have a blocky pattern, moderately resistant to weathering to a moderate depth; overlying mantle is thick, good surface drainage, weathered zone can be excavated with moderate ease; weathered rock is difficult; moderate to fast drilling rate.

A copy of the USGS Geological Map of the Sassamansville Quadrangle, indicating the site is included as Figure II.

4.3 SOILS

Soil records indicate the site soils to be of the following series:

UrxB – Urban land – Penn complex, 0 to 8 percent slopes: This series consists of residuum weathered shale and siltstone with artificially covered areas such as pavements and buildings. A typical soil profile consists of the following:

- 0 to 6 inches: variable
- 0 to 8 inches: channery silt loam
- 8 to 21 inches: channery silt loam
- 21 to 34 inches: very channery silt loam
- 34 to 44 inches: bedrock

A copy of the USDA Soil Survey Website indicates the site is included as Figure III.

5. LABORATORY TESTING

Representative soil samples taken during the field exploration were tested in DBA's laboratory for basic engineering properties. The laboratory testing consisted of classification of soil samples for engineering purposes. The laboratory testing included Particle Size Analysis (ASTM D442), Plastic and Liquid Limits (ASTM D4318), and Natural Moisture Content (ASTM D2216). The Unified Soil Classification System (USCS) was used to assign group symbols and group names to the soils tested.

A summary of the test results is provided in Table I. A photocopy of the particle size analysis results and the plastic and liquid limit analysis results are included in the appendix of this report.

6. SUBSURFACE CONDITIONS

The results of the drilling program revealed a fairly consistent subsurface profile. Beneath 6 to 16 inches of topsoil or 3 inches of asphalt and 5 to 9 inches of subbase stone, the following strata can describe a typical soil profile.

Stratum IF: 0.5' to 4.0' thick; Fill consisting of orange-brown and reddish-brown fine sand and clayey silt with shale fragments. Portions of this fill include gray to brown organic silt (See test boring B6). This stratum is loose to medium dense with SPR¹ values ranging from 4 blow per foot (B/F) to 10 B/F. This stratum was encountered in three (3) of the test borings completed.

Stratum II: 1.75' to 7.0' thick: Reddish-brown and orange-brown fine sand and silt with some clay and shale fragments. This stratum is medium dense to dense with SPR values ranging from 15 B/F to over 100 B/F. The average SPR value is 49 B/F. This stratum was encountered in nine (9) of the test borings completed.

Stratum III: 4.17' to 13.0' thick: Weathered red shale. This stratum is dense to very dense with SPR values ranging from 28 B/F to over 100 B/F. The average SPR value for this stratum is 50 blows per 3 inches.

Bedrock: Shale bedrock was encountered in test borings completed at depths ranging from 6.58 feet to 16.33 feet below the existing grade, corresponding to rock surface elevations ranging from 198.41 feet to 209.64 feet. Dense weathered shale, as indicated by augering difficulty, was also encountered in nine of the test borings completed at depths ranging from 4.0 to 14.74 feet below the existing grade, corresponding to dense weathered rock elevations ranging from 199.39 feet to 213.84 feet. See Table IIA- Approximate Rock Elevation for information at each boring location.

Groundwater²: Groundwater was encountered in four of the test borings completed at depths ranging from 9.75 feet to 18.50 feet below the existing grade at elevations from 196.92 feet to 204.99 feet. See Table IIB – Approximate Groundwater Elevations for information at each boring location.

NOTES:

1. SPR = the Standard Penetration Resistance or number of blows required of a 140-pound hammer dropping 30", to drive a 2" OD split spoon sampler one foot.

2. The groundwater information provided is based on conditions encountered during the drilling program. Seasonal fluctuations in the groundwater table are to be expected.

7. GEOTECHNICAL ANALYSIS AND RECOMMENDATIONS

The results of our exploration indicate the presence of the following subsurface conditions:

- Existing fill soils
- Shallow depth to bedrock
- Shallow depth to groundwater

The drilling program encountered a 6 inch to 4-foot-thick deposit of existing fill (Stratum IF). This fill was intermittently encountered in three of the test borings completed (B2, B6, B9). It consists of a mixture of orange-brown and reddish-brown fine sand and clayey silt with shale fragments, and organic silt. This stratum is variable in strength and content, with SPR values ranging from 4 blows per foot (B/F) to 10 B/F. The native soil profile includes a layer of reddish-brown fine sand and silt with shale fragments (Stratum II). This layer then transitions to red weathered shale (Stratum III) at depths ranging from 2.67 feet to 8 feet below the existing grade.

Shale bedrock was encountered in nine of the borings completed. Depths to bedrock range from 6.58 feet to 16.33 feet below the existing grade at elevations from 198.41 feet to 209.64 feet. Dense weathered shale, as shown by augering difficulty, was also encountered in nine of the test borings completed at depths ranging from 4 to 14.74 feet below the existing grade at elevations of 199.39 feet to 213.84 feet. See Table IIA- Approximate Rock Elevation for information at each boring location.

Groundwater was encountered in four of the test borings completed at depths ranging from 9.75 feet to 18.50 feet below the existing grade at elevations from 196.92 feet to 204.99 feet. See Table IIB – Approximate Groundwater Elevations for information at each boring location.

The site conditions are suitable for the support of the proposed structure. A shallow foundation system with slab-on-grade construction is feasible, provided that the recommendations included herein are followed. DBA recommends a removal and replacement of stratum IF as noted in section 7.1A – Soil Replacement.

7.1 SITE PREPARATION

All deleterious materials including topsoil, root mass, trees and vegetation, asphalt and other materials determined in the field by the Geotechnical Engineer to be unsuitable shall be removed from all structural areas (buildings, pavements, and walkways) prior to placement of *structural fill*. Recycling of the asphalt and underlying stone can be accomplished on site if the asphalt is milled to a maximum 1-inch particle size and the material is used in the upper fill zones of pavement areas only. This fill is not suitable for other structural areas.

7.1A SOIL REPLACEMENT

Based on the conditions encountered during the completed test borings, DBA recommends the complete removal and replacement of the intermittent deposits of existing fill (Stratum IF) from within portions of two different building footprint areas. In addition, this removal shall extend beyond the building footprint for a minimum lateral distance equal to the removal depth to ensure adequate lateral stability. Once this material is removed, the exposed subgrade is to be proof rolled using a heavy (minimum 15-ton) smooth drum roller. This proof-roll shall be completed under the supervision of the Geotechnical Engineer. Any loose/soft zones identified shall be repaired to the satisfaction of the Geotechnical Engineer. Subgrade repair would typically include undercutting and/or the use of stabilization fabric or open-graded ballast. It is anticipated that this soil replacement procedure would include the southeast section of the existing press box/maintenance facilities building (Boring B2) at a maximum

depth of approximately 4 feet below the existing grade, and the northwest corner of the proposed visitor side concession stand and restroom building (See boring B9) at a depth of 3 feet. A fill deposit was found at the west end of the planned concession patio area (See boring B6), extending to a depth of 1.75 feet below the current grade. It is expected that this fill deposit may be removed during site grading, with a proof roll recommended to confirm that no soft spots remain.

7.2 FOUNDATIONS

Foundations shall bear on the undisturbed soils of Stratum II, Stratum III, or structural fill. Soft conditions encountered during foundation construction shall be excavated and replaced with structural fill. Refer to Section 7.7, Fill and Compaction Criteria.

Foundations shall be designed for a maximum soil bearing capacity of 4.0 KSF on Stratum II, Stratum III, or structural fill. If the column loads are to be revised during a re-design of the structure, the soil bearing capacity may need to be modified. DBA shall be notified of any significant changes in this regard.

Foundation settlements for the bearing capacities provided herein are to be within a tolerance of one inch total and a half-inch differential. It is anticipated that the bulk of this settlement will take place during the construction period. Settlements of this magnitude are within normal construction tolerances. In the event that more stringent settlement tolerances are required, a reduction of the allowable bearing capacity and/or a change in depth to bearing strata may be required.

To protect against differential settlements, foundations shall not be placed intermittently on soils and boulders. Small boulders encountered within or directly beneath the footing bearing surface shall be removed and replaced with *structural fill*. Larger boulders shall either be fully or partially removed and

replaced with *structural fill*. Trench excavation through areas containing boulders may require substantial over-excavation to facilitate boulder removal. Partial removal can be completed by splitting or hammering to a minimum of 12" below the footing bottom elevation. Boulder protrusion into the bottom or side of the proposed footing is not an acceptable condition. Boulders can be dislodged in bulk excavations.

Should foundation excavation encounter hard bedrock, the foundation is to be undercut a minimum of 12 inches. The undercut area is to be backfilled with a *structural fill* or a low strength flowable fill (max 500 psi) "cushioning" layer to limit the potential for differential foundation settlement and the development of stress concentrations caused by rock protrusion.

Exterior foundations or foundations in unheated areas shall be provided with a minimum of 36" compacted soil cover above the footing bottom for frost protection.

The variable soil conditions noted make it imperative that full-time Quality Control services are provided for all Geotechnical phases of this project.

7.2.1 SITE SEISMIC CONDITIONS

The 2021 edition of the International Building Code (IBC) specifies seismic design requirements applicable to the structural design of the proposed building. In particular, Chapter 16, Section 1613.2.2 is relevant to this structural design. This, in turn, requires that the project site be classified geotechnically as "Site Class" A through F, based on the Measured or Estimated Shear Wave Velocity Profile Table 20.3-1 Site Classification in Chapter 20 of ASCE 7.

**ASCE Table 20.2-1
SITE CLASSIFICATION**

SITE CLASS	V_s
A - Hard rock	>5,000 ft/s
B - Rock	>3,000 to 5,000 ft/s
BC Soft Rock	>2,100 to 3,000 ft/s
C - Very dense soil and soft rock	>1,450 to 2,100 ft/s
CD Dense Sand or Very Stiff Clay	>1,000 to 1,450 ft/s
D – Medium Dense Sand or Stiff Clay	>700 to 1,000 ft/s
DE Loose Sand or Medium Stiff Clay	>500 to 700 ft/s
E – Very Loose Sand or Soft Clay	<600 ft/s
F Soils requiring site response analysis in accordance with Section 21.1	See ASCE 7 Section 20.2.1

V_s = Shear Velocity, For SI: 1 ft/s = 0.3048 m/s

Comparing the criteria from Table 20.3-1 with the field data collected during the site exploration for this site on 3/27/2026, the subsurface profile has an estimated shear velocity within 5000 ft/s, and therefore, the site may be classified as Site Class “A” (Hard rock), for purposes of implementing the IBC 2021 seismic structural design requirements.

7.3 ROCK REMOVAL

Hard shale bedrock was encountered in nine of the borings completed from 6.58 feet to 16.33 feet below the existing grade at elevations from 198.41 feet to 209.64 feet. Dense weathered shale, as shown by augering difficulty, was also encountered in nine of the test borings completed at depths ranging from 4 to 14.74 feet below the existing grade at elevations of 199.39 feet to 213.84 feet. See Table IIA- Approximate Rock Elevation for information at each boring location. The hard bedrock encountered is not rippable using hydraulic rippers mounted on a bulldozer. It is expected that removing hard bedrock will require hydraulic hammers or explosives (if permitted by local authorities). A monitoring program

to record ground vibrations will be required. If blasting is required for changes or utility trenches, a monitoring program to record ground vibrations should be developed. DBA can develop and execute a program, if required and as directed by the Client, which may include blasting methods that increase the amount of shot rock reusable as structural fill.

The Geotechnical Engineer, during the excavating process, should verify areas and quantities of hard bedrock in the field. Excavation requiring hydraulic rippers or similar is considered rock excavation, however, it is generally less expensive than excavation of hard rock which requires blasting. A clear definition of rock should be included in any bid package.

7.4 SLAB ON-GRADE

The proposed slab-on-grade section for each of the planned buildings has an assumed thickness of 8 inches (4 inches of concrete and 4 inches of subbase stone). Existing grades around the existing football field are at approximately elevation 212.0 feet with grades sloping upward on the north side of the field (home side). It is presumed that the proposed building slab on grade elevations will approximately match the existing grades. Therefore, it is anticipated that the intermittent deposits of Stratum IF identified in the drilling program will remain in the range of 0.5 to 4.00 feet thick. Portions of this fill deposit which are in excess of 18 inches in thickness will need to be removed and replaced with structural fill (see Section 7.1A SOIL REPLACEMENT). Portions of this fill which are 18 inches thick or less and free of organic content shall be densified through proof rolling as described below.

Prior to the placement of fill the exposed slab subgrade areas shall be proof rolled with a heavy smooth drum roller (minimum 15-ton static weight) to detect the presence of loose or soft zones. This proof rolling operation shall be performed

in the proposed fill areas under the supervision of the Geotechnical Engineer. Proof rolling of the subgrade shall also be performed in the cut areas when the required grades have been achieved and immediately prior to pouring the floor slab. Loose or soft zones detected during the proof rolling operation shall be repaired to the satisfaction of the Geotechnical Engineer.

Based on the soil type encountered, standard penetration testing of the existing slab subgrade, and provided that all structural fill will be placed in accordance with the fill and compaction criteria set forth in Section 7.7, an estimated modulus of subgrade reaction of 150 psi/inch may be used for the design of slab sections. Should an increased modulus of subgrade reaction be required for the proposed design it is recommended that field or laboratory testing be completed to establish specific modulus values.

All slab subgrade areas shall be evaluated by the Geotechnical Engineer prior to pouring the slab so that repair can be completed. It is recommended that the slab be poured under roof during periods of harsh weather.

A smooth drum roller shall be made available to seal the subgrade in the event of predicted precipitation.

7.5 BACKFILL OF FOUNDATION AND UTILITY TRENCHES

All foundation and utility trenches shall be backfilled with *structural fill*, under the supervision of a Geotechnical Engineer (Refer to Section 7.7, Fill and Compaction Criteria).

7.6 PAVEMENTS AND WALKWAYS

Pavement and sidewalk areas shall be prepared in a manner similar to the slab on-grade areas. A minimum of 8 inches of crushed aggregate base shall be used beneath exterior pavements due to the frost heave potential of the subgrade soils. The pavement subgrade shall be graded to drain water from beneath the

pavement system to prevent ponding and subsequent pumping of silty subgrade soils.

For pavement design a preliminary estimated California Bearing Ratio (CBR) Value of 5.0 may be used for stabilized subgrade areas consisting of on-site silty sand with gravel (Stratum II and Stratum III) or Structural fill selected and placed in accordance with Section 7.7 of this report. Should anticipated heavy-duty pavement requirements or other project conditions require final site-specific CBR values, DBA can complete field and/or laboratory CBR testing of proposed subgrade soils at the client's request.

7.7 FILL AND COMPACTION CRITERIA

Fill supporting slabs, pavements, and foundations is considered herein to be *structural fill*. *Structural fill* shall be placed on an approved, proof rolled, nonyielding, level subgrade, in lifts not exceeding 8 inches (loose thickness), unless otherwise directed by the Geotechnical Engineer. *Structural fill* shall be maintained nominally at *Optimum Moisture Content* (ASTM D-698) and uniformly compacted to the percentages of *Maximum Dry Density* (ASTM D-698) provided in Table III - Compaction Criteria.

Suitable *structural fill* shall consist of clean soils without deleterious inclusions. On-site soils identified as Stratum II and Stratum *III* are acceptable for use as structural fill if given the opportunity to dry and are maintained nominally at Optimum Moisture Content. Samples retrieved from the upper 5 feet of the subgrade indicated moisture contents ranging from 4.90 to 13.74 percent. The optimum moisture content for compaction of these soils is estimated to range between 8 and 12 percent. Therefore, some of these soils may require aeration and drying prior to reusing them as structural fill, which is best accomplished in the summer months. Specific moisture content test results and

associated depths are indicated on the test boring logs in the appendix of this report.

Borrow fill shall be clean, well-graded soils with good strength characteristics, with a maximum particle size of 3 inches and containing not more than 20% silt/clay (by weight). Samples of on-site or borrowed sources of fill shall be submitted to the Geotechnical Engineer for testing at least 1 week before use on-site. A minimum of 65 lbs. or two (2) five-gallon buckets is required for testing.

7.8 LATERAL EARTH PRESSURES - RETAINING WALLS

The retaining/loading dock walls of the structure, if proposed, should be designed for an at rest condition (K_0). The foundations and walls must be fully drained to relieve potential hydrostatic pressure. A foundation/wall drainage system is recommended. Soil backfill around the basement walls shall be well compacted and should consist of granular soils to prevent the trapping of water.

Retaining walls outside the structure which are free to rotate should be similarly designed except with an active earth pressure as opposed to K_0 condition. Soil parameters used to establish effective fluid pressures (excluding hydrostatic loads) and some additional parameters which may be used in the design of a retaining wall system are summarized in the following table:

SOIL PROPERTIES FOR DETERMINATION OF LATERAL LOADS

Parameter	Stratum II	Stratum III
Angle of Internal Friction, ϕ	32 degrees	35 degrees
Moist unit weight, γ_m	120 pcf	134 pcf
Active Earth Pressure Coefficient, K_a	0.31	0.27
Passive Earth Pressure Coefficient, K_p	3.25	3.69
At Rest Earth Pressure Coefficient, K_o	0.47	0.43
Soil/Mass concrete interface friction Angle, δ	21 degrees	23 degrees

8. QUALITY CONTROL

This report was prepared to provide design criteria for the design team. DBA assumes that Geotechnical and Construction Quality Control Services will be provided in order to implement the recommendations provided herein and to identify unanticipated or changed conditions. The Geotechnical Engineer's representative should review the consistency of the exposed soils with the conditions encountered by this exploration as described herein. Since localized loose and yielding subgrade conditions may be encountered between test locations, provisions for the undercutting and subsequent replacement of these materials should be anticipated in the construction documents. The environmental quality of the subgrade soils was not reviewed as part of this evaluation. All materials generated by grading and excavation shall be managed in accordance with regulatory requirements.

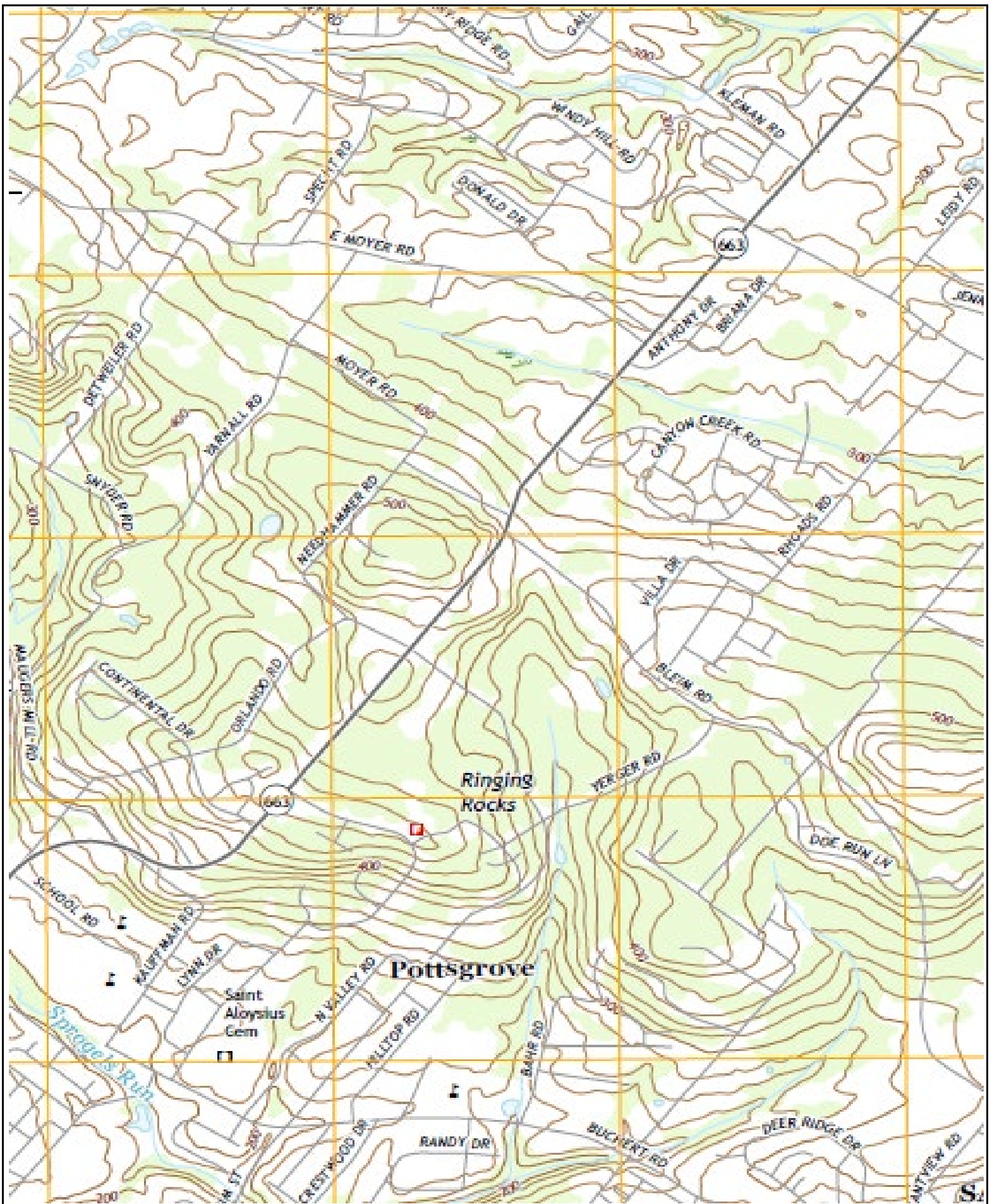
DBA can provide a contract for Geotechnical and Construction Quality Control Services (Special Inspections), as required. A pre-work meeting with the design professionals, contractors, and the Geotechnical Engineer is strongly recommended.

9. LIMITATIONS

Services performed by DBA, including the Geotechnical Exploration, report, and any subsequent construction monitoring have been or will be conducted in a manner consistent with that level of care and skill ordinarily exercised by members of the profession currently practicing in the same locality under similar conditions. No other warranty or guarantee is indicated or intended in this report or any opinion, document or otherwise stated.

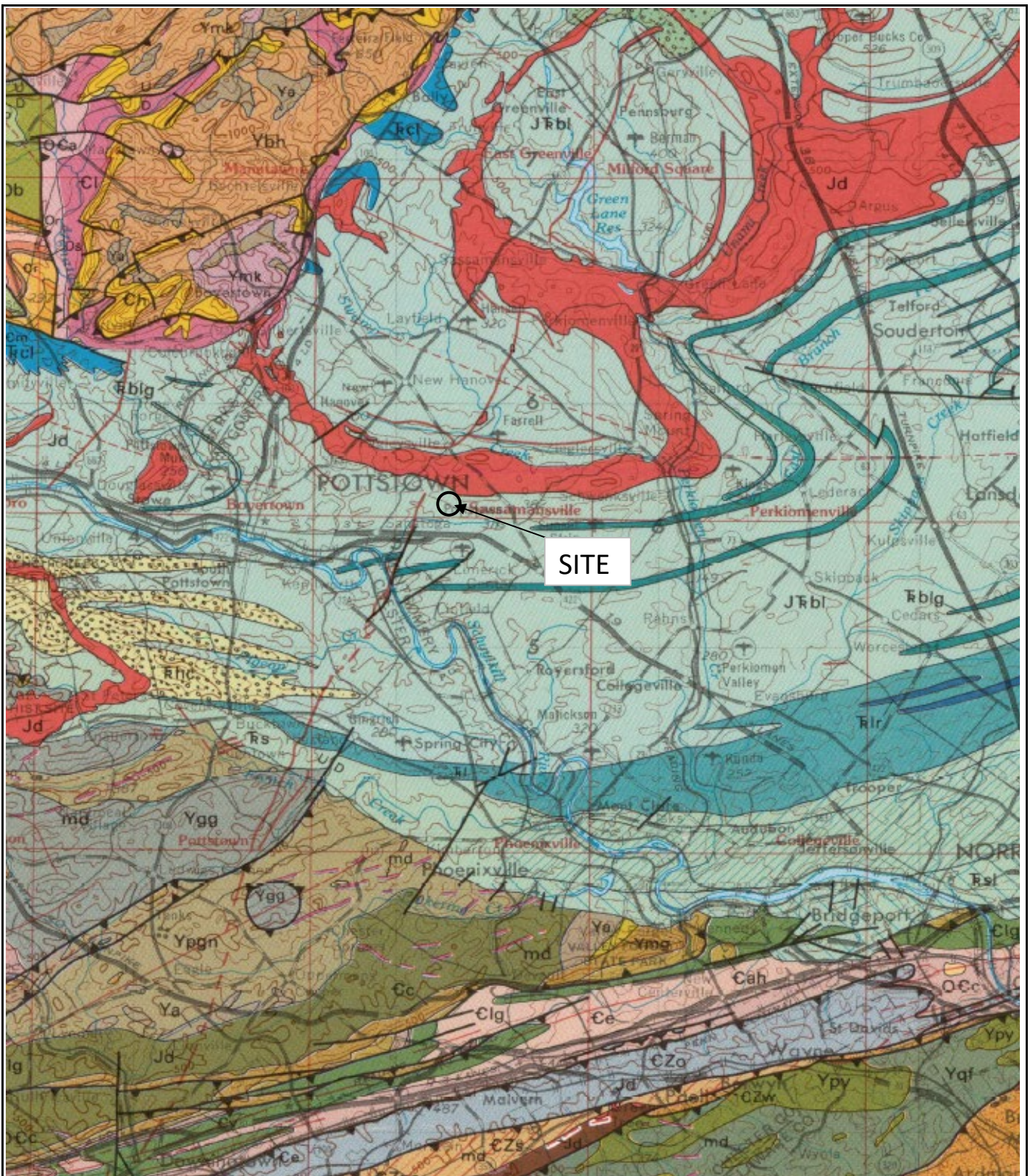
The recommendations included herein are based on the conditions encountered by the test borings performed at the subject site. It is noted that, although soil quality has been inferred from the interpolation of the site sampling data, subsurface conditions beyond the test borings are, in fact, unknown. As a result, these recommendations may require modifications based on the conditions encountered and exposed during construction excavation. Should any conditions encountered during construction differ from those described in the report, this office should be notified immediately in order to review and possibly modify the recommendations included in this report.

FIGURES AND TABLES



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Project 5813G1
Figure I
SITE LOCATION & TOPOGRAPHY
 U.S.G.S. 7.5 Minute Topographic Quadrangle
 Sassamansville Quadrangle



KEY

TrB - Brunswick Formation

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Telephone: (610) 495-6255 FAX: (610) 495-7353

Project 5813G1

**Figure II
SITE GEOLOGY**

USGS Geologic Northeast Corridor Report



KEY

UrxB - urban Land - Penn complex, 0 to 8 percent slopes

GEOTECHNICAL & ENVIRONMENTAL ENGINEERS

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Project 5813G1

Figure III

Soils

Soil Mapping via USDA Natural Resources Conservation Service

TABLE I
LABORATORY TEST RESULTS

BORING #	B1 B3	B6 B1	B4 B9
SAMPLE #	S-2 S-1	S-4 S-3	S-1 S-2
DEPTH	4' – 6' 2' – 4'	10' – 12' 8' – 10'	2' – 4' 4' – 6'
STRATUM	II	III	II
NMC* (%)	11.65 8.67	5.35 4.90	9.21 13.74

* NMC = Natural Moisture Content

SOIL PARTICLE SIZE DISTRIBUTION

SIEVE #

PERCENT PASSING BY WEIGHT

3/4"	100.0	100.0	100
3/8"	91.5	83.7	89.5
4	76.9	63.1	75.0
10	63.7	43.5	58.7
40	46.1	24.3	39.4
100	39.6	16.9	33.5
200	36.8	14.9	31.3

ATTERBERG LIMIT ANALYSIS

LL*	N/A	N/A	N/A
PL*	N/A	N/A	N/A
PI*	N/A	N/A	N/A

* LL = Liquid Limit; PL = Plastic Limit; PI = Plasticity Index

USCS CLASSIFICATION

Eng. Class.	SM	SM	SM
Descr.	Silty sand with gravel	Silty sand with gravel	Silty sand with gravel

TABLE I (CONT'D)
LABORATORY TEST RESULTS

BORING #	B2	B9
SAMPLE #	S-1	S-1
DEPTH	2' – 4'	2' – 4'
STRATUM	IF	IF
NMC* (%)	21.98	12.08
ORG** (%)	N/A-	N/A

* NMC = Natural Moisture Content

** ORG = Organic Silt Content

TABLE IIA
APPROXIMATE ROCK ELEVATIONS

Boring Number	Surface Elevation	Depth to Dense Weathered Rock ¹	Dense Weathered Rock Elevation	Depth to Bedrock ²	Bedrock Elevation
B1	211.39 feet	12.0 feet	199.39 feet	12.17 feet	199.22 feet
B2	214.74 feet	14.75 feet	199.99 feet	15.83 feet	198.91 feet
B3	213.74 feet	5.50 feet	208.24 feet	15.33 feet	198.41 feet
B4	215.49 feet	11.58 feet	203.91 feet	11.75 feet	203.74 feet
B5	214.93 feet	4.0 feet	210.93 feet	14.83 feet	200.10 feet
B6	217.18 feet	8.0 feet	209.18 feet	16.33 feet	200.85 feet
B7	218.77 feet	9.50 feet	209.27 feet	10.25 feet	208.52 feet
B8	219.31 feet	9.50 feet	209.81 feet	9.67 feet	209.64 feet
B9	215.42 feet	<21.0 feet	<194.42 feet	N/E	N/A
B10	215.51 feet	1.67 feet	213.84 feet	6.58 feet	208.93 feet

NOTES:

Surface elevations at each boring location were determined in the field using a Trimble GPS Rover.

¹ As determined by drilling difficulty and Standard Penetration Resistance data.

² As determined by auger refusal.

TABLE IIB
APPROXIMATE GROUNDWATER ELEVATIONS

Boring Number	Surface Elevation	Depth to Groundwater	Groundwater Elevation ³
B2	214.74 feet	9.75 feet	204.99 feet
B5	214.93 feet	12.42 feet	202.51 feet
B6	217.18 feet	12.67 feet	204.51 feet
B9	215.42 feet	18.50 feet	196.92 feet

³ Groundwater information is based on conditions encountered during the field exploration. Seasonal fluctuation in groundwater elevation shall be expected.

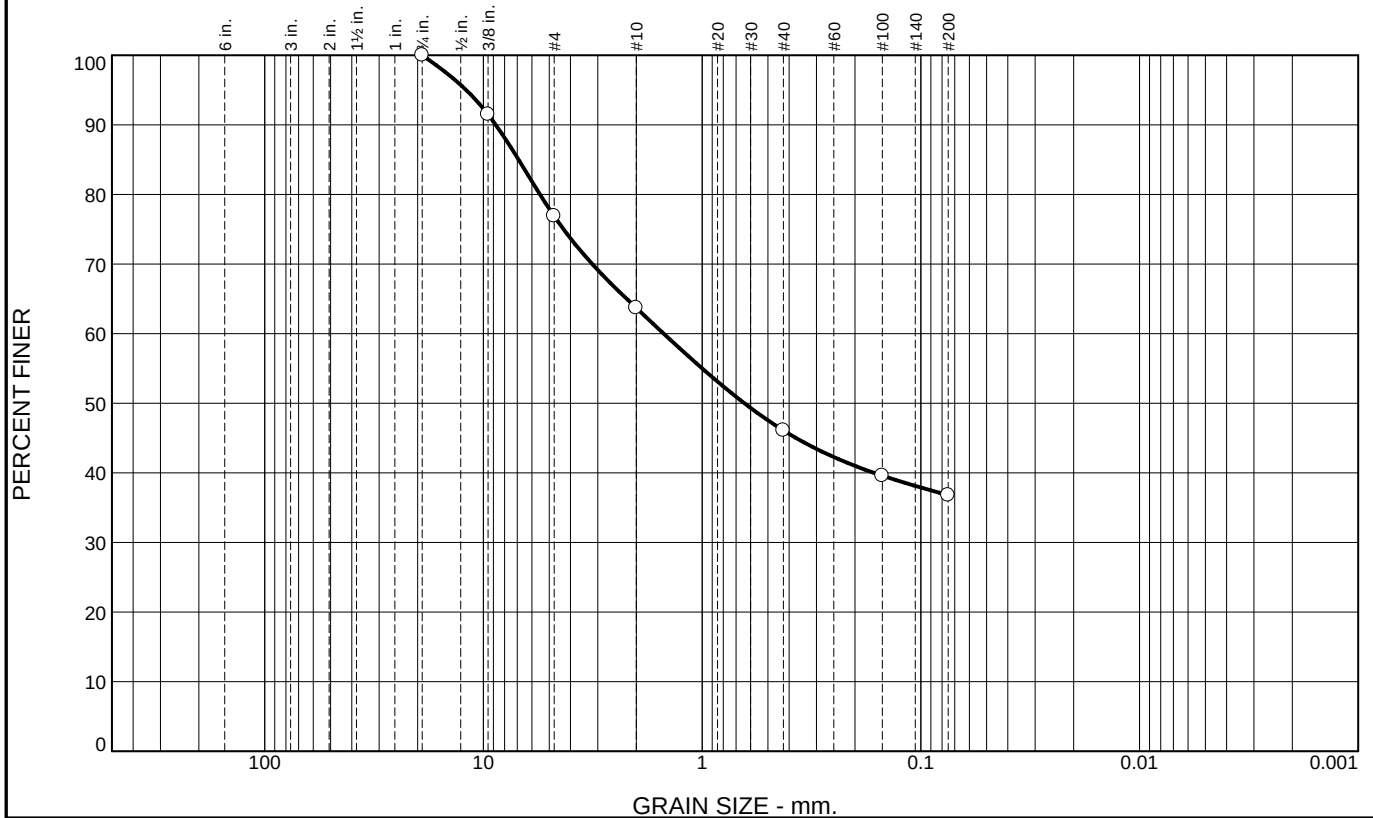
**TABLE III
COMPACTION CRITERIA**

LOCATION	PERCENT COMPACTION (ASTM-D698)
Foundations	98%
Floor Slabs	98%
Pavements	95%
Berms (non-structural)	93%

APPENDIX

SOIL PARTICLE SIZE ANALYSIS RESULTS

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	23.1	13.2	17.6	9.3	36.8	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
.75	100.0		
.375	91.5		
#4	76.9		
#10	63.7		
#40	46.1		
#100	39.6		
#200	36.8		

* (no specification provided)

Material Description

Red to dark red weathered shale.
(silty sand with gravel)

Atterberg Limits

PL= NP

LL= NV

PI= NP

Coefficients

D₉₀= 8.7798

D₈₅= 6.9098

D₆₀= 1.4980

D₅₀= 0.6402

D₃₀=

D₁₅=

D₁₀=

C_u=

C_c=

Classification

USCS= SM

AASHTO= A-4(0)

Remarks

B1/S2 as received moisture content of 11.65%.

B3/S1 as received moisture content of 8.67%.

Source of Sample: B1/B3
Sample Number: S2/S1

Depth: 2'-6"

Date: 5/15/2026

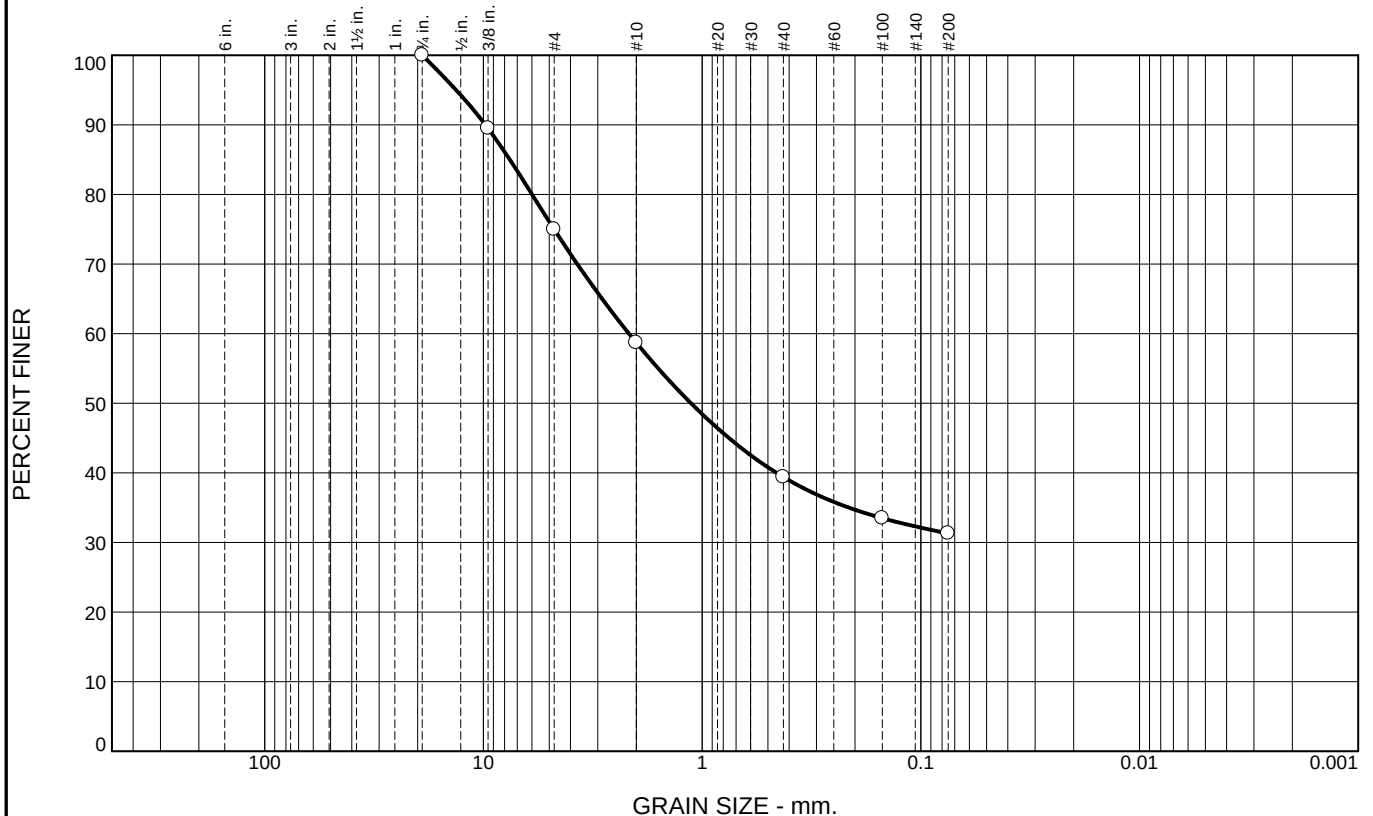


Client: Pottsgrove School District C/O CMTA
Project: Pottsgrove High School Stadium

Project No: 5813G1

Plate

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	25.0	16.3	19.3	8.1	31.3	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
.75	100.0		
.375	89.5		
#4	75.0		
#10	58.7		
#40	39.4		
#100	33.5		
#200	31.3		

* (no specification provided)

Material Description

Red to dark red weathered shale with traces of clay.
(silty sand with gravel)

Atterberg Limits

PL= NP

LL= NV

PI=

Coefficients

D₉₀= 9.7915

D₈₅= 7.5738

D₆₀= 2.1610

D₅₀= 1.1241

D₃₀=

D₁₅=

D₁₀=

C_u=

C_c=

Classification

USCS= SM

AASHTO= A-2-4(0)

Remarks

B4/S1 as received moisture content of 9.21%.

B9/S2 as received moisture content of 13.74%.

Source of Sample: B4/B9
Sample Number: S1/S2

Depth: 2'-6"

Date: 5/15/2026

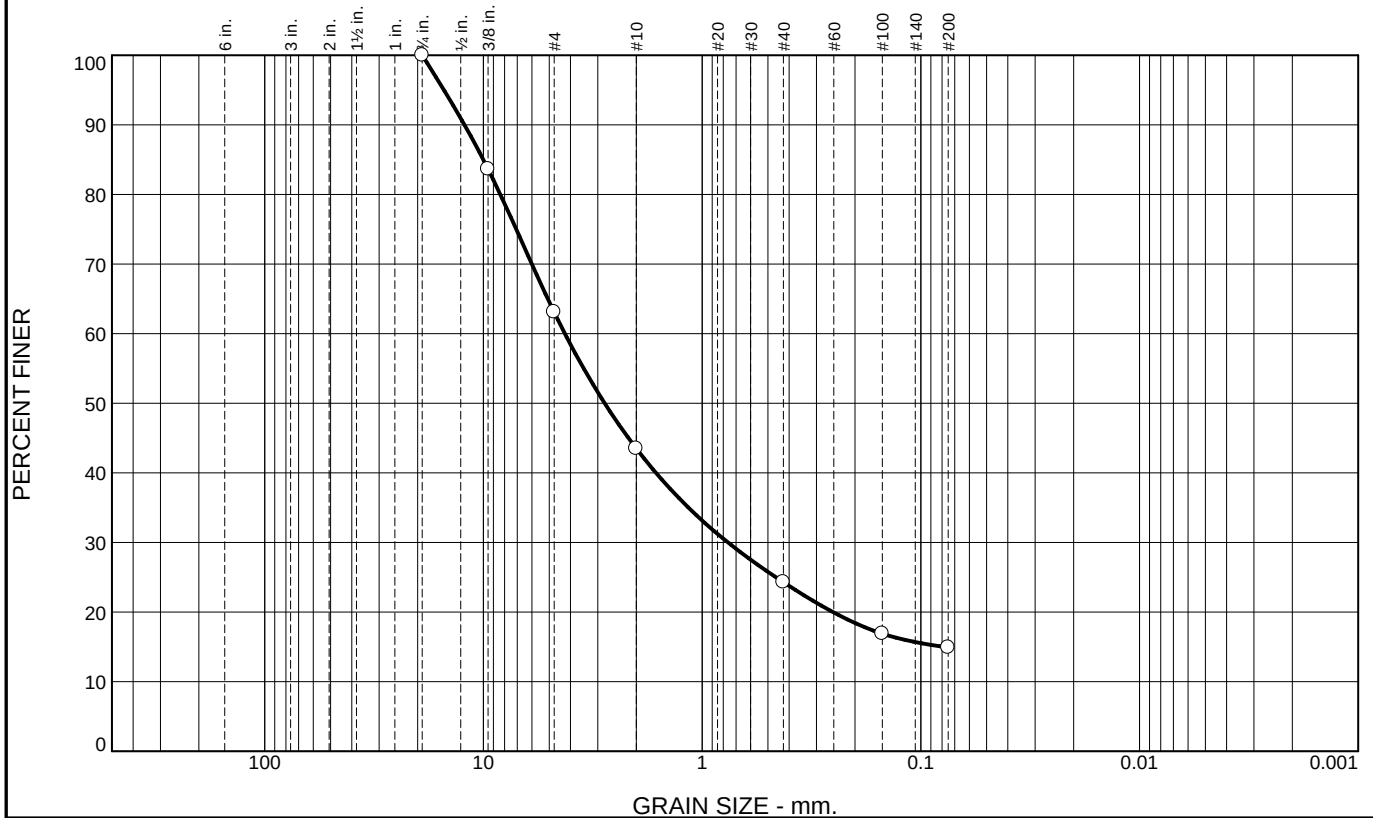


Client: Pottsgrove School District C/O CMTA
Project: Pottsgrove High School Stadium

Project No: 5813G1

Plate

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	36.9	19.6	19.2	9.4	14.9	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
.75	100.0		
.375	83.7		
#4	63.1		
#10	43.5		
#40	24.3		
#100	16.9		
#200	14.9		

* (no specification provided)

<u>Material Description</u>		
Red to dark red weathered shale. (silty sand with gravel)		
<u>Atterberg Limits</u>		
PL= NP	LL= NV	PI=
<u>Coefficients</u>		
D ₉₀ = 12.2082	D ₈₅ = 10.0138	D ₆₀ = 4.2409
D ₅₀ = 2.7894	D ₃₀ = 0.7615	D ₁₅ = 0.0786
D ₁₀ =	C _u =	C _c =
<u>Classification</u>		
USCS= SM	AASHTO=	A-1-a
<u>Remarks</u>		
B6/S4 as received moisture content of 5.35%.		
B1/S3 as received moisture content of 4.90%.		

Source of Sample: B6/B1
Sample Number: S4/S3

Depth: 9'-14"

Date: 5/15/2026



Client: Pottsgrove School District C/O CMTA
Project: Pottsgrove High School Stadium

Project No: 5813G1

Plate

TEST BORING LOGS



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Boring Number: B1

Sheet 1 of 1

Project: Pottsgrove High School Stadium
Location: 1301 Kauffman Road
Twp/City/State: Pottstown/ Montgomery County/ PA
Drilling Contractor: Corcoran Drilling Co. Inc.
X Coordinate (ft): Y Coordinate (ft):
Drilling Method #1: Solid Auger from 0' to 12.17'
Drilling Method #2:

Project Number: 5813G1
Date Drilled: 3/28/2026
Inspected By: AYM
Boring Depth: 12.17'
Ground Surface Elevation (ft msl): 211.39
Water Level - Immediate (ft bgs): Dry (5 min)
Water Level - Static (ft bgs): Dry (5 Hrs)

DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY				SAMPLING DATA														
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)							
							10	20	30	40										
0			Topsoil (12")	1.00	210.39															
3			Stratum II Reddish-brown fine sand and silt with some shale fragments			S-1*					4-8-12-18	20								
6							S-2*				9-13-25-29	38								
9																				
12			Stratum III Weathered red shale	8.00	203.39						32-50/3"	100								
15																				
18			Auger Refusal on Shale bedrock	12.17	199.22															
21			Notes: *Moist Hard augering from 9.75' Very hard augering from 12'																	

The boring results represent subsurface conditions at the boring locations only and are not necessarily representative of conditions at other locations. Water levels are taken at the time of drilling and are not indicative of seasonal variations in the ground water level. NR = No Recovery, S = Split Spoon sample (2" O.D.), C = Rock Core Sample (4" O.D.)

Project: Pottsgrove High School Stadium
 Location: 1301 Kauffman Road
 Twp/City/State: Pottstown/ Montgomery County/ PA
 Drilling Contractor: Corcoran Drilling Co. Inc.
 X Coordinate (ft): Y Coordinate (ft):
 Drilling Method #1: Solid Auger from 0' to 15.83'
 Drilling Method #2:

Project Number: 5813G1
 Date Drilled: 3/28/2026
 Inspected By: AYM
 Boring Depth: 15.83'
 Ground Surface Elevation (ft msl): 214.74
 Water Level - Immediate (ft bgs): 14.50' (5 min)
 Water Level - Static (ft bgs): 9.75' (5 Hrs)

DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY				SAMPLING DATA																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																								
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																	
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0			Topsoil (6")	0.50	214.24																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									



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Boring Number: B3

Sheet 1 of 1

Project: Pottsgrove High School Stadium
Location: 1301 Kauffman Road
Twp/City/State: Pottstown/ Montgomery County/ PA
Drilling Contractor: Corcoran Drilling Co. Inc.
X Coordinate (ft): Y Coordinate (ft):
Drilling Method #1: Solid Auger from 0' to 15.33'
Drilling Method #2:

Project Number: 5813G1
Date Drilled: 3/28/2026
Inspected By: AYM
Boring Depth: 15.33'
Ground Surface Elevation (ft msl): 213.74
Water Level - Immediate (ft bgs): Dry (5 min)
Water Level - Static (ft bgs): Dry (5 Hrs)

DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY				SAMPLING DATA							
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)
							10	20	30	40			
0			Topsoil (16")										
			Stratum II	1.33	212.41								
			Orange-brown fine sand and silt/ trace of clay	1.50	212.24								
3			Reddish-brown fine sand and silt with some shale fragments	3.25	210.49	S-1					8-21-43-50/3"	64	
			Stratum III										
			Weathered red shale			S-2					36-50/5"	100	
6													
9						S-3					50/3"	100	
12													
15						S-4					50/1"	100	
				15.33	198.41								
			Auger Refusal on Shale bedrock										
			Notes: Hard augering from 5.5'										
18													
21													

The boring results represent subsurface conditions at the boring locations only and are not necessarily representative of conditions at other locations. Water levels are taken at the time of drilling and are not indicative of seasonal variations in the ground water level. NR = No Recovery, S = Split Spoon sample (2" O.D.), C = Rock Core (4" O.D.)



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Boring Number: B4

Sheet 1 of 1

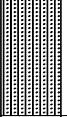
Project: Pottsgrove High School Stadium
Location: 1301 Kauffman Road
Twp/City/State: Pottstown/ Montgomery County/ PA
Drilling Contractor: Corcoran Drilling Co. Inc.
X Coordinate (ft): Y Coordinate (ft):
Drilling Method #1: Solid Auger from 0' to 15.33'
Drilling Method #2:

Project Number: 5813G1
Date Drilled: 3/28/2026
Inspected By:
Boring Depth: 11.75'
Ground Surface Elevation (ft msl): 215.49
Water Level - Immediate (ft bgs): Dry (5 min)
Water Level - Static (ft bgs): Dry (5 Hrs)

DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY				SAMPLING DATA							
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)
							10	20	30	40			
0			Topsoil (13")	1.07	214.42								
			Stratum II Orange-brown fine sand and clayey silt	2.58	212.91								
3			Reddish-brown and gray fine sand and clayey silt with shale fragments	4.58	210.91	S-1*					9-10-21-37	31	
			Stratum III Weathered red shale			S-2					20-50/5"	100	
6													
9						S-3					28-50/4"	100	
12			Auger Refusal on Shale bedrock	11.75	203.74								
15			Notes: *Moist Hard augering from 10' Very hard augering from 11.58'										
18													
21													

Project: Pottsgrove High School Stadium
 Location: 1301 Kauffman Road
 Twp/City/State: Pottstown/ Montgomery County/ PA
 Drilling Contractor: Corcoran Drilling Co. Inc.
 X Coordinate (ft): Y Coordinate (ft):
 Drilling Method #1: Solid Auger from 0' to 14.83'
 Drilling Method #2:

Project Number: 5813G1
 Date Drilled: 3/27/2026
 Inspected By: AYM
 Boring Depth: 14.83'
 Ground Surface Elevation (ft msl): 214.93
 Water Level - Immediate (ft bgs): Dry (5 min)
 Water Level - Static (ft bgs): 12.42 (5 Hrs)

DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY				SAMPLING DATA							
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)
							10	20	30	40			
0			Asphalt (3") Modified stone (5")	0.67	214.26								
			Stratum II Reddish-brown fine sand and silt with many shale fragments	2.67	212.26	S-1					11-19-34-50/3"	53	
3			Stratum III Weathered red shale			S-2					50/5"	100	
6													
9						S-3					50/3"	100	
12													
15				14.83	200.10	S-4					50/1"-N.R.	100	
18			Auger Refusal on Shale bedrock										
21			Notes: Hard augering from 4'										



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Boring Number: B6

Sheet 1 of 1

Project: Pottsgrove High School Stadium
Location: 1301 Kauffman Road
Twp/City/State: Pottstown/ Montgomery County/ PA
Drilling Contractor: Corcoran Drilling Co. Inc.
X Coordinate (ft): Y Coordinate (ft):
Drilling Method #1: Solid Auger from 0' to 16.33'
Drilling Method #2:

Project Number: 5813G1
Date Drilled: 3/27/2026
Inspected By: AYM
Boring Depth: 16.33'
Ground Surface Elevation (ft msl): 217.18
Water Level - Immediate (ft bgs): Dry (5 min)
Water Level - Static (ft bgs): 12.67 (5 Hrs)

DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY				SAMPLING DATA							
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)
							10	20	30	40			
0			Asphalt (3")	0.83	216.35								
			Modified Stone (7")	1.42	215.76								
			Stratum IF Fill consisting of brown and gray organic silt	2.42	214.76								
3			Stratum II Orange-brown fine sand and silt Reddish-brown fine sand and silt with many shale fragments	5.00	212.18	S-1					4-6-9-15	15	
6			Stratum III Weathered red shale			S-2					14-16-29-23	45	
9						S-3					50/3"	100	
12													
15						S-4					50/3"	100	
16.33				16.33	200.85								
18			Auger Refusal on Shale bedrock										
21			Notes: Fairly hard augering from 7' hard augering from 8'										

The boring results represent subsurface conditions at the boring locations only and are not necessarily representative of conditions at other locations. Water levels are taken at the time of drilling and are not indicative of seasonal variations in the ground water level. NR = No Recovery, S = Split Spoon sample (2" O.D.), C = Rock Core Sample



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Boring Number: B7

Sheet 1 of 1

Project: Pottsgrove High School Stadium
Location: 1301 Kauffman Road
Twp/City/State: Pottstown/ Montgomery County/ PA
Drilling Contractor: Corcoran Drilling Co. Inc.
X Coordinate (ft): Y Coordinate (ft):
Drilling Method #1: Solid Auger from 0' to 10.25'
Drilling Method #2:

Project Number: 5813G1
Date Drilled: 3/27/2026
Inspected By: AYM
Boring Depth: 10.25'
Ground Surface Elevation (ft msl): 218.77
Water Level - Immediate (ft bgs): Dry (5 min)
Water Level - Static (ft bgs): Dry (5 Hrs)

DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY				SAMPLING DATA							
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)
							10	20	30	40			
0			Asphalt (3") Modified stone (9")	1.00	217.77								
			Stratum II Reddish-brown fine sand and silt with some shale fragments	2.75	216.02								
3			Stratum III Weathered red shale			S-1					16-19-30-44	49	
						S-2					30-26-50/3"	100	
6													
9						S-3					50/1"	100	
				10.25	208.52								
12			Auger Refusal on Shale bedrock										
			Notes: Hard augering from 5.25' Very hard augering from 9.5'										
15													
18													
21													

The boring results represent subsurface conditions at the boring locations only and are not necessarily representative of conditions at other locations. Water levels are taken at the time of drilling and are not indicative of seasonal variations in the ground water level. NR = No Recovery, S = Split Spoon sample (2" O.D.), C = Recovery (2" O.D.)



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Geotechnical & Environmental Engineers

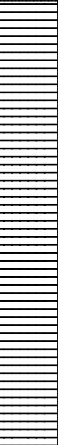
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Boring Number: B8

Sheet 1 of 1

Project: Pottsgrove High School Stadium
Location: 1301 Kauffman Road
Twp/City/State: Pottstown/ Montgomery County/ PA
Drilling Contractor: Corcoran Drilling Co. Inc.
X Coordinate (ft): Y Coordinate (ft):
Drilling Method #1: Solid Auger from 0' to 9.67'
Drilling Method #2:

Project Number: 5813G1
Date Drilled: 3/27/2026
Inspected By: AYM
Boring Depth: 9.67'
Ground Surface Elevation (ft msl): 219.31
Water Level - Immediate (ft bgs): Dry (5 min)
Water Level - Static (ft bgs): Dry (5 Hrs)

DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY				SAMPLING DATA															
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)								
							10	20	30	40			10	20	30	40	60	80			
0			Topsoil (6")	0.50	218.81																
			Stratum II Reddish-brown fine sand and silt with many shale fragments																		
3				3.00	216.31	S-1						25-44-50/5"	100								
				Stratum III Weathered red shale									23-50/5"	100							
							S-2														
6																					
9				9.67	209.64	S-3						50/1"	100								
			Auger Refusal on Shale bedrock																		
12			Notes: Hard augering from 6' Very hard augering from 9.5'																		
15																					
18																					
21																					



DAVID BLACKMORE & ASSOCIATES, INC.
Geotechnical & Environmental Engineers

Phone: 610-495-6255 Fax: 610-495-7353
 www.dbaengineering.com

Boring Number: B9

Sheet 1 of 1

Project: Pottsgrove High School Stadium
 Location: 1301 Kauffman Road
 Twp/City/State: Pottstown/ Montgomery County/ PA
 Drilling Contractor: Corcoran Drilling Co. Inc.
 X Coordinate (ft): Y Coordinate (ft):
 Drilling Method #1: Solid Auger from 0' to 19'
 Drilling Method #2: Split Spoon from 19' to 21'

Project Number: 5813G1
 Date Drilled: 3/27/2026
 Inspected By: AYM
 Boring Depth: 21'
 Ground Surface Elevation (ft msl): 215.42
 Water Level - Immediate (ft bgs): Dry (5 min)
 Water Level - Static (ft bgs): 18.5' (5 Hrs)

DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY				SAMPLING DATA																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									
							10	20	30	40			10	20	30	40	60	80																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
0			Asphalt (3") Modified stone (4")	0.58	214.84																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																	

The boring results represent subsurface conditions at the boring locations only and are not necessarily representative of conditions at other locations. Water levels are taken at the time of drilling and are not indicative of seasonal variations in the ground water level. NR = No Recovery, S = Split Spoon sample (2" O.D.), C = Rock Coring Run.



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Boring Number: B10

Sheet 1 of 1

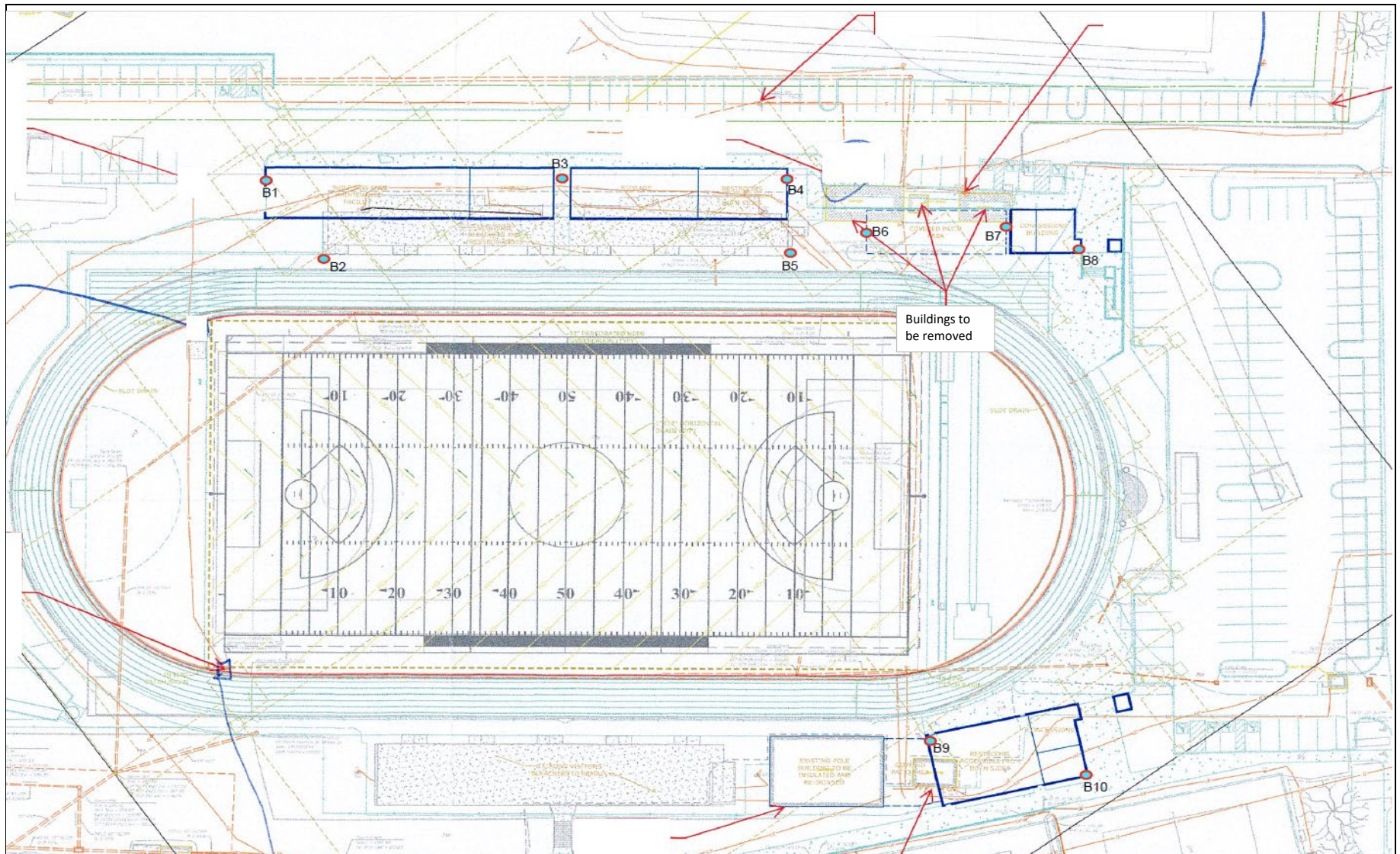
Project: Pottsgrove High School Stadium
 Location: 1301 Kauffman Road
 Twp/City/State: Pottstown/ Montgomery County/ PA
 Drilling Contractor: Corcoran Drilling Co. Inc.
 X Coordinate (ft): Y Coordinate (ft):
 Drilling Method #1: Solid Auger from 0' to 6.33'
 Drilling Method #2:

Project Number: 5813G1
 Date Drilled: 3/27/2026
 Inspected By: AYM
 Boring Depth: 6.58'
 Ground Surface Elevation (ft msl): 215.51
 Water Level - Immediate (ft bgs): Dry (5 min)
 Water Level - Static (ft bgs): Dry (5 Hrs)

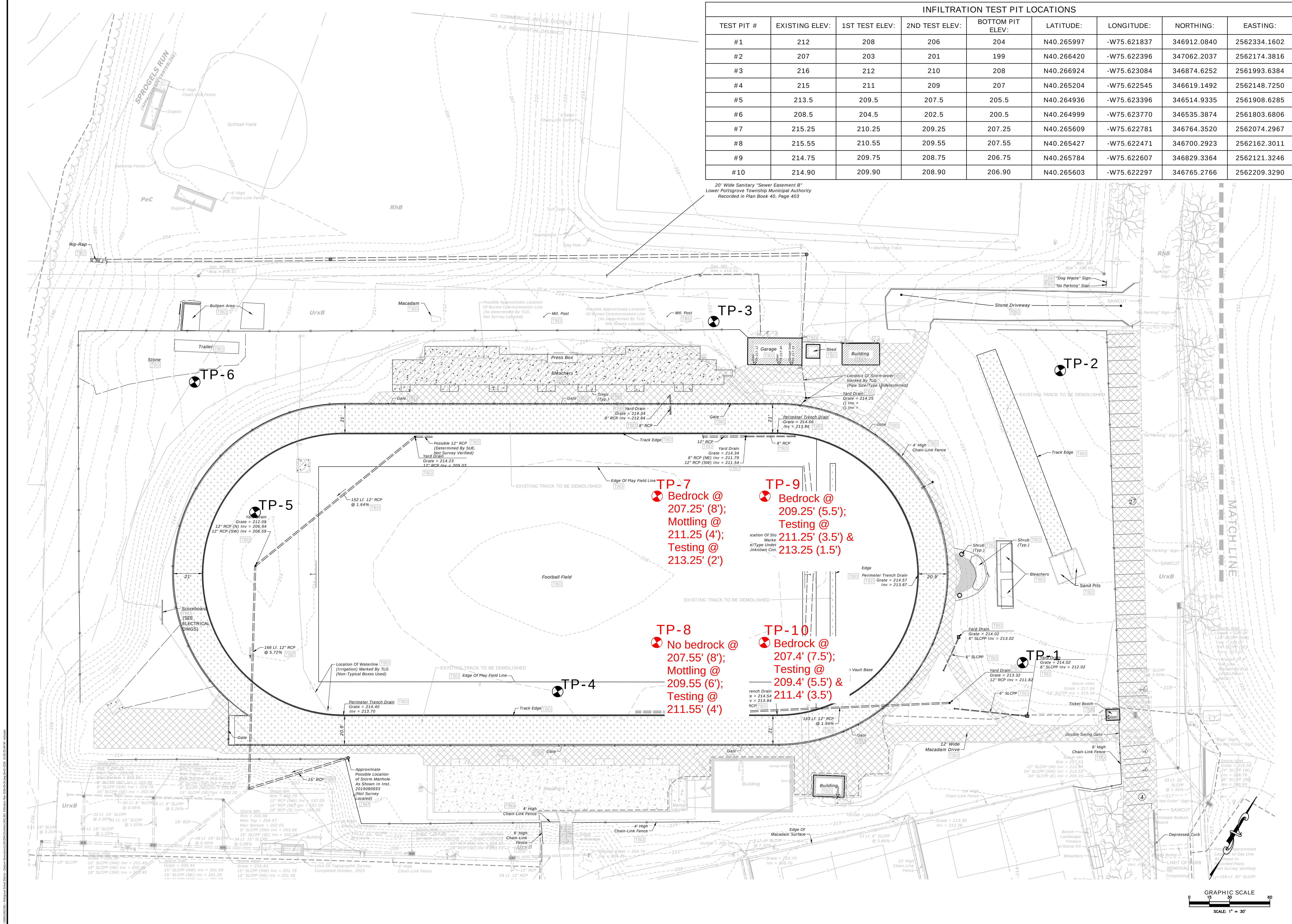
DEPTH BELOW SURFACE (FT)	WATER LEVEL	LITHOLOGY			SAMPLING DATA																	
		LITHOLOGIC SYMBOL	GEOLOGIC DESCRIPTION OF SOIL AND ROCK STRATA	DEPTH (FT)	ELEVATION (FT)	NUMBER	Water Content				SPT DATA	SPT Value	SPT GRAPH (Blows Per Foot)									
							10	20	30	40			10	20	30	40	60	80				
0			Topsoil (8")	0.67	214.84																	
			Stratum III Weathered red shale																			
				S-1								50/3"	100									
3																						
				S-2								50/1"	100									
			Auger Refusal on Shale Bedrock																			
6																						
9			Notes: Hard augering from 1.67' Very hard augering from 6.25' 1st attempt: Auger refusal at 6.33' 2nd attempt: Moved 10' towards B9 Hard augering from 2' Auger refusal at 6.58'																			
12																						
15																						
18																						
21																						

The boring results represent subsurface conditions at the boring locations only and are not necessarily representative of conditions at other locations. Water levels are taken at the time of drilling and are not indicative of seasonal variations in the ground water level. NR = No Recovery, S = Split Spoon sample (2" O.D.), C = Rock Coring Run.

TEST BORING LOCATION PLAN



	<p>KEY</p> <p>● TEST BORING</p>	<p>GEOTECHNICAL & ENVIRONMENTAL ENGINEERS DAVID BLACKMORE & ASSOC., INC. 3335 West Ridge Pike Pottstown, Pennsylvania 19464 Telephone: (610) 495-6255 FAX: (610) 495-7353</p>	<p>Project 5813G1 POTTSGROVE STADIUM TEST BORING LOCATION PLAN 1301 Kauffman Road Pottsgrove Twp, Montgomery County, PA</p>
--	--	--	--



INFILTRATION TEST PIT LOCATIONS								
TEST PIT #	EXISTING ELEV.	1ST TEST ELEV.	2ND TEST ELEV.	BOTTOM PIT ELEV.	LATITUDE:	LONGITUDE:	NORTHING:	EASTING:
#1	212	208	206	204	N40.265997	-W75.621837	346912.0840	2562334.1602
#2	207	203	201	199	N40.266420	-W75.622396	347062.2037	2562174.3816
#3	216	212	210	208	N40.266924	-W75.623084	346874.6252	2561993.6384
#4	215	211	209	207	N40.265204	-W75.622545	346619.1492	2562148.7250
#5	213.5	209.5	207.5	205.5	N40.264936	-W75.623396	346514.9335	2561908.6285
#6	208.5	204.5	202.5	200.5	N40.264999	-W75.623770	346535.3874	2561803.6806
#7	215.25	210.25	209.25	207.25	N40.265609	-W75.622781	346764.3520	2562074.2967
#8	215.55	210.55	209.55	207.55	N40.265427	-W75.622471	346700.2923	2562162.3011
#9	214.75	209.75	208.75	206.75	N40.265784	-W75.622607	346829.3364	2562121.3246
#10	214.90	209.90	208.90	206.90	N40.265603	-W75.622297	346765.2766	2562209.3290

INFLTRATION TESTING PLAN

PRELIMINARY INFILTRATION PLAN

FOR

POTTS GROVE SCHOOL DISTRICT

PROFESSIONAL SEAL

SCALE: JANUARY 6, 2026

K&W PROJECT: 2402.001

DRAWN BY:

CAD DRAWING: 2402.001 Infiltration Plan 2026-05-26.dwg

REVISIONS

PLAN TYPE: PRELIMINARY INFILTRATION TESTING PLAN

SHEET: 1 OF 1

K&W

DESIGNING ENVIRONMENTS

2201 North Front Street, Suite 200

Harrisburg, PA 17110

P: 717.635.2835

F: 717.635.2835

www.kandwengineers.com

MONTGOMERY COUNTY, PA

LOWER POTTS GROVE TOWNSHIP

NO.

DATE

DESCRIPTION

1

2

3

4

5

6

7

NO.

DATE

DESCRIPTION

1

2

3

4

5

6

7



Project: **Pottsgrove SD - Stadium Improvements**
Project No.: **2402.001**
Client: **Pottsgrove School District**

TEST PIT LOG

Test Pit No.: **TP-1**
Page: **1** of **1**

Excavation Company: Cat Iron	Date of Test Pit: 2/17/2026	Test Pit Elevation ±212 FT
Excavation Equipment: Kubota KX57	Field Representative: M. Wright	Groundwater Yes ☐ No ☒ Elevation: Not Enc.
Operator: Mike	Drawn/Compiled By: M. Wright	
	Approved By: D. Buckwalter	
	Weather/Temperature Sunny 37°	

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0'- 0.66' Brown to Dark Brown Sandy Silt	Topsoil	
			0.66' - 2.5' Red Brown Sandy Gravel and Cobble, Highly Weathered Shale Bedrock (HWSB)		
2				Stratum II	2
4			-Bucket Refusal @ 2.5 Feet- -End of Test Pit @ 2.5 Feet-		4
6					6
8					8
10					10
12					12
14					14

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Project: **Pottsgrove SD - Stadium Improvements**
Project No.: **2402.001**
Client: **Pottsgrove School District**

TEST PIT LOG

Test Pit No.: **TP-2**
Page: **1** of **1**

Excavation Company: Cat Iron	Date of Test Pit: 2/18/2026	Test Pit Elevation ±207 FT
Excavation Equipment: Kubota KX57	Field Representative: M. Wright	Groundwater Yes ☒ No ☐ Elevation: Not Enc.
Operator: Mike	Drawn/Compiled By: M. Wright	
	Approved By: D. Buckwalter	
	Weather/Temperature Sunny 38°	

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0'- 0.66' Brown to Dark Brown Sandy Silt	Topsoil	
			0.66' - 2.0' Brown to Red Brown Silty Clay, increaseing Gravel w/ depth		
2		▼		Stratum I	2
			2.0' - 3.5' Red Brown Sandy Gravel and Cobble, Highly Weathered Shale Bedrock (HWSB)	Water @ 2.5'	
				Stratum II	
4			-Bucket Refusal @ 3.5 Feet- -End of Test Pit @ 3.5 Feet-		4
6					6
8					8
10					10
12					12
14					14

▼ Initial Water Level Notes:
▽ Final Water Level



Project: **Pottsgrove SD - Stadium Improvements**
Project No.: **2402.001**
Client: **Pottsgrove School District**

TEST PIT LOG

Test Pit No.: **TP-3**
Page: **1 of 1**

Excavation Company: Cat Iron	Date of Test Pit: 2/18/2026	Test Pit Elevation ±216 FT
Excavation Equipment: Kubota KX57	Field Representative: M. Wright	Groundwater Yes ☐ No ☒ Elevation: Not Enc.
Operator: Mike	Drawn/Compiled By: M. Wright	
	Approved By: D. Buckwalter	
	Weather/Temperature Sunny 40°	

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0'- 0.66' Brown to Dark Brown Sandy Silt	Topsoil	
			0.66' - 4.5' Brown Silt with Gravel		
2					2
4				Stratum I	4
			4.5' - 6.0' Red Brown Sandy Gravel and Cobble, Highly Weathered Shale Bedrock (HWSB)		
6				Stratum II	6
			-Bucket Refusal @ 6.0 Feet-		
			-End of Test Pit @ 6.0 Feet-		
8					8
10					10
12					12
14					14



Project: Pottsgrove SD - Stadium Improvements
Project No.: 2402.001
Client: Pottsgrove School District

TEST PIT LOG

Test Pit No.: TP-4
Page: 1 of 1

Excavation Company: <u>Cat Iron</u>	Date of Test Pit: <u>2/17/2026</u>	Test Pit Elevation <u>±215 FT</u>
Excavation Equipment: <u>Kubota KX57</u>	Field Representative: <u>M. Wright</u>	Groundwater Yes ☐ No ☒ Elevation: <u>Not Enc.</u>
Operator: <u>Mike</u>	Drawn/Compiled By: <u>M. Wright</u>	
	Approved By: <u>D. Buckwalter</u>	
	Weather/Temperature <u>Sunny 39°</u>	

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0'- 0.66' Brown to Dark Brown Sandy Silt	Topsoil	
			0.66' - 6.0' Brown to Red Brown Silty Clay, trace Gravel		
2					2
4					4
6				Stratum I	6
			-End of Test Pit @ 6.0 Feet-		
8					8
10					10
12					12
14					14

Initial Water Level Notes:
 Final Water Level



Project: **Pottsgrove SD - Stadium Improvements**
Project No.: **2402.001**
Client: **Pottsgrove School District**

TEST PIT LOG

Test Pit No.: **TP-5**
Page: **1 of 1**

Excavation Company: Cat Iron	Date of Test Pit: 2/17/2026	Test Pit Elevation ±213.5 FT
Excavation Equipment: Kubota KX57	Field Representative: M. Wright	Groundwater Yes ☐ No ☒ Elevation: Not Enc.
Operator: Mike	Drawn/Compiled By: M. Wright	
	Approved By: D. Buckwalter	
	Weather/Temperature Sunny 41°	

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0'- 1.0' Brown to Dark Brown Sandy Silt	Topsoil	
			1.0' - 8.0' Brown to Red Brown Silty Clay, trace Gravel		
2					2
4			increasing Gravel and Cobble		4
6					6
8				Stratum I	8
			-End of Test Pit @ 8.0 Feet-		
10					10
12					12
14					14



Project: **Pottsgrove SD - Stadium Improvements**
Project No.: **2402.001**
Client: **Pottsgrove School District**

TEST PIT LOG

Test Pit No.: **TP-6**
Page: **1 of 1**

Excavation Company: Cat Iron	Date of Test Pit: 2/18/2026	Test Pit Elevation ±208.5 FT
Excavation Equipment: Kubota KX57	Field Representative: M. Wright	Groundwater Yes ☐ No ☒ Elevation: Not Enc.
Operator: Mike	Drawn/Compiled By: M. Wright	
	Approved By: D. Buckwalter	
	Weather/Temperature Sunny 40°	

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0'- 0.83' Brown to Dark Brown Sandy Silt	Topsoil	
			0.83' - 2.0' Brown Silty Sand with Gravel		
2				Stratum I	2
			2.0' - 4.0' Red Brown Sandy Gravel and Cobble, Highly Weathered Shale Bedrock (HWSB)		
4				Stratum II	4
			-Bucket Refusal @ 4.0 Feet-		
			-End of Test Pit @ 4.0 Feet-		
6					6
8					8
10					10
12					12
14					14

Initial Water Level

Final Water Level

Notes:



Project: **Pottsgrove SD - Stadium Improvements**

Project No.: **2402.001**

Client: **Pottsgrove School District**

TEST PIT LOG

Test Pit No.: **TP-7**

Page: **1** of **1**

Excavation Company: **Cat Iron**

Excavation Equipment: **Kubota KX57**

Operator: **Mike**

Date of Test Pit: **6/2/2026**

Field Representative: **M. Wright**

Drawn/Compiled By: **M. Wright**

Approved By: **D. Buckwalter**

Weather/Temperature **Sunny 60°**

Test Pit Elevation
±215.25 FT

Groundwater
Yes ☐ No ☒
Elevation: **Not Enc.**

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0' - 0.83' Brown to Dark Brown Sandy Silt	Topsoil	
			0.83' - 2.0' Brown to Red Brown Silty Sand with Gravel		
2				Fill	2
			2.0' - 6.0' Brown Sandy Silt with Gravel		
4			soil mottling @ 4.0 feet		4
				Stratum I	6
6					
			6.0' - 8.0' Red Brown Sandy Gravel and Cobbles, Highly Weathered Shale Bedrock (HWSB)		
8				Stratum II	8
10			-Bucket Refusal @ 207.25 Feet-		10
			-End of Test Pit @ 207.25 Feet-		
12					12
14					14



Initial Water Level Notes:



Final Water Level



Project: **Pottsgrove SD - Stadium Improvements**
Project No.: **2402.001**
Client: **Pottsgrove School District**

TEST PIT LOG

Test Pit No.: **TP-8**
Page: **1 of 1**

Excavation Company: Cat Iron	Date of Test Pit: 6/2/2026	Test Pit Elevation ±215.55 FT
Excavation Equipment: Kubota KX57	Field Representative: M. Wright	Groundwater Yes ☐ No ☒ Elevation: Not Enc.
Operator: Mike	Drawn/Compiled By: M. Wright	
	Approved By: D. Buckwalter	
	Weather/Temperature Sunny 60°	

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0'- 0.83' Brown to Dark Brown Sandy Silt	Topsoil	
			0.83' - 3.0' Brown to Red Brown Clayey Sand and Gravel		
2					2
				Fill	
4			3.0' - 8.0' Red Brown to Brown Clay with Sand and Gravel		4
			very soft moist soil		
6			soil mottling @ 6.0 feet		6
8				Stratum I	8
			-Bucket Refusal @ 207.55 Feet-		
			-End of Test Pit @ 207.55 Feet-		
10					10
12					12
14					14



Project: **Pottsgrove SD - Stadium Improvements**
Project No.: **2402.001**
Client: **Pottsgrove School District**

TEST PIT LOG

Test Pit No.: **TP-9**
Page: **1 of 1**

Excavation Company: Cat Iron	Date of Test Pit: 6/2/2026	Test Pit Elevation ±214.75 FT
Excavation Equipment: Kubota KX57	Field Representative: M. Wright	Groundwater Yes ☐ No ☒ Elevation: Not Enc.
Operator: Mike	Drawn/Compiled By: M. Wright	
	Approved By: D. Buckwalter	
	Weather/Temperature Sunny 60°	

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0' - 0.83' Brown to Dark Brown Sandy Silt	Topsoil	
			0.83' - 3.5' Brown to Red Brown Silty Sand with Gravel		
2					2
4			3.5' - 5.5' Red Brown Sandy Gravel and Cobbles, Highly Weathered Shale Bedrock (HWSB)	Stratum I	4
6				Stratum II	6
			-Bucket Refusal @ 209.25 Feet-		
			-End of Test Pit @ 209.25 Feet-		
8					8
10					10
12					12
14					14

	Initial Water Level	Notes:
	Final Water Level	



Project: **Pottsgrove SD - Stadium Improvements**
Project No.: **2402.001**
Client: **Pottsgrove School District**

TEST PIT LOG

Test Pit No.: **TP-10**
Page: **1 of 1**

Excavation Company: Cat Iron	Date of Test Pit: 6/2/2026	Test Pit Elevation: ±214.90 FT
Excavation Equipment: Kubota KX57	Field Representative: M. Wright	Groundwater Yes ☐ No ☒ Elevation: Not Enc.
Operator: Mike	Drawn/Compiled By: M. Wright	
	Approved By: D. Buckwalter	
	Weather/Temperature Sunny 60°	

Depth (Ft.)	Lithology	Water Level (ft.)	Soil / Rock Visual Descriptions	Remarks	Depth (Ft.)
			0.0'- 0.83' Brown to Dark Brown Sandy Silt	Topsoil	
			0.83' - 5.5' Red Brown Silt with Sand, trace Gravel		
2					2
4					4
6				Stratum I	
			5.5' - 7.5' Red Brown Sandy Gravel and Cobbles, Highly Weathered Shale Bedrock (HWSB)		6
8				Stratum II	
			-Bucket Refusal @ 207.4 Feet-		8
			-End of Test Pit @ 207.4 Feet-		
10					10
12					12
14					14

	Initial Water Level	Notes:
	Final Water Level	



Project:	Pottsgrove SD - Stadium Improvements	Date of Testing	2/17/2026 2/18/2026 6/2/2026 37-45°F 50-75°F	INFILTRATION TESTING LOG
Project No.:	2402.001			
Client:	Pottsgrove School District	Temperature		Compiled By: Michael Wright Date Compiled: 2/19/2026 & 6/3/2026

Test Hole Number	Surface Elevation (feet)	Infiltration Test Elev (feet)	Drop in Water within Inner Ring during Presoak Period (inches)		Readings/Measurements - Drop in Water within Inner Ring at Time (inches)						Time Increment (Minutes) 10/30	* Infiltration Rate
			30 minutes	60 minutes	1	2	3	4	5	6		
												Inches/hour
IT-1A	212.0	N/A	Infiltration Testing not Completed due to Bedrock									
IT-1B	212.0	N/A	Infiltration Testing not Completed due to Bedrock									
IT-2A	207.0	N/A	Infiltration Testing not Completed due to Perched Water & Bedrock									
IT-2B	207.0	N/A	Infiltration Testing not Completed due to Perched Water & Bedrock									
IT-3A	216.0	214.0	1.25	0.625	0.325	0.325	0.325	0.25			30	0.50
IT-3B	216.0	212.0	0.125	0.0	0.0	0.0	0.0	0.0			30	0.00
IT-4A	215.0	213.0	0.0	0.0	0.0	0.0	0.0	0.0			30	0.00
IT-4B	215.0	211.0	0.0	0.0	0.0	0.0	0.0	0.0			30	0.00
IT-5A	213.5	209.5	0.25	0.25	0.25	0.25	0.25	0.125			30	0.25
IT-5B	213.5	207.5	0.0	0.0	0.0	0.0	0.0	0.0			30	0.00
IT-6A	208.5	206.5	0.25	0.125	0.125	0.125	0.125	0.125			30	0.09**
IT-6B	208.5	N/A	Infiltration Testing not Completed due to Bedrock									
IT-7A	215.25	213.75	0.375	0.125	0.00	0.00	0.00	0.00			30	0.00
IT-7B	215.25	N/A	Infiltration Testing not Completed due to Soil Mottling									
IT-8A	215.55	211.55	0.50	0.50	0.50	0.50	0.50	0.50			30	1.00
IT-8B	215.55	N/A	Infiltration Testing not Completed due to Soil Mottling									
IT-9A	214.75	211.25	0.00	0.00	0.00	0.00	0.00	0.00			30	0.00
IT-9B	214.75	213.25	3.50	2.75	0.50	0.375	0.375	0.375			10	0.72**
IT-10A	214.90	209.40	0.00	0.00	0.00	0.00	0.00	0.00			30	0.00
IT-10B	214.90	211.40	0.00	0.00	0.00	0.00	0.00	0.00			30	0.00

* Stabilized infiltration rates do not include a factor of safety for the design infiltration rate

** Percolation test completed in these tests due to subsurface soil conditions; double ring infiltrometers could not be sealed properly

00 31 34 - Page: 12 of 1