

PROJECT TEAM

CLIENT
Center for Student Learning
Charter School at Pennsbury
345 Lakeside Drive
Levittown, PA 19054
215-269-7390

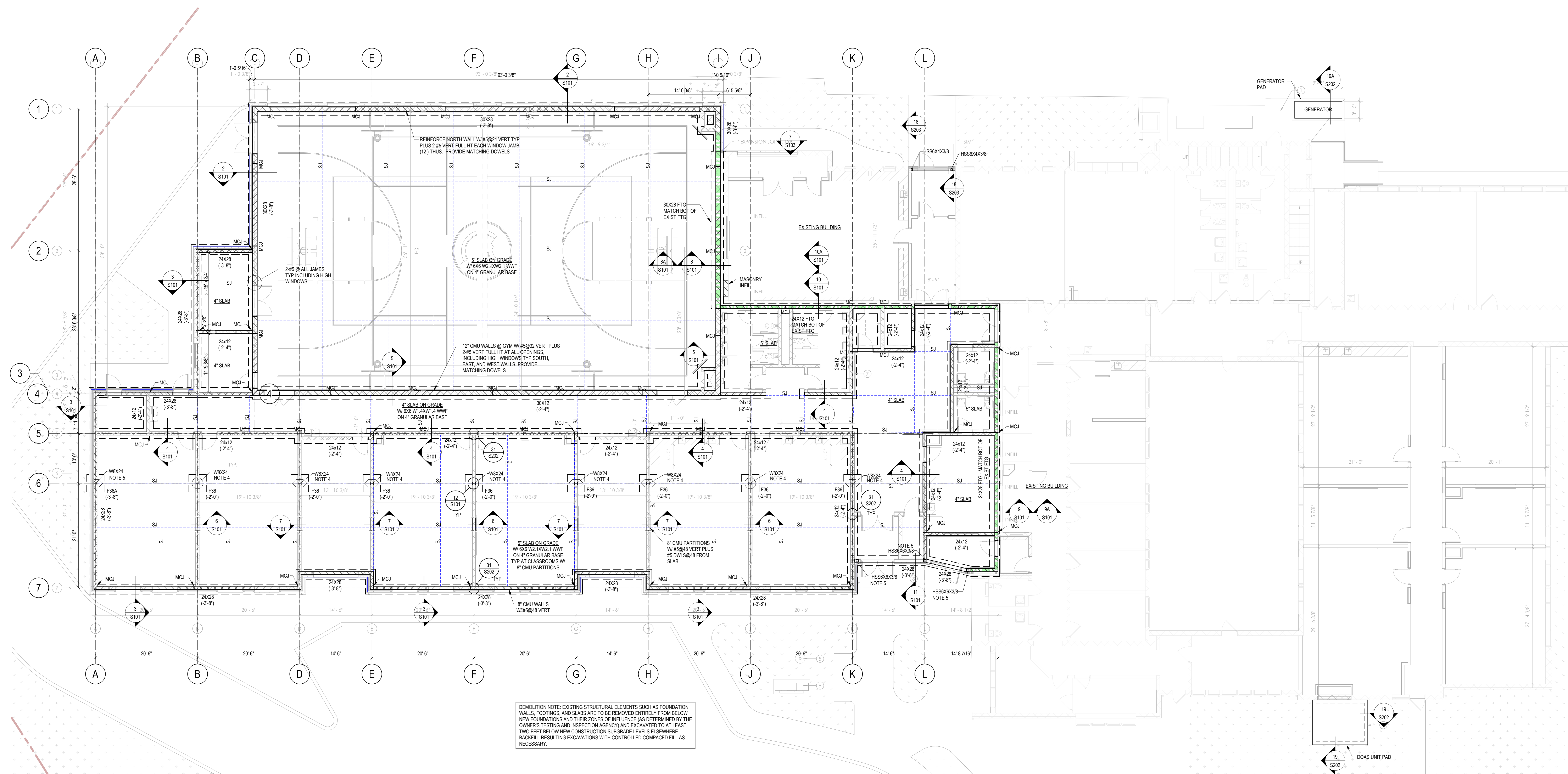
ARCHITECTURAL
Heckendorn Shiles Architects
347 East Conestoga Road
Wayne, PA 19087
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STRUCTURAL ENGINEER
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500 Fayette Street, Suite 100
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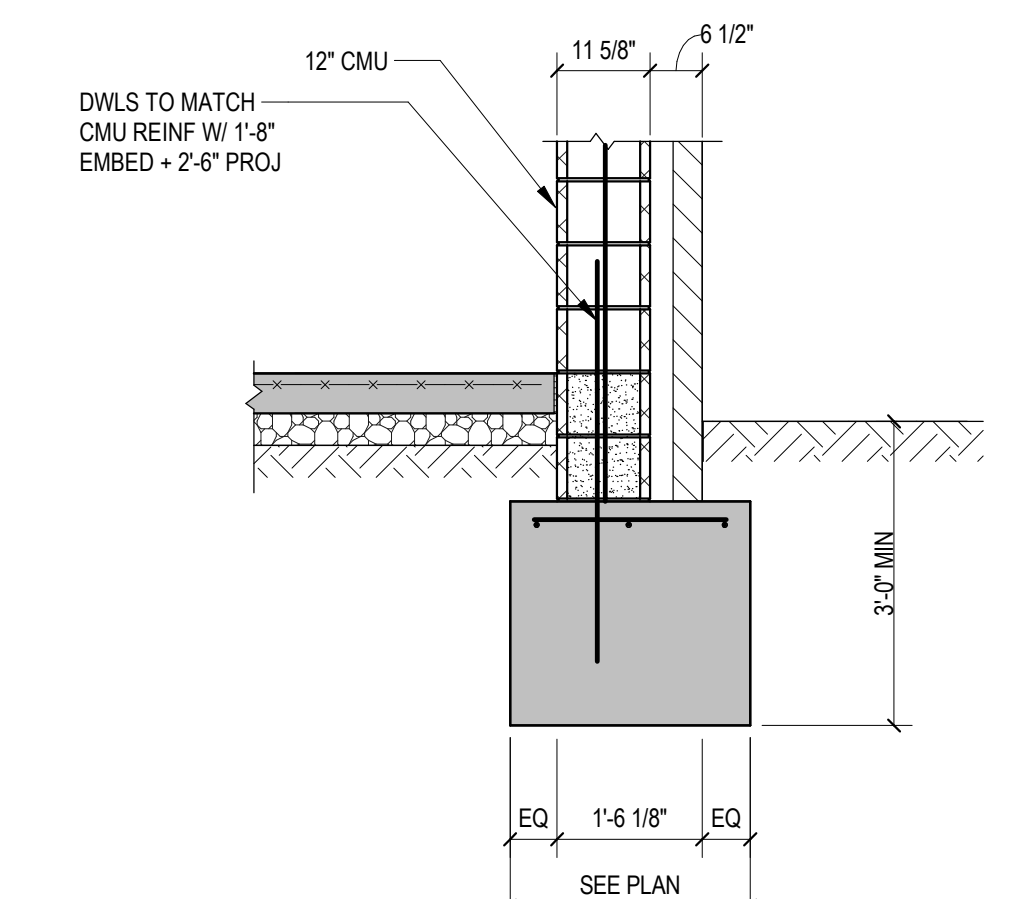
CIVIL ENGINEER
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1515 Market Street, Suite 920
Philadelphia, PA 19102
267-402-3400

NOT FOR
CONSTRUCTION

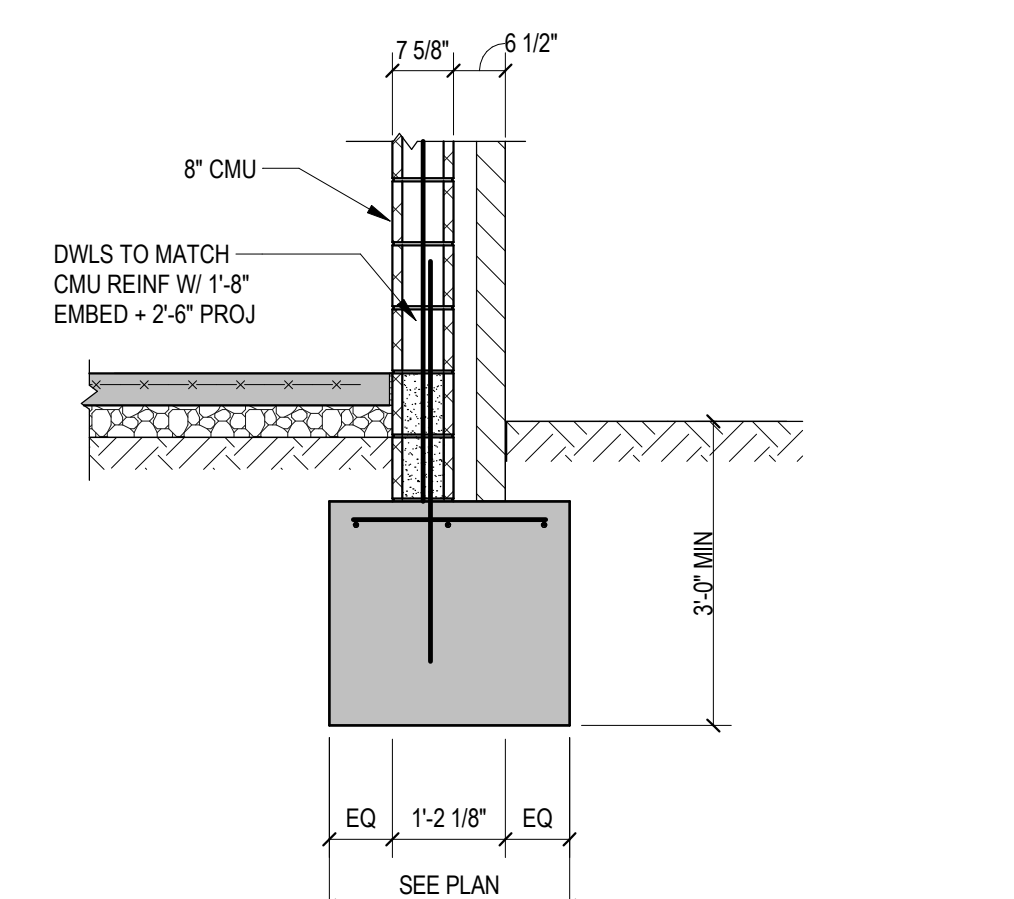


FOUNDATION PLAN
1 S101 1/2" = 1'-0"

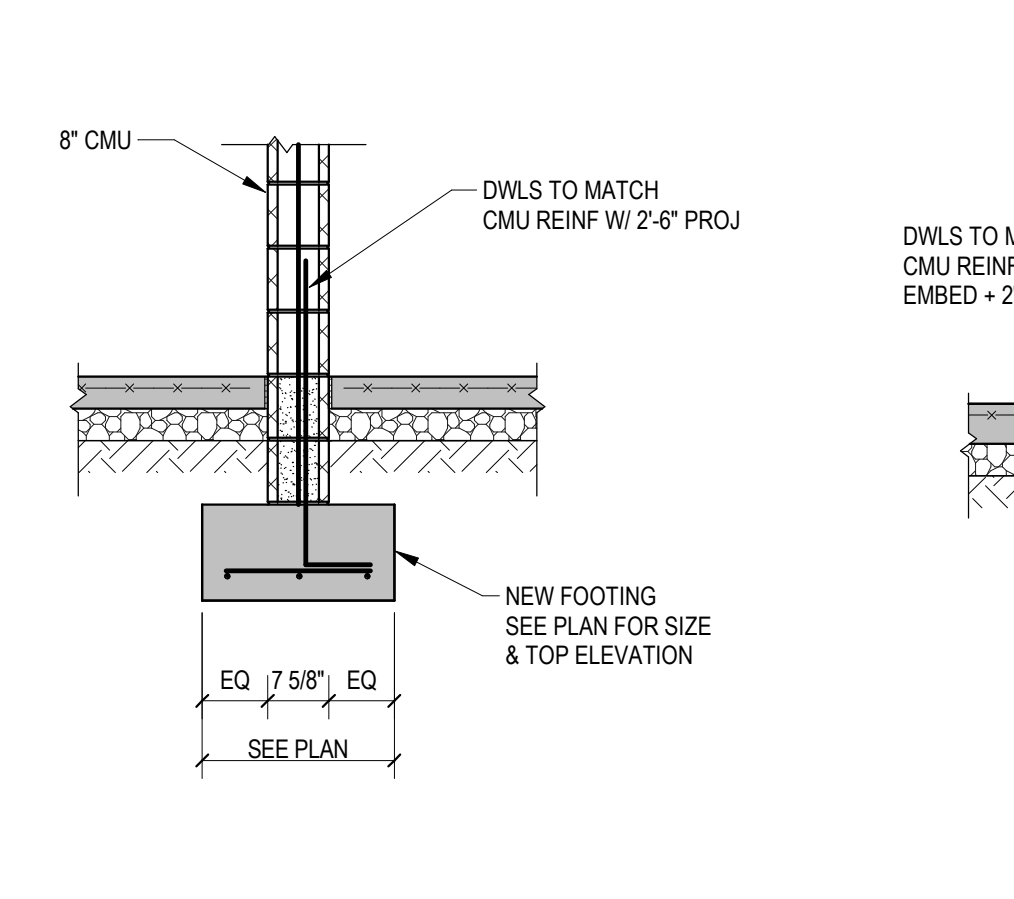
- NOTES:
1. TOP OF SLAB TO MATCH EXISTING TOP OF SLAB ELEVATION. (REF. EL. 0'-0")
 2. NUMBER IN PARENTHESIS IS BOTTOM OF FOOTING ELEVATION FROM REFERENCE ELEVATION 0'-0".
 3. REINFORCE ALL 8" CMU WALLS WITH #5@8" VERT. MINIMUM PLUS TYPICAL VERTICAL AND HORIZONTAL REINFORCING AT OPENINGS, JAMBS, CORNERS, AND INTERSECTIONS PER DETAILS ON S202. REINFORCE 12" CMU WALLS WITH #5@8" VERT. MINIMUM PLUS #5@8" VERT. MINIMUM AT NORTH WALL PLUS TYPICAL VERTICAL AND HORIZONTAL REINFORCING AT OPENINGS, JAMBS, CORNERS, AND INTERSECTIONS PER DETAILS ON S202. SEE PLAN FOR ADDITIONAL REINFORCING NOTES.
 4. BOTTOM OF COLUMN BASE PLATE: 4'-11"
 5. BOTTOM OF COLUMN BASE PLATE: 1'-3"
 6. PROVIDE 1" EXPANSION JOINT BETWEEN NEW AND EXISTING BUILDING.



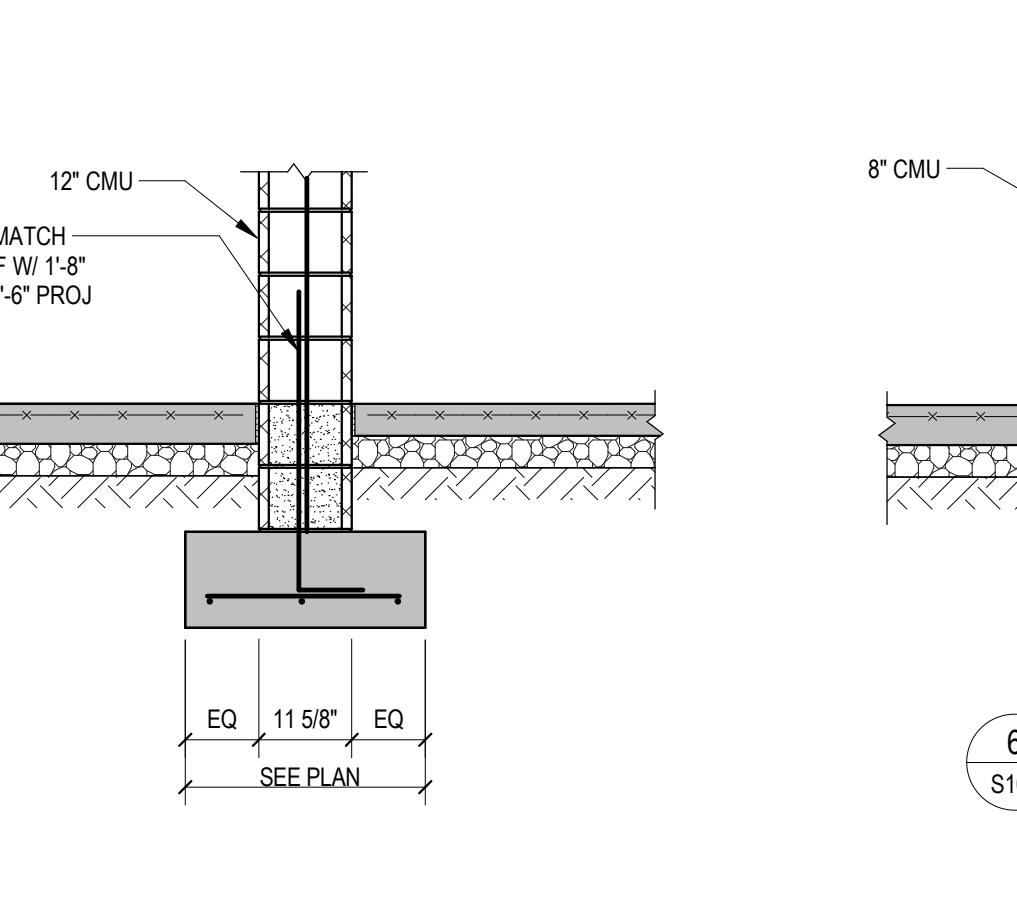
2 SECTION
S101 1/2" = 1'-0"



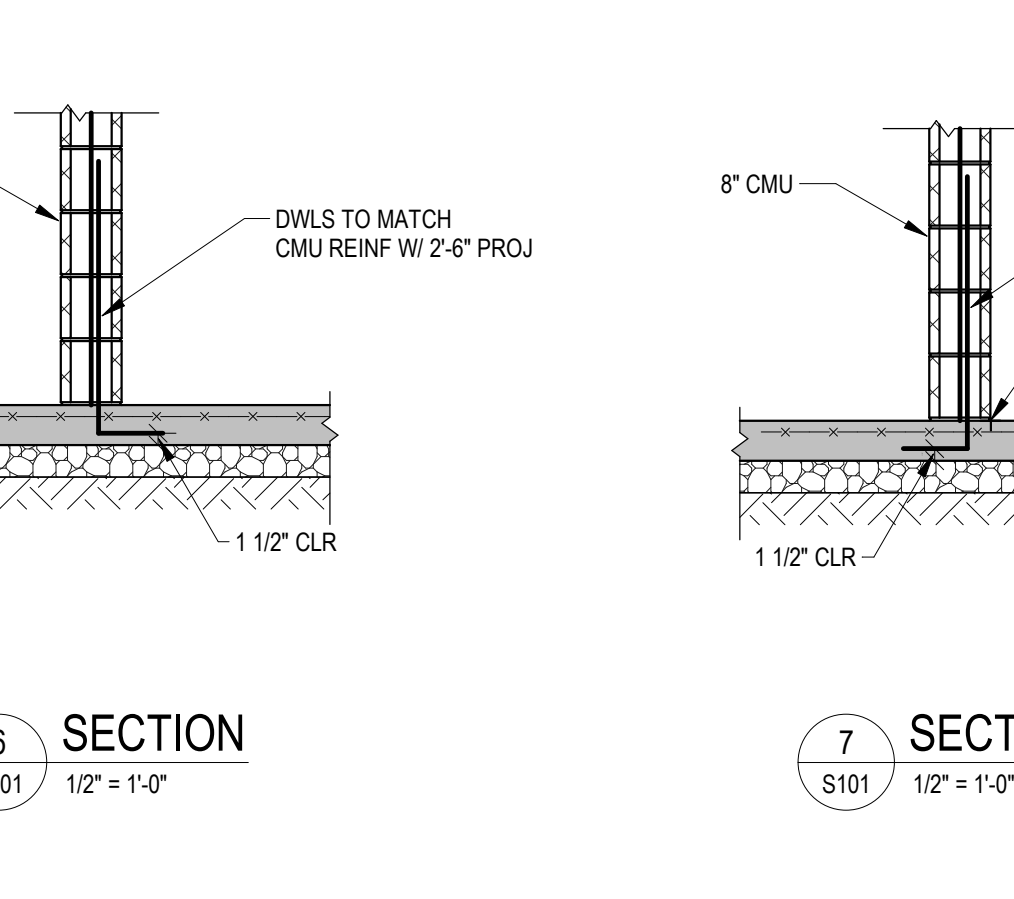
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S101 1/2" = 1'-0"



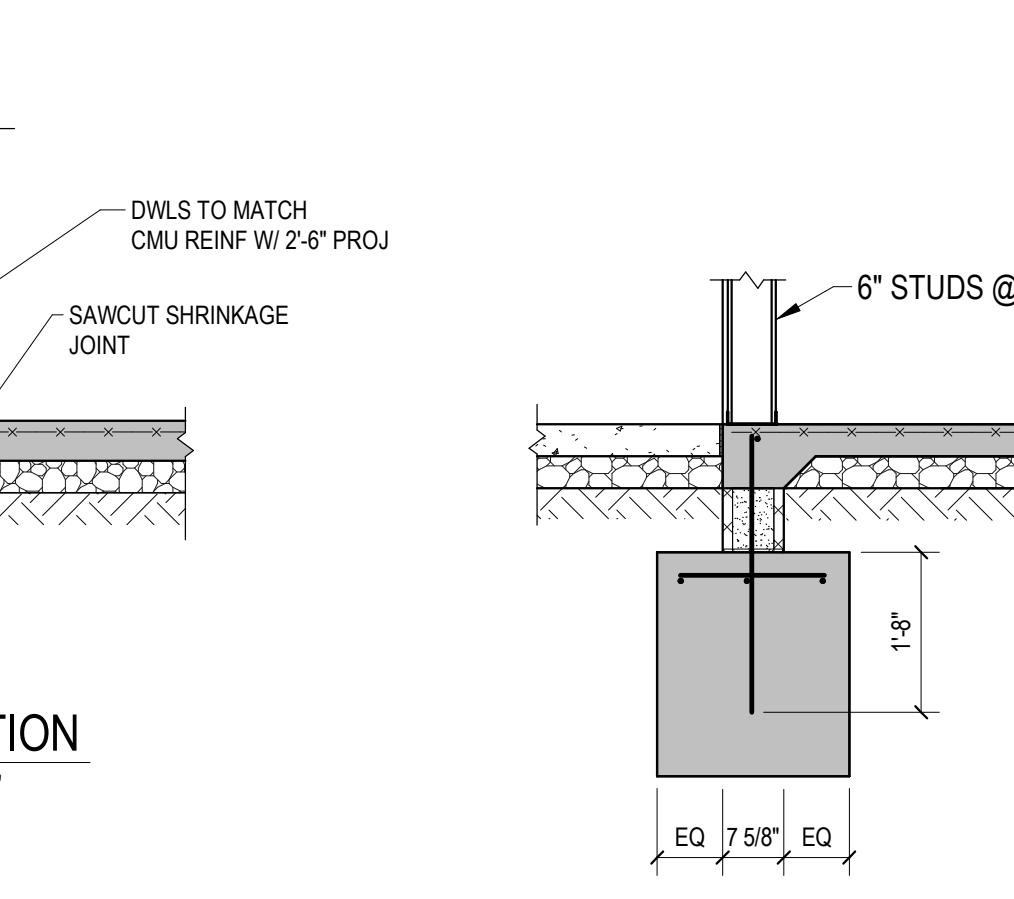
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S101 1/2" = 1'-0"



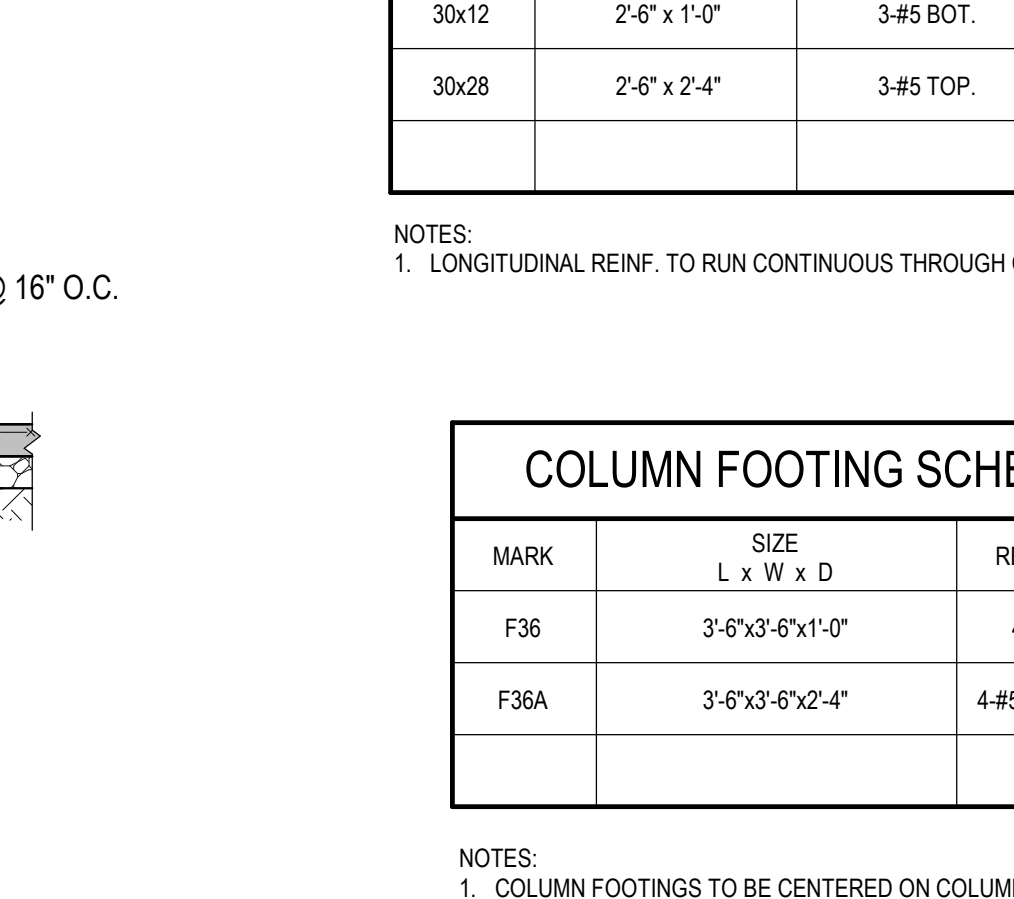
5 SECTION
S101 1/2" = 1'-0"



6 SECTION
S101 1/2" = 1'-0"



7 SECTION
S101 1/2" = 1'-0"



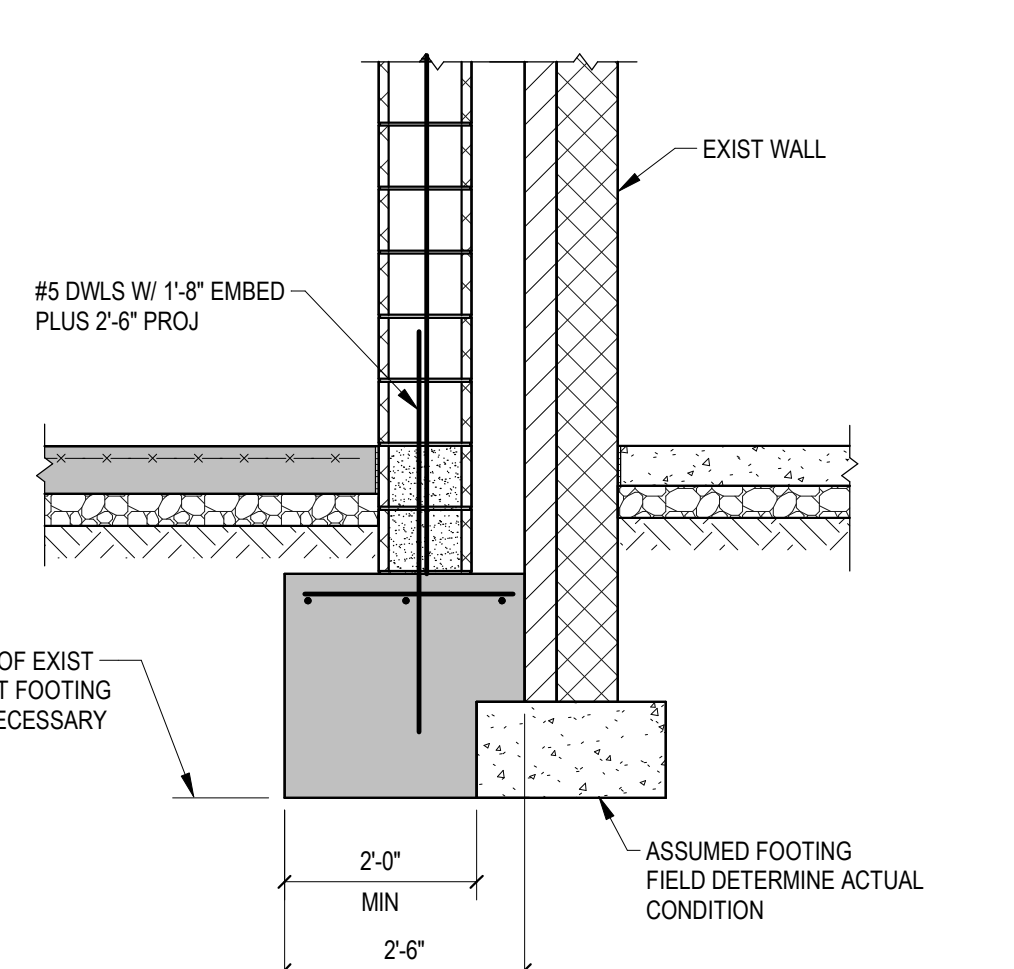
11 SECTION
S101 1/2" = 1'-0"

CONTINUOUS FOOTING SCHEDULE			
MARK	SIZE W x D	LONGITUDINAL REINFORCEMENT	TRANSVERSE REINFORCEMENT
24x12	2'-0" x 1'-0"	3#4S BOT.	#4@48 BOT.
24x28	2'-0" x 2'-4"	3#4S TOP.	#4@48 TOP.
30x12	2'-6" x 1'-0"	3#4S BOT.	#4@24 BOT.
30x28	2'-6" x 2'-4"	3#4S TOP.	#4@24 TOP.

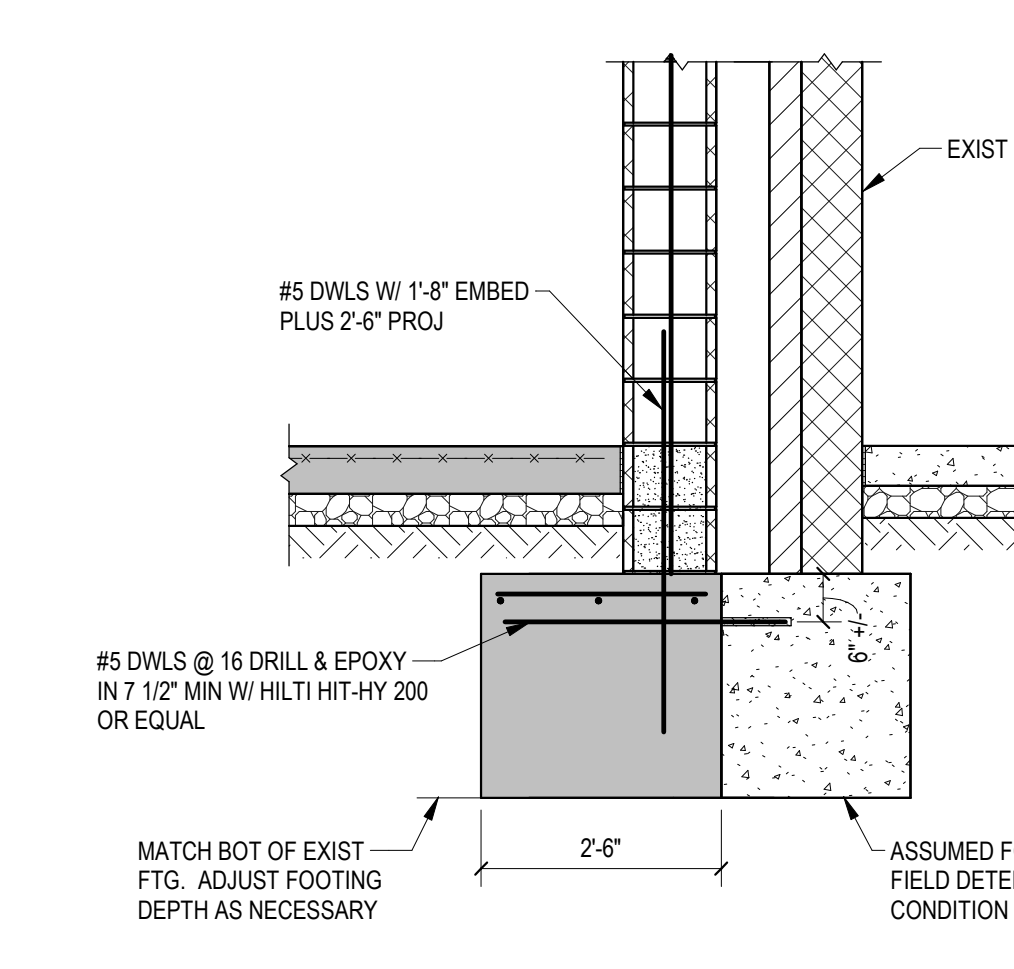
NOTES:
1. LONGITUDINAL REIN. TO RUN CONTINUOUS THROUGH COL. FOOTINGS.

COLUMN FOOTING SCHEDULE		
MARK	SIZE L x W x D	REINFORCEMENT
F36	3'-6"x3'-6"x1'-0"	4#4S BOT. E.W.
F36A	3'-6"x2'-6"x2'-4"	4#4S TOP & BOT. E.W.

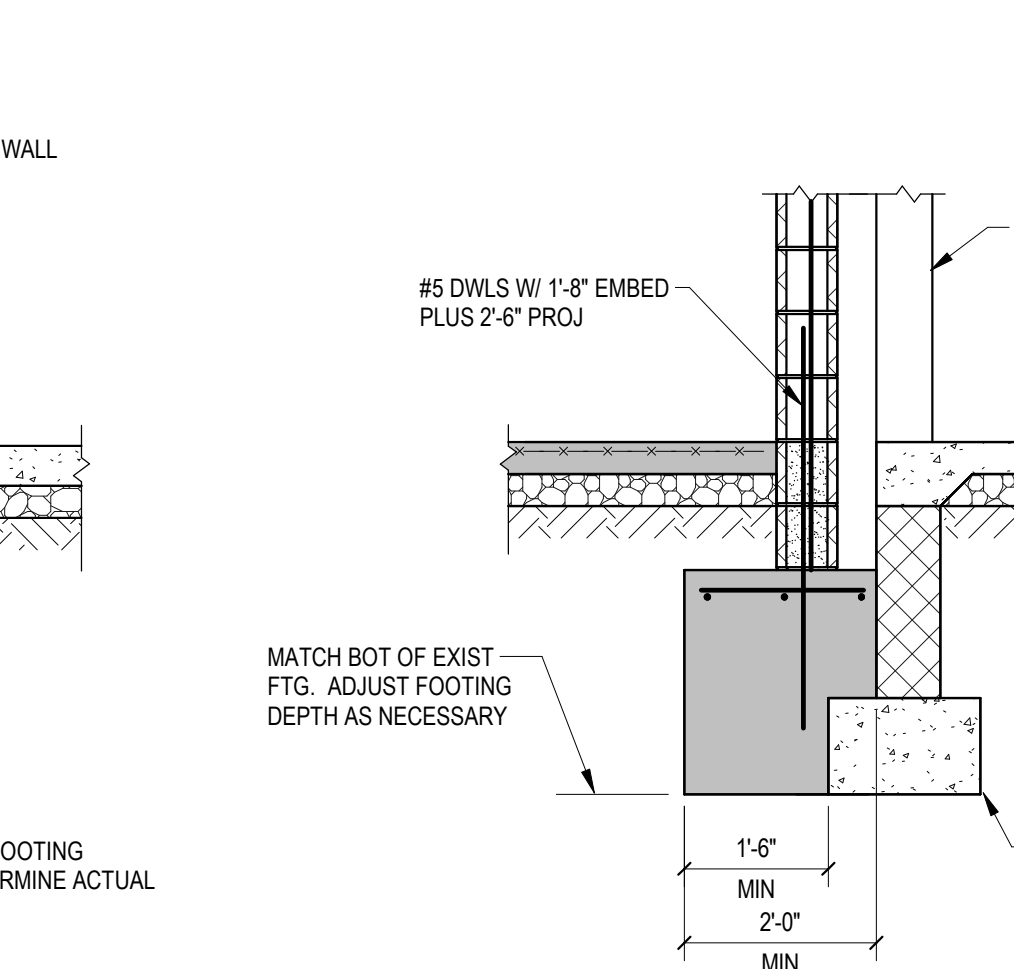
NOTES:
1. COLUMN FOOTINGS TO BE CENTERED ON COLUMN U.O.



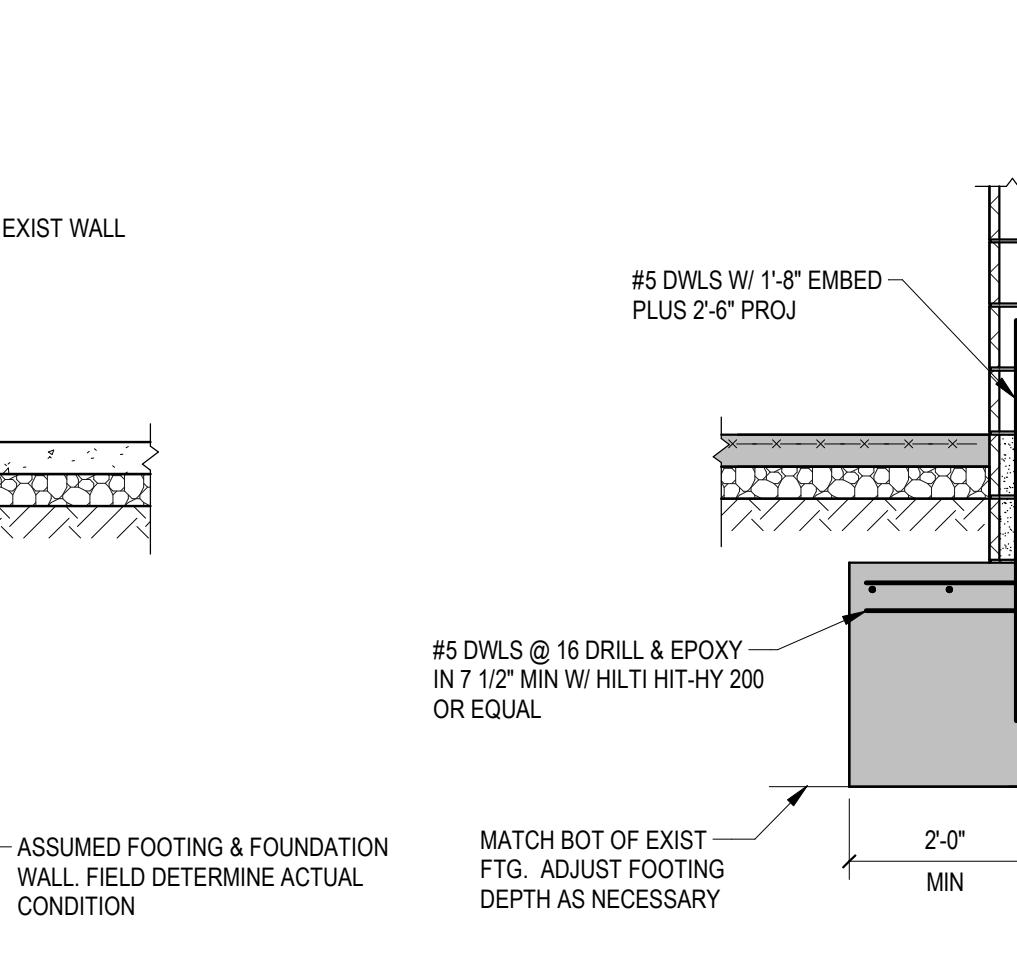
8 SECTION
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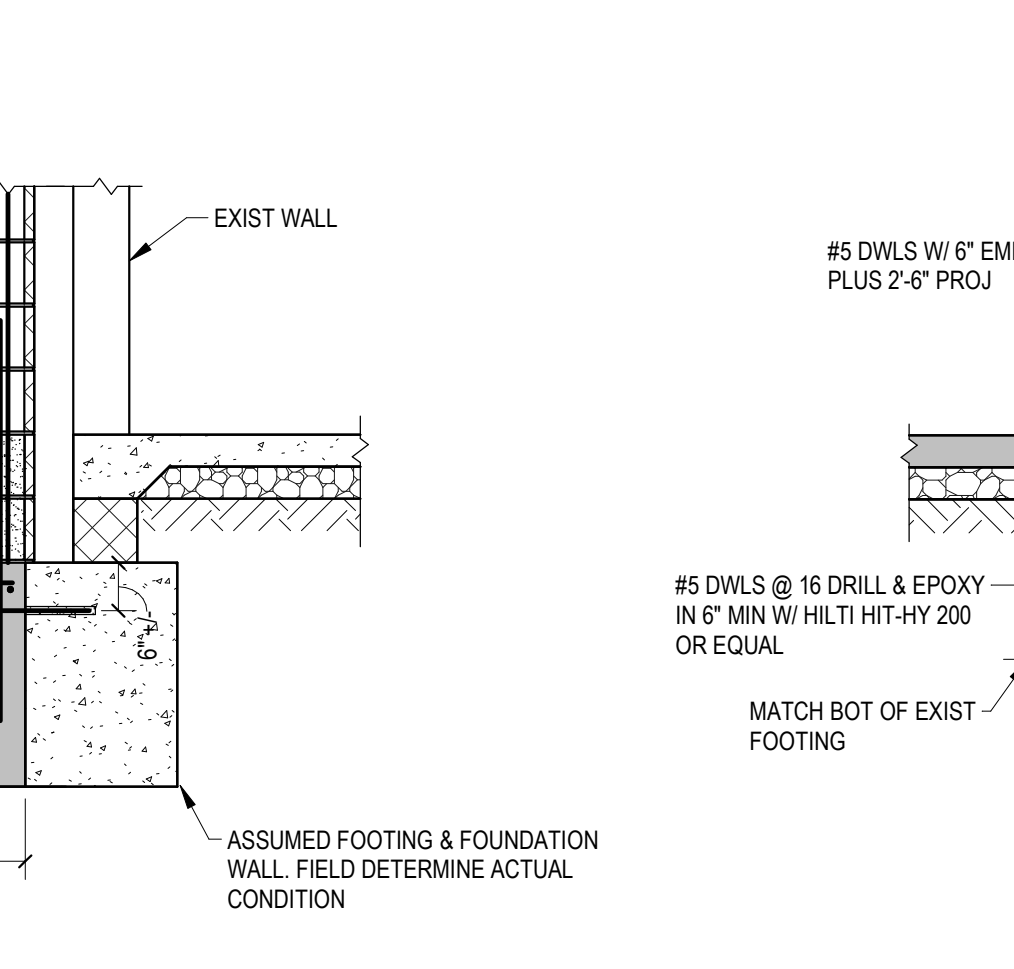
9A SECTION
S101 1/2" = 1'-0"



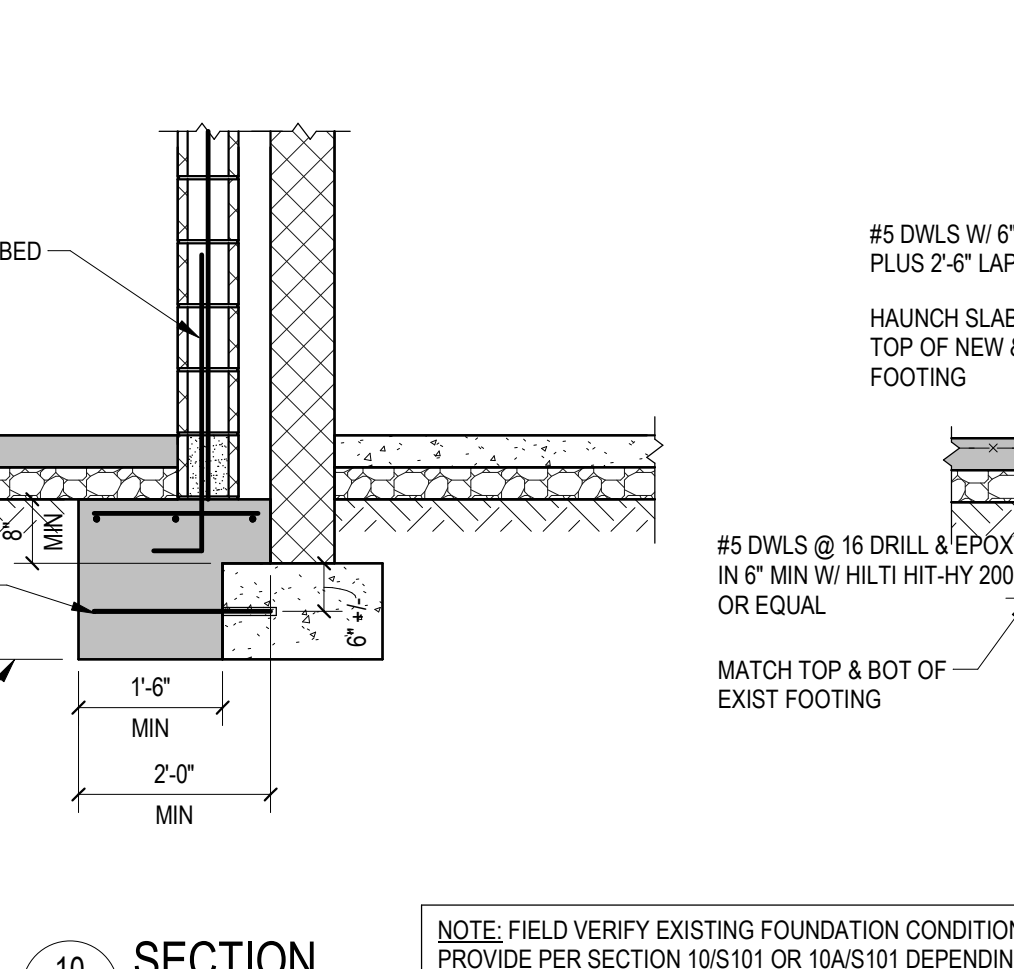
9 SECTION
S101 1/2" = 1'-0"



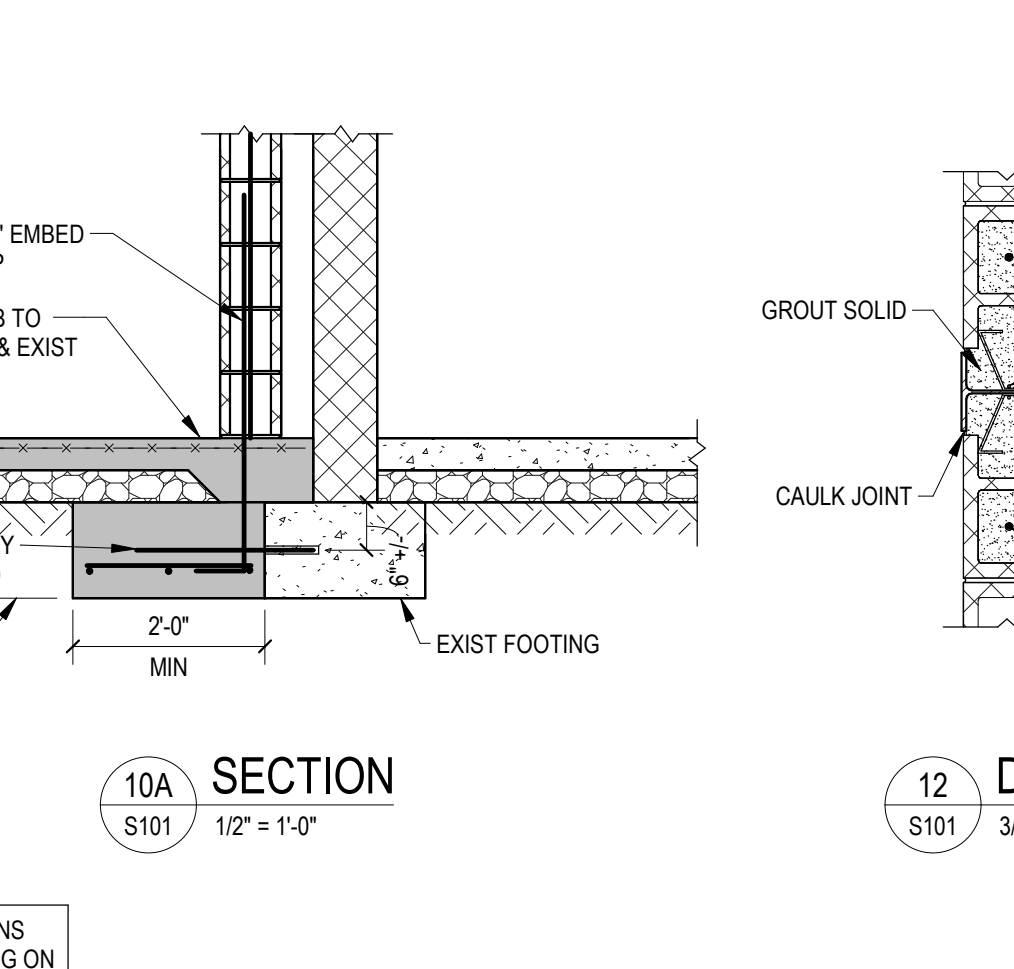
11A SECTION
S101 1/2" = 1'-0"



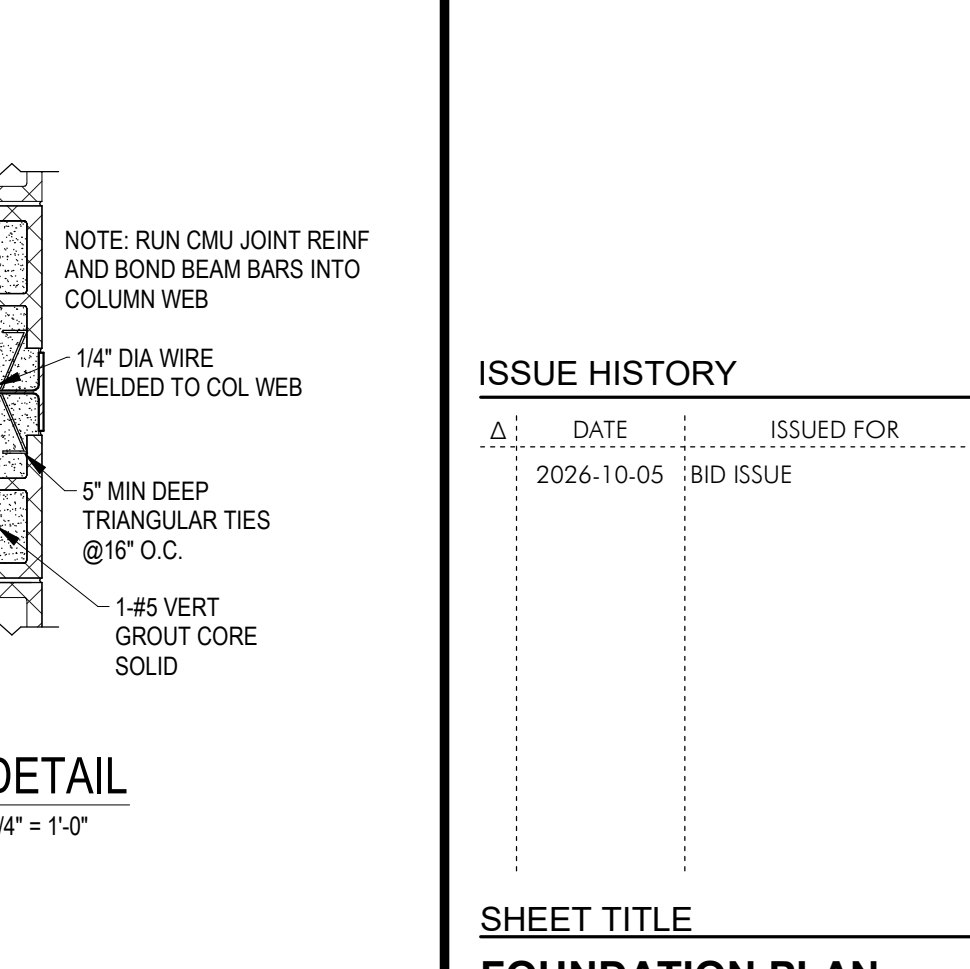
12A SECTION
S101 1/2" = 1'-0"



13 SECTION
S101 1/2" = 1'-0"



14A SECTION
S101 1/2" = 1'-0"



12 DETAIL
S101 3/4" = 1'-0"

ISSUE HISTORY

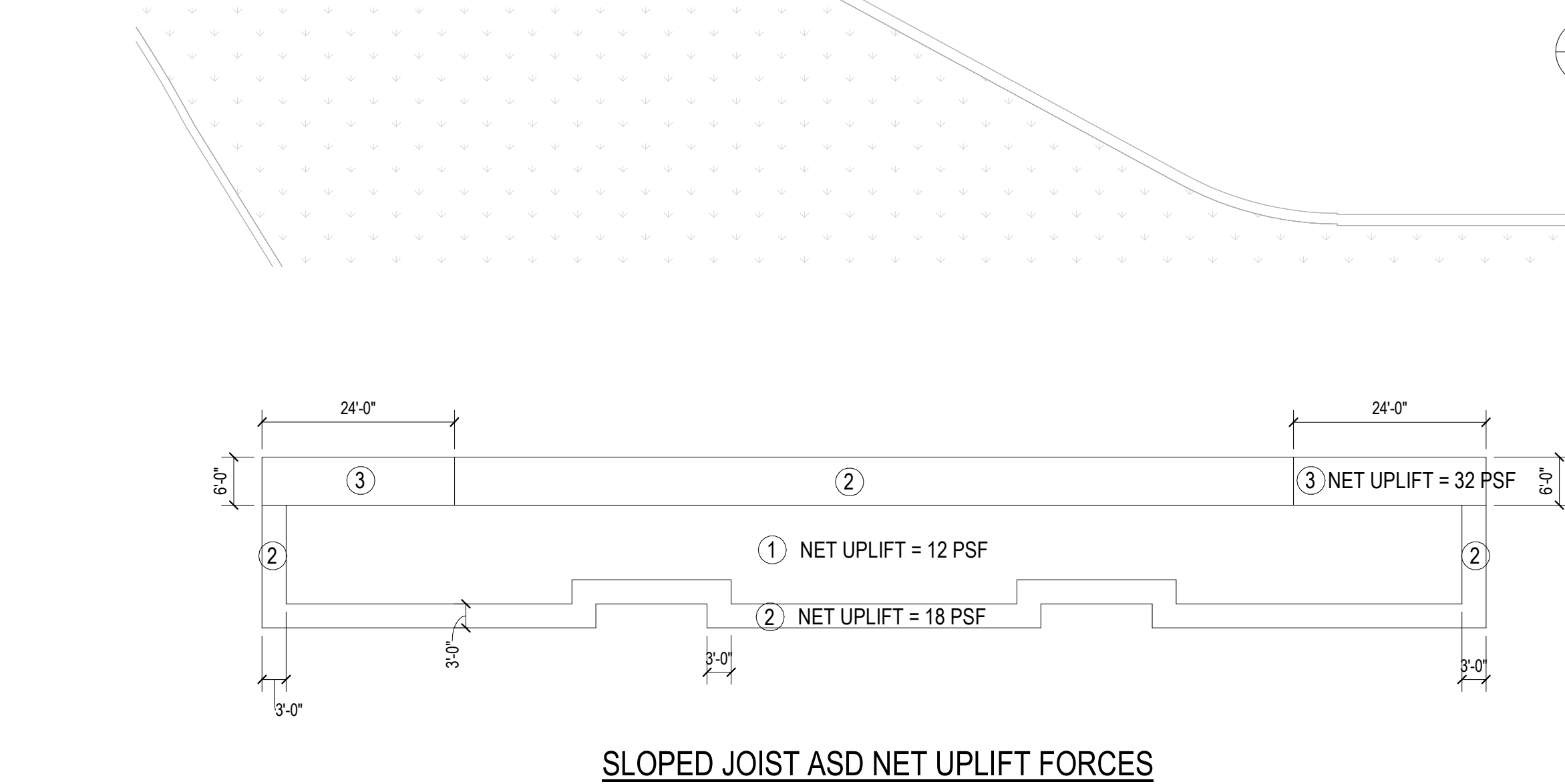
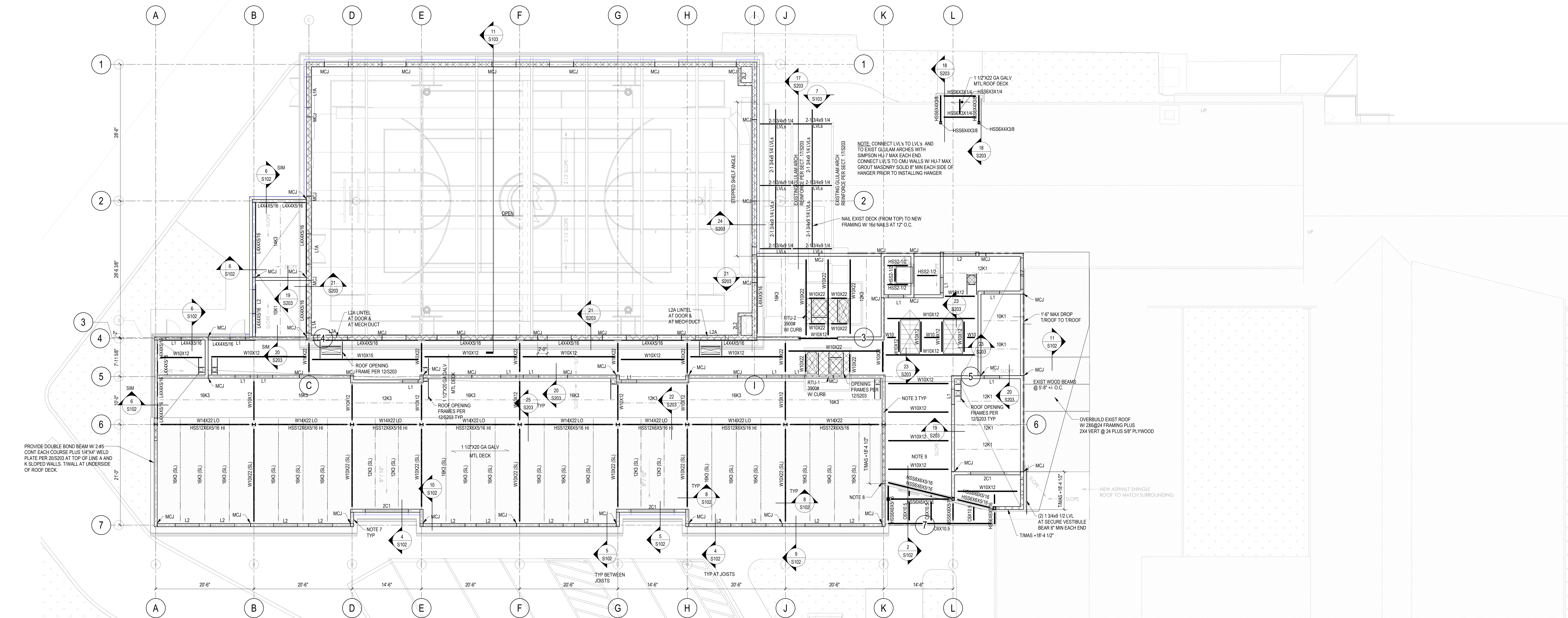
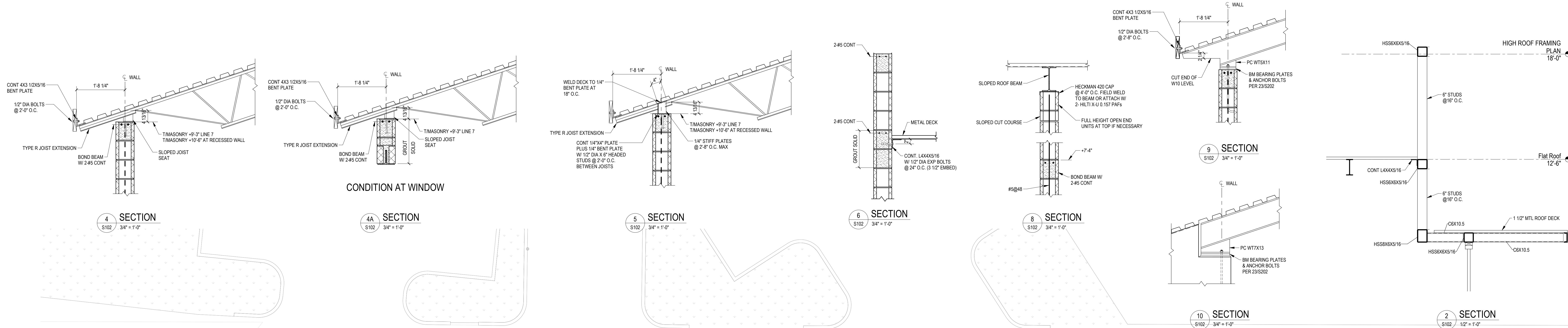
A	DATE	ISSUED FOR
1	2024-10-05	RD ISSUE

SHEET TITLE

FOUNDATION PLAN

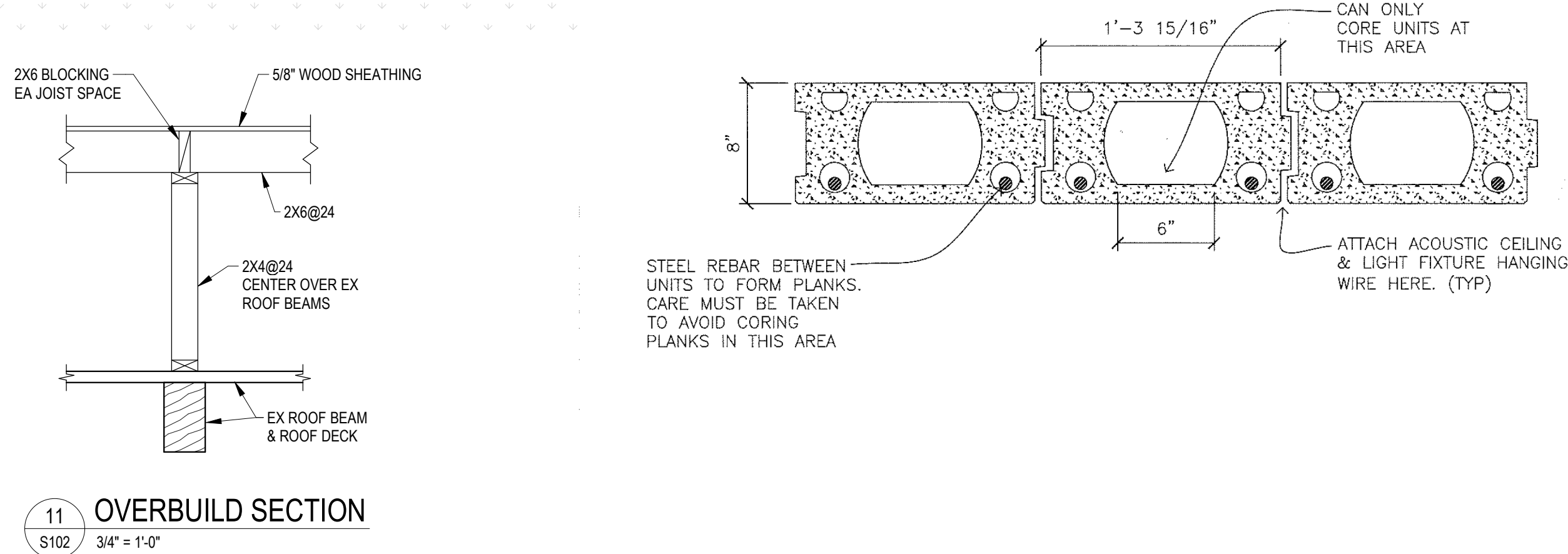
DRAWING NUMBER

S101



- 1 MAIN ROOF FRAMING PLAN
S102 3/4" = 1'-0"
- NOTES:
1. PROVIDE BOND BEAMS WITH 2#5 CONT. + CORNER BARS AT TOP OF ALL 6" AND 12" CMU WALLS.
2. PROVIDE 1" EXPANSION JOINT BETWEEN NEW AND EXISTING BUILDINGS.
3. BEAM BEARING PLATES PER DETAILS 23S202 TO 23S202 TYP.
4. PROVIDE JOIST BEARING PLATES PER 19S203 TYP.
5. W10 INDICATES W10X12.
6. HSS 12 INDICATES HSS 2 1/2X12 1/2X16. PROVIDE JOIST BEARING PLATES AT MASONRY WALLS.
7. PROVIDE 1/4"X4" WELD PLATE PER 20S203 AT TOP OF LINE D, E, G, & H SLOPED RETURN WALLS. TOP OF WALL AT UNDERSIDE OF ROOF DECK.
8. PROVIDE BEAM BEARING PLATES PER 23S202 EACH TUBE POCKETING INTO CMU WALL (3 TYP).
9. BRACE TOP OF MASONRY PARTITIONS AT ENTRY PER 22S202.
10. PROVIDE LINTELS OVER DOOR AND WINDOW MASONRY OPENINGS AS INDICATED ON PLAN. PROVIDE LINTELS OVER DUCT AND OTHER MASONRY OPENINGS PER DETAILS 26S202 AND 26A5202.

LINTEL SCHEDULE		
MARK	SIZE	REMARKS
L1	(2) LSK3 12X3/16	6" CMU
L1A	(3) LSK3 12X3/16	12" CMU
L2	(2) LSK3 12X3/16	6" CMU
L2A	(3) LSK3 12X3/16	12" CMU
2C1	(2) CRX11.5 + 3/8" EXTENDED BOT PLATE	MATCH CMU THICKNESS
2C2	(2) CRX10.5 + 1/4" BOT PLATE	MATCH CMU THICKNESS
2C2A	(2) CRX10.5 + 3/8" EXTENDED BOT PLATE	MATCH CMU THICKNESS



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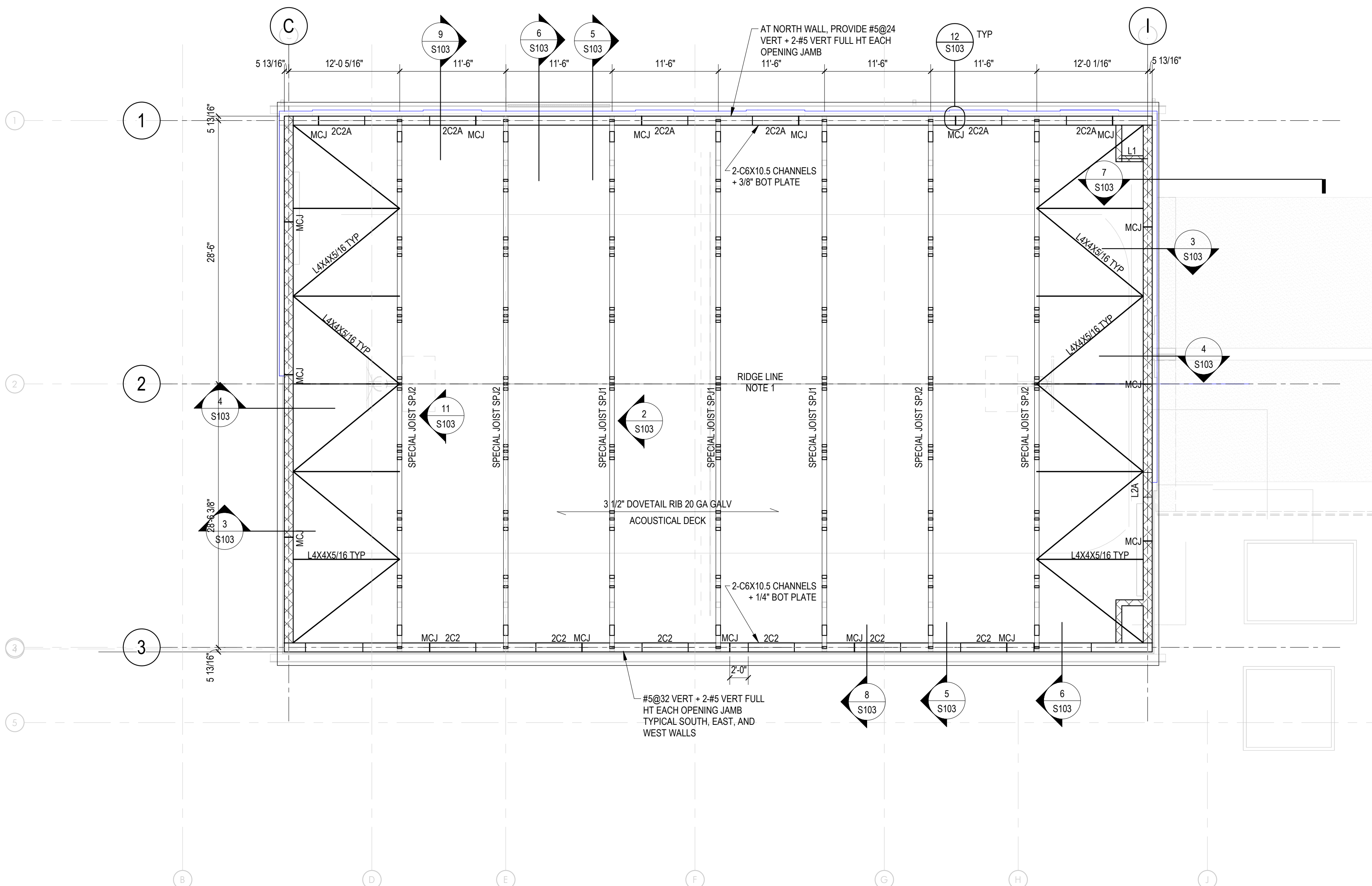
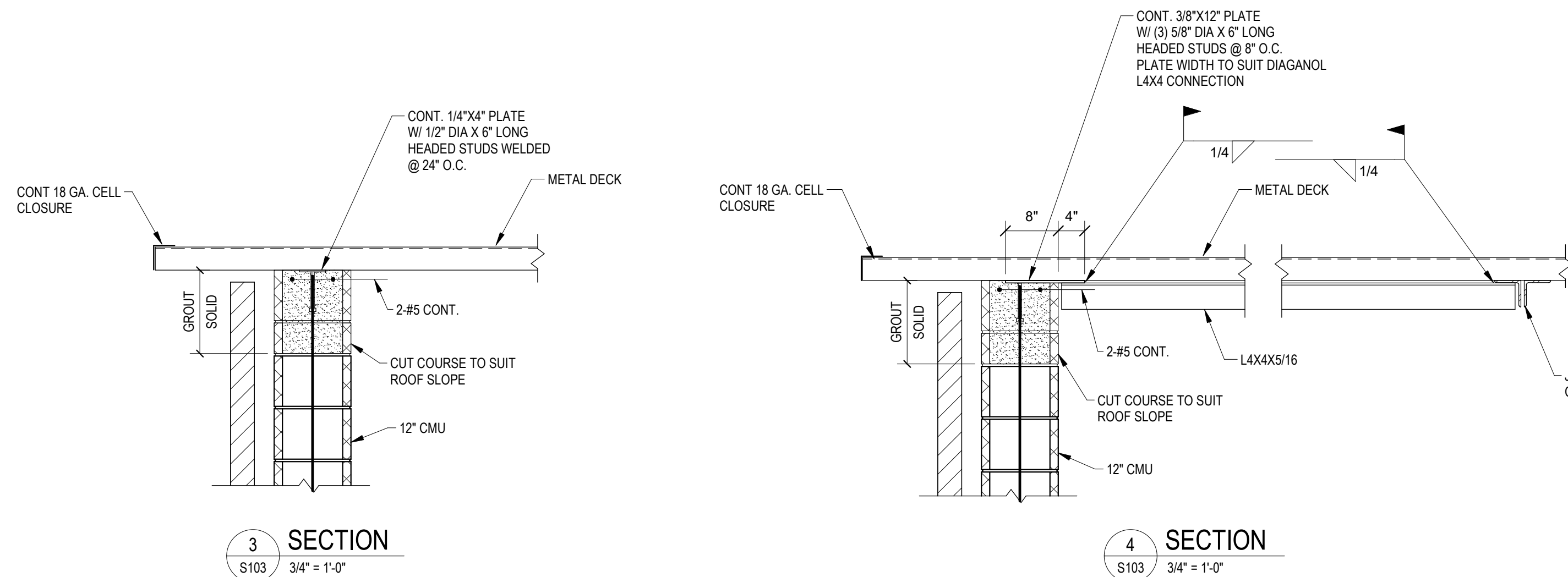
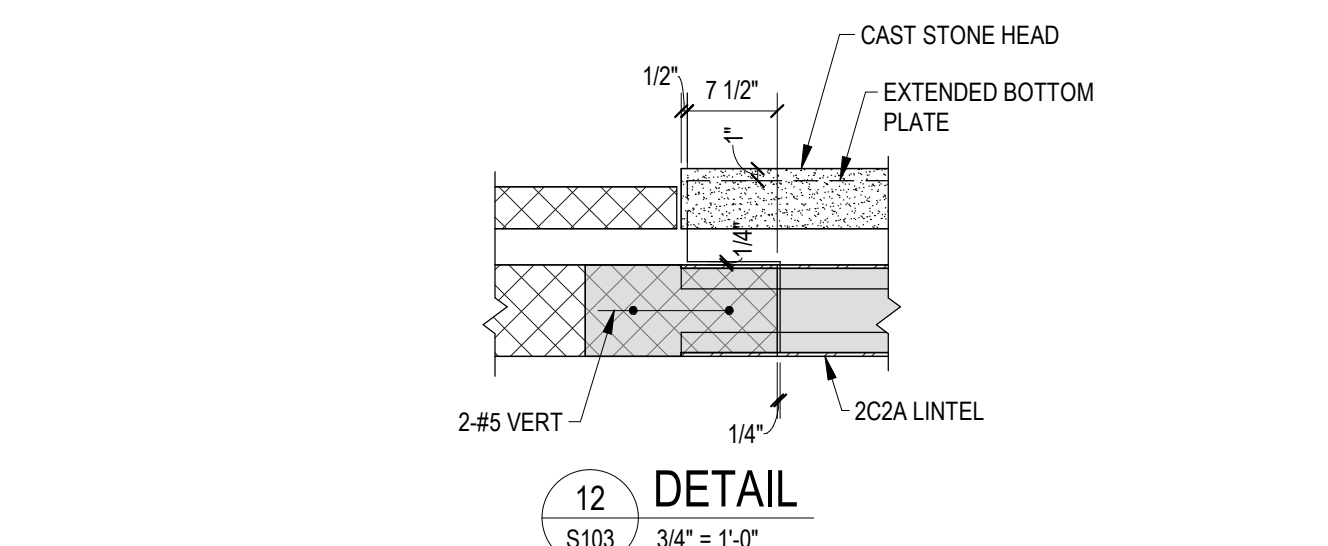
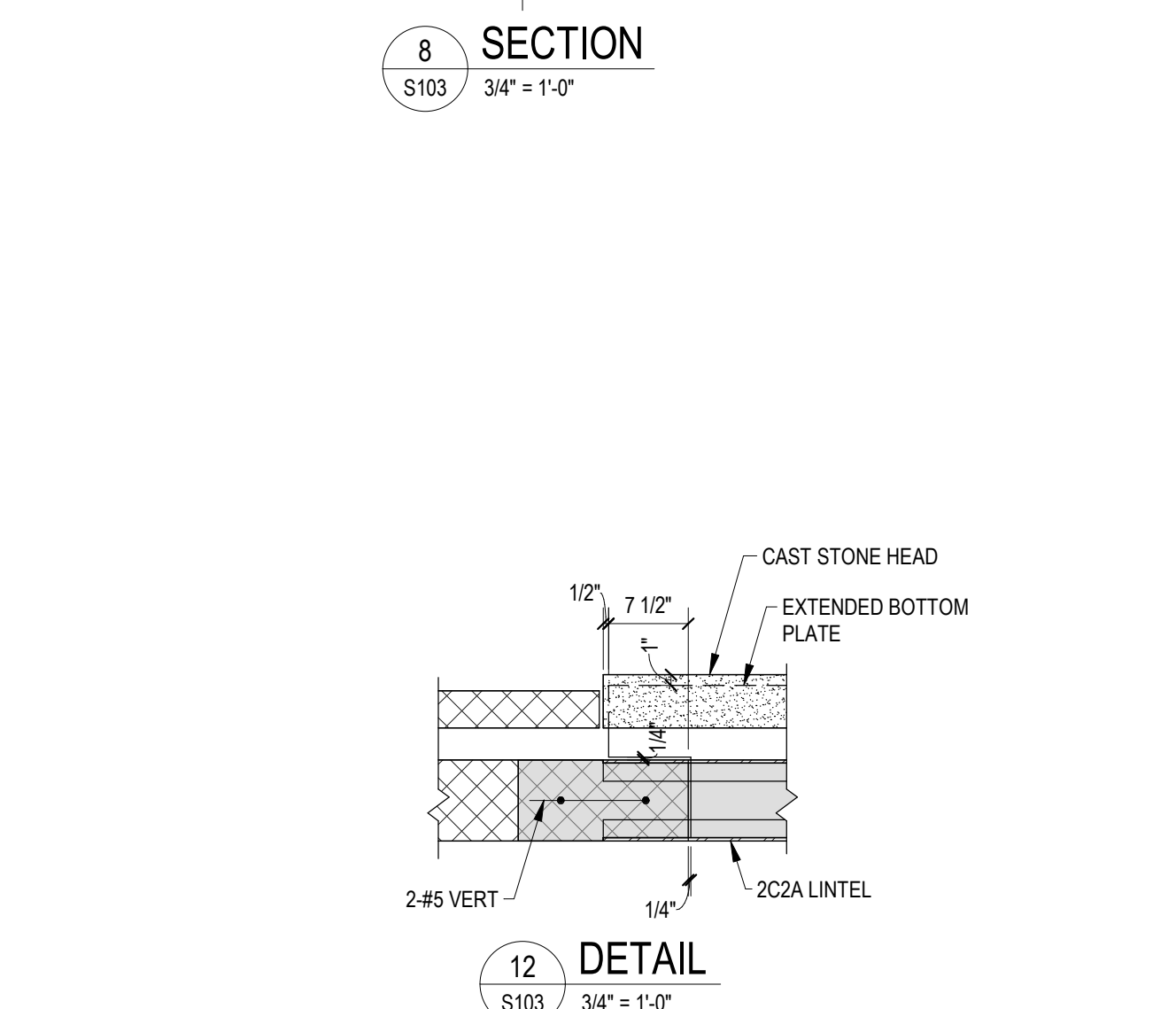
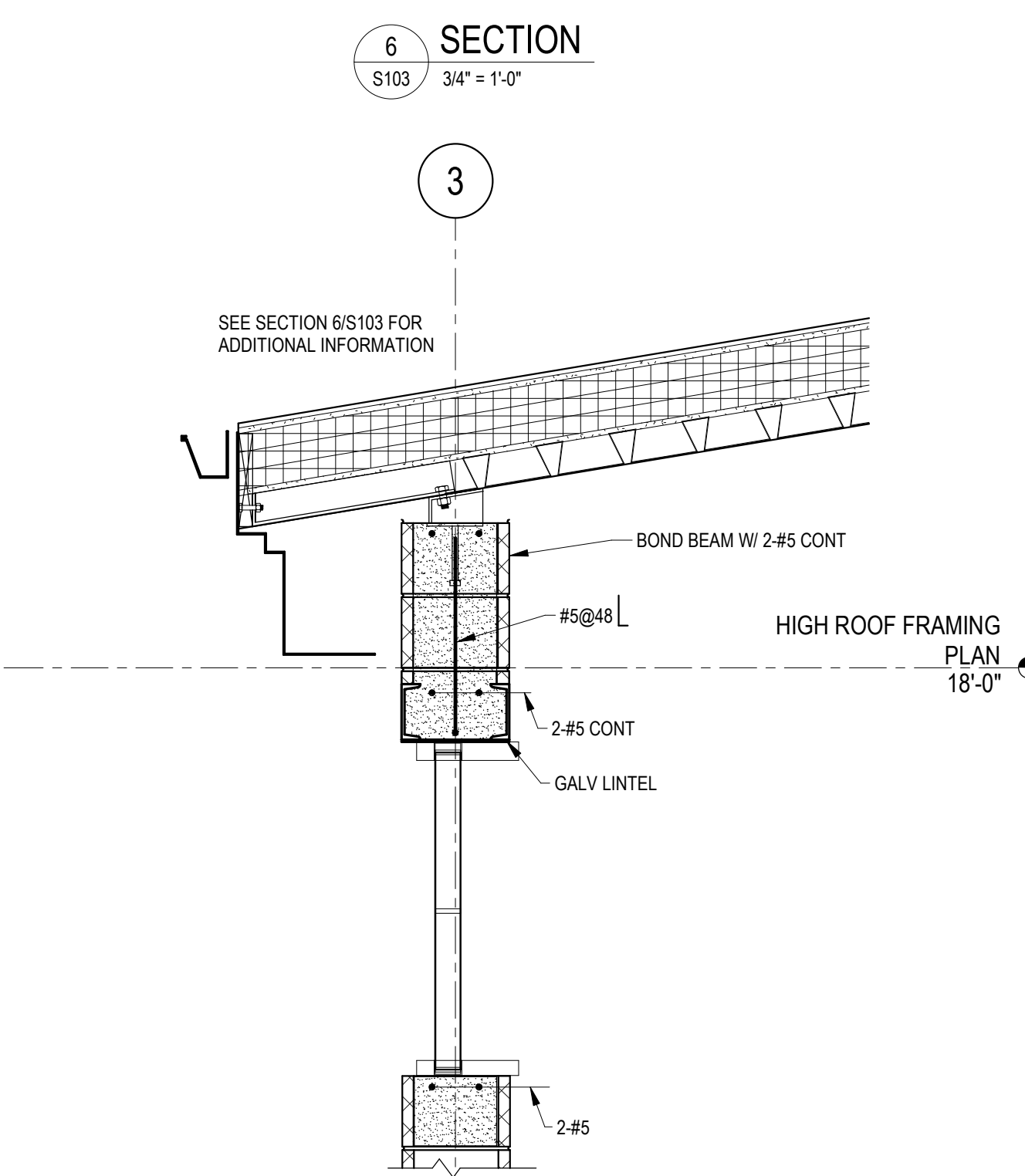
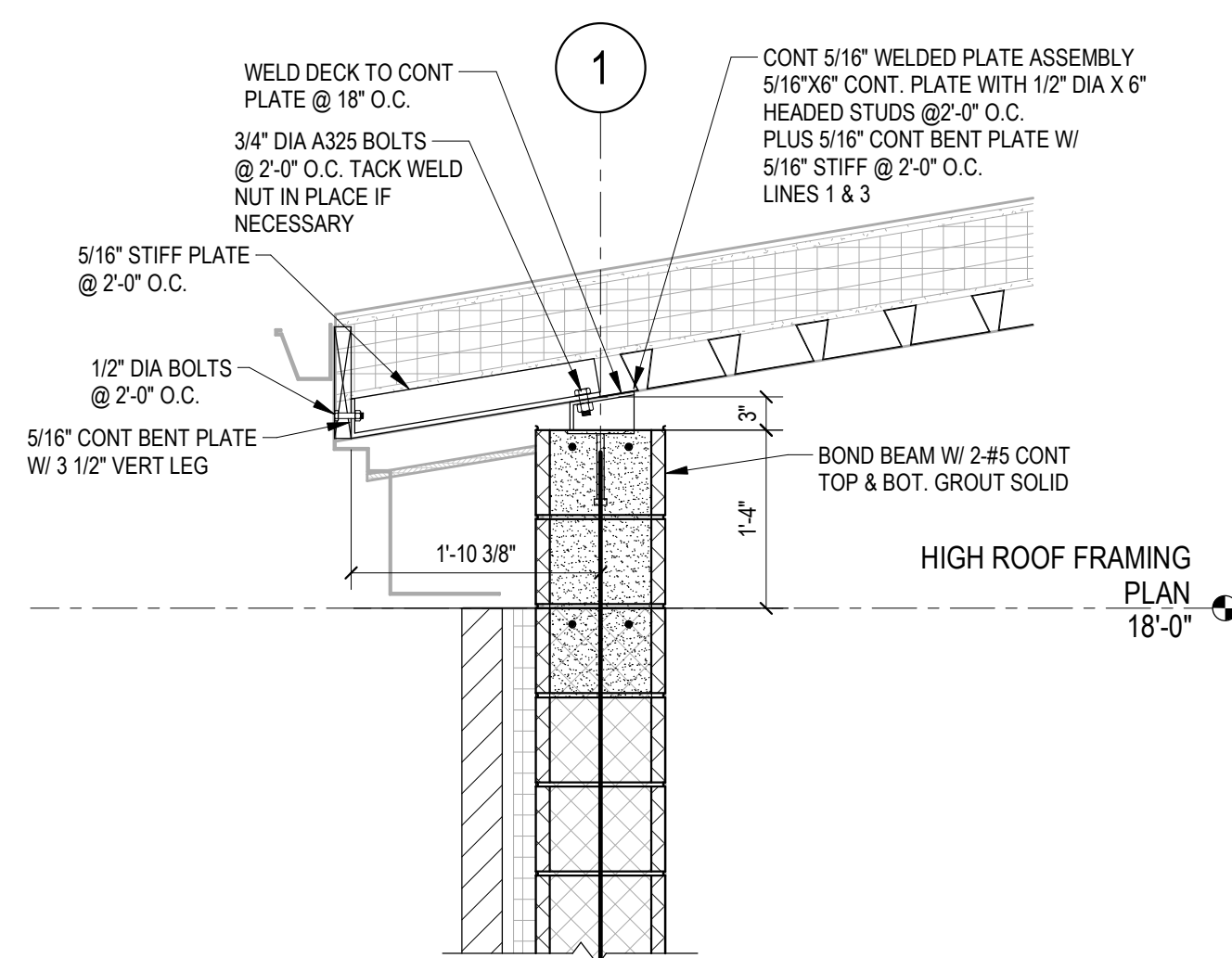
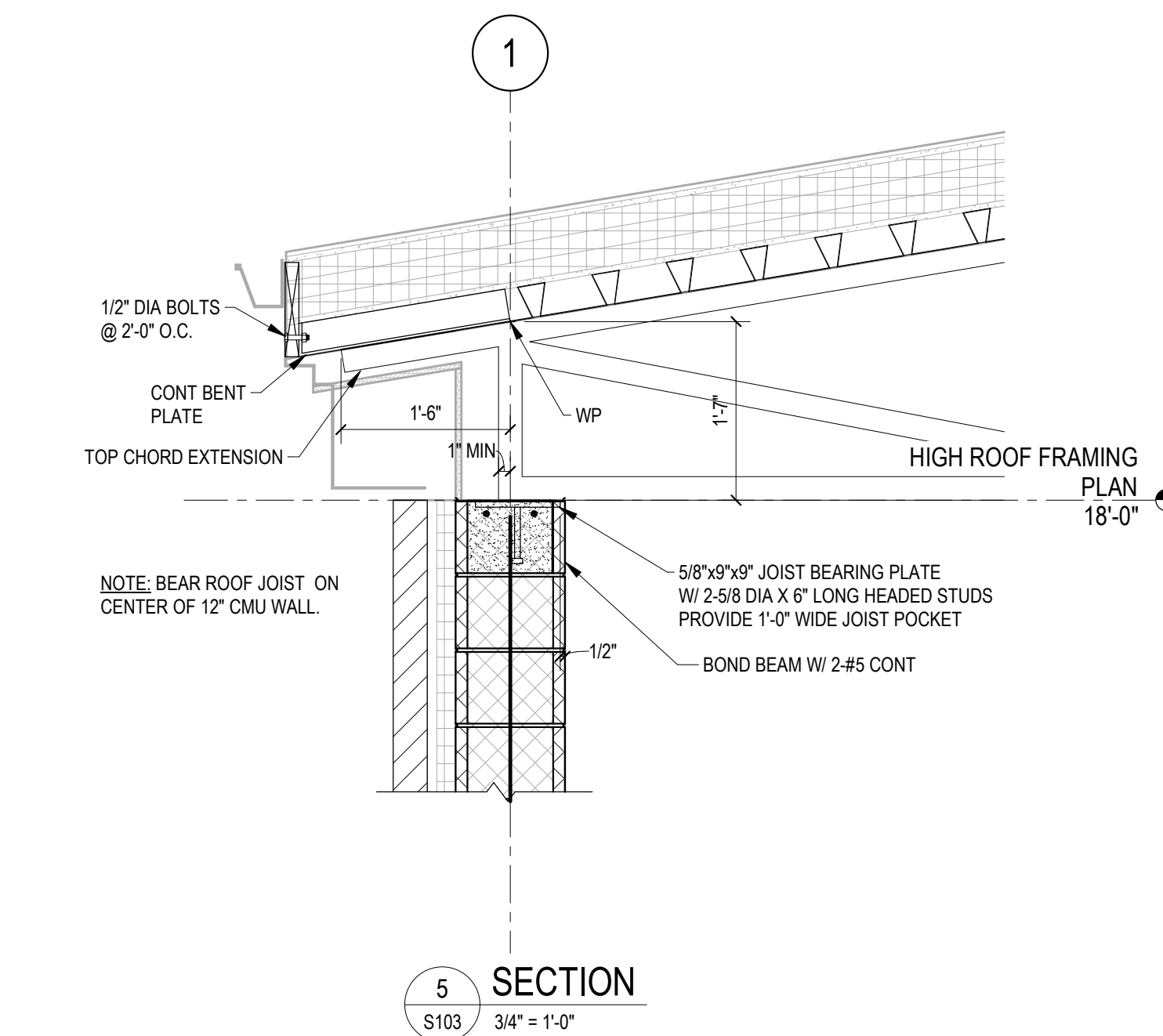
DATE	ISSUED FOR
2026-10-05	RD ISSUE

SHEET TITLE

HIGH ROOF FRAMING
PLAN

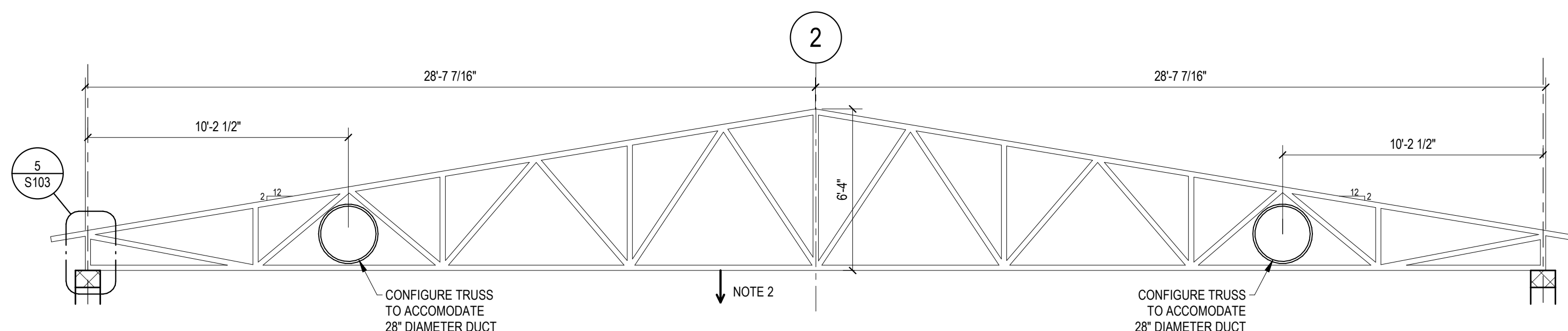
DRAWING NUMBER

S103



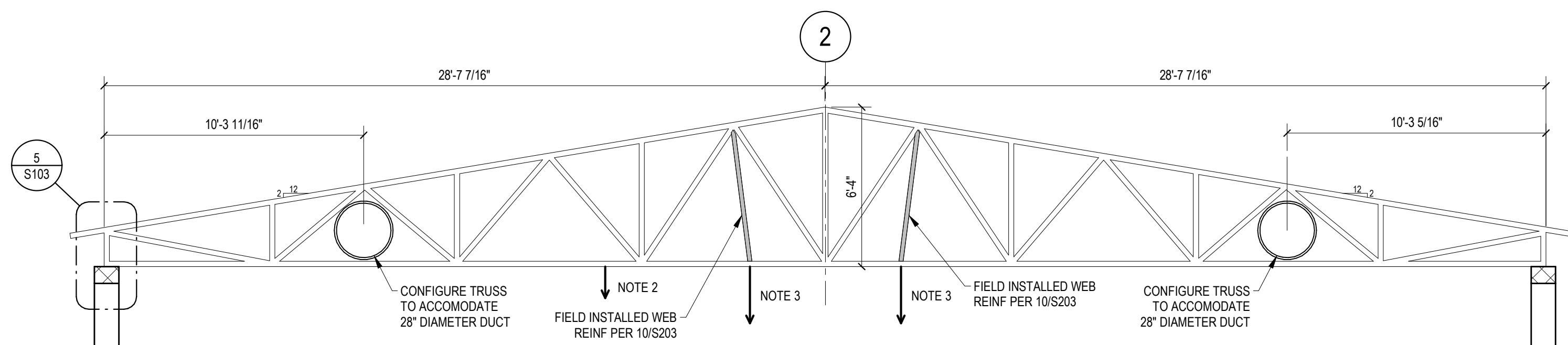
HIGH ROOF FRAMING PLAN

NOTE:
1. PROVIDE 18 GAX18\"/>



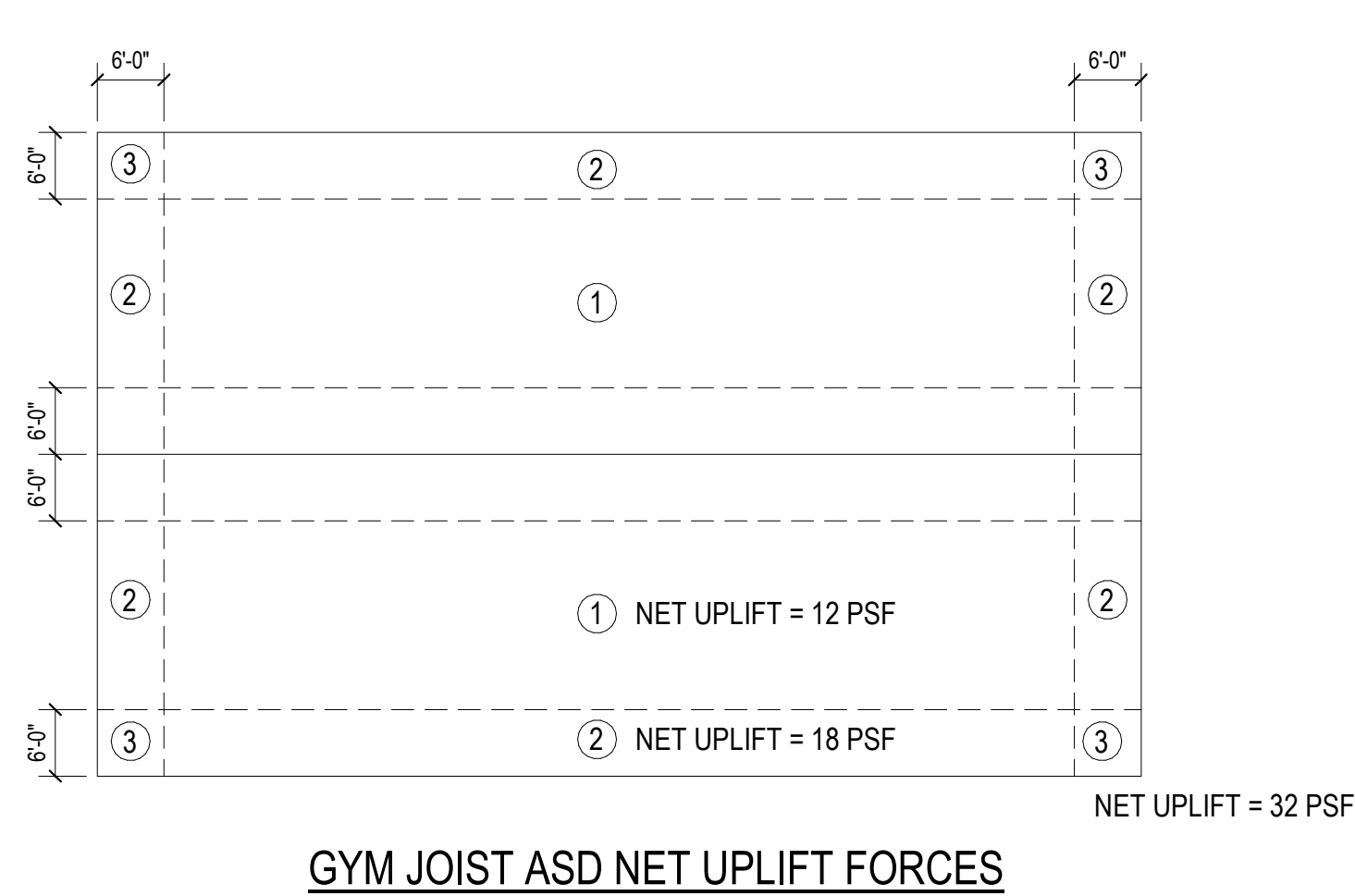
SPECIAL JOIST SPJ1 PROFILE

NOTES:
1. DESIGN JOIST FOR TOP CHORD DL=234 PLF TOP CHORD LL=360 PLF
BOTTOM CHORD DL=102 PLF AND BOTTOM CHORD LL=102 PLF
2. DESIGN JOIST FOR CONCURRENT CONCENTRATED BOTTOM CHORD
LIVE OF 250 POUNDS APPLIED ANYWHERE ALONG BOTTOM CHORD.

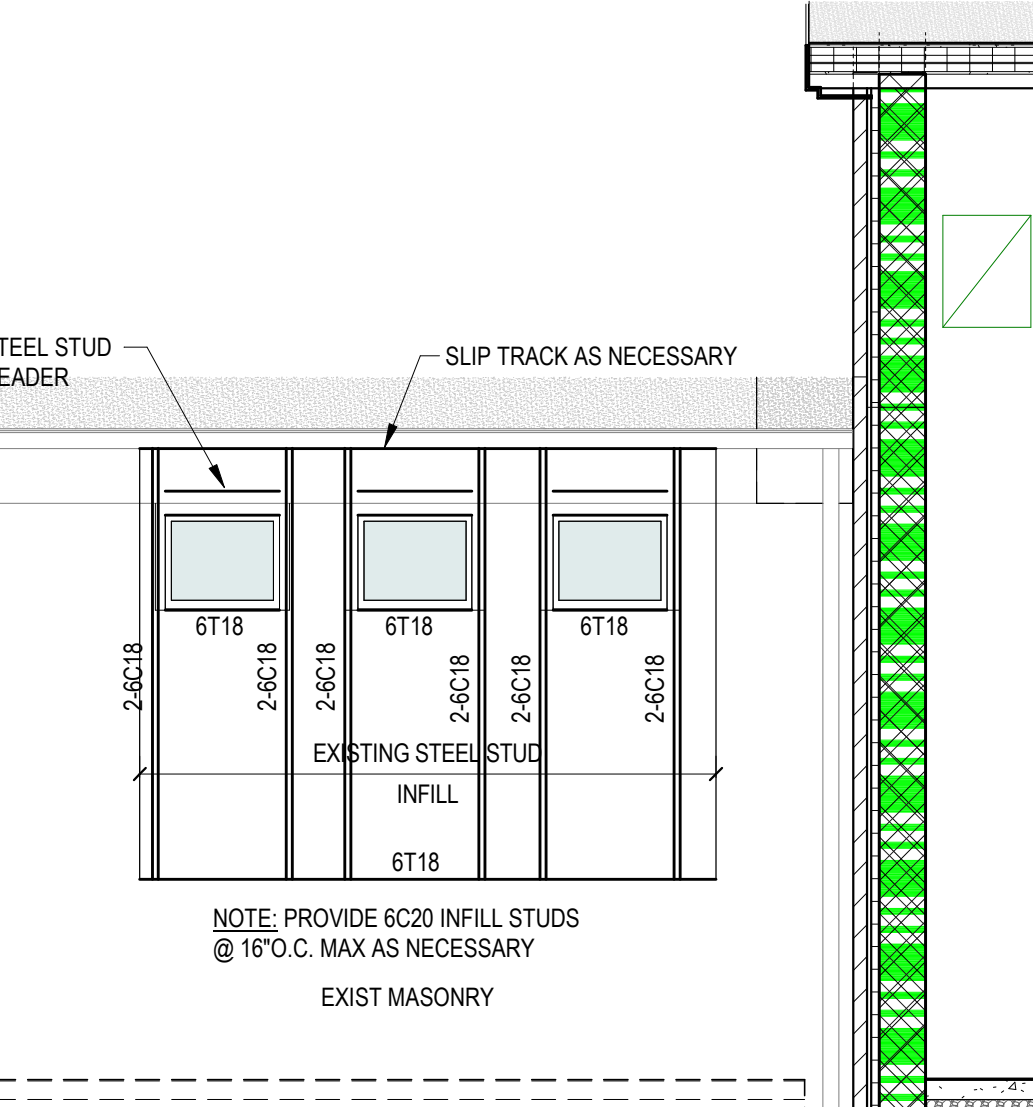
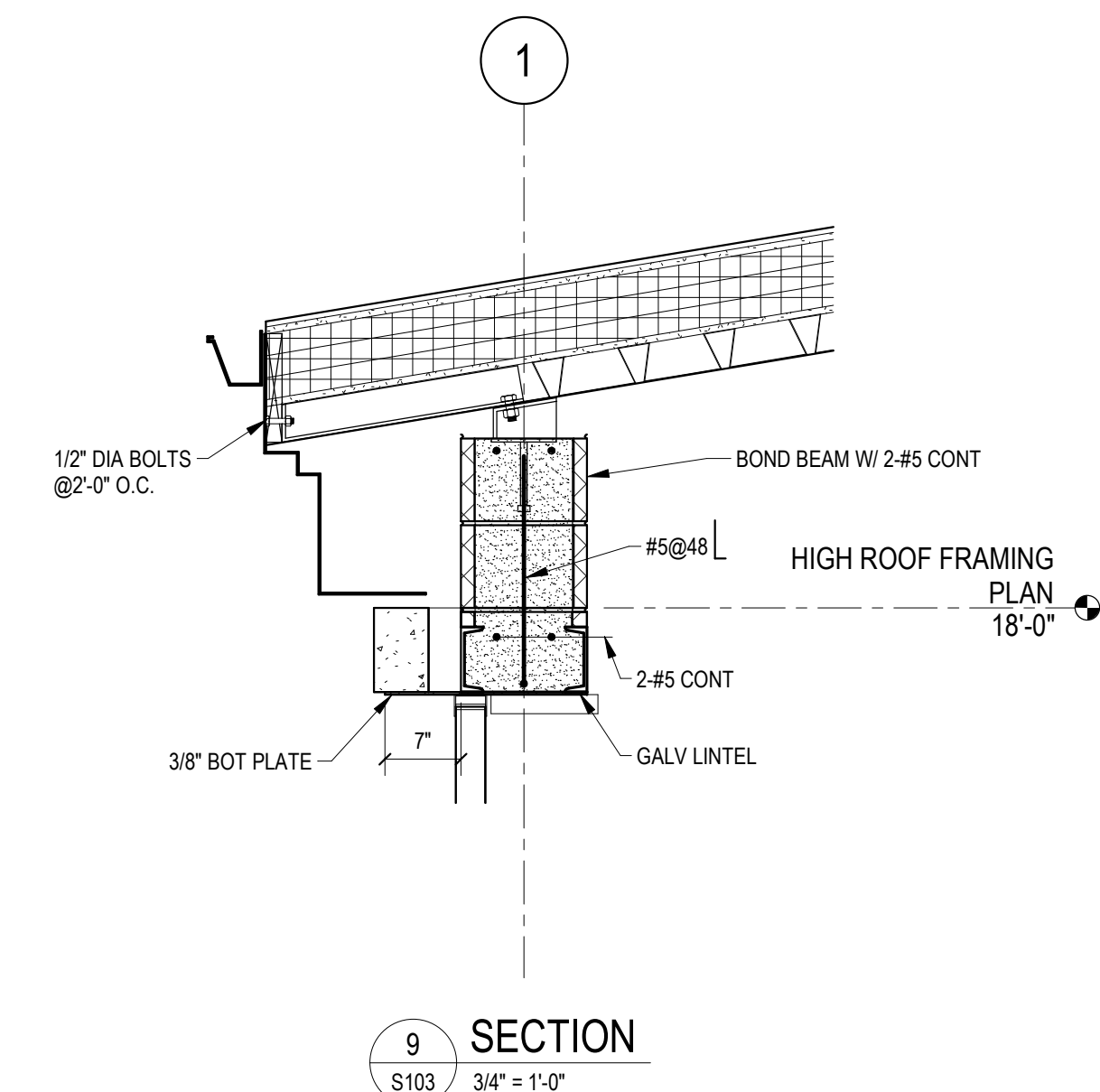


SPECIAL JOIST SPJ2 PROFILE

NOTES:
1. DESIGN JOIST FOR TOP CHORD DL=234 PLF TOP CHORD LL=360 PLF
BOTTOM CHORD DL=102 PLF AND BOTTOM CHORD LL=102 PLF
2. DESIGN JOIST FOR CONCURRENT CONCENTRATED BOTTOM CHORD
LIVE OF 250 POUNDS APPLIED ANYWHERE ALONG BOTTOM CHORD
3. DESIGN JOIST FOR ADDITIONAL CONCENTRATED LIVE LOAD OF 1000 POUNDS
FROM BASKETBALL HOOP: FIELD REINFORCE WEB PER 10\"/>



GYM JOIST ASD NET UPLIFT FORCES



ELEVATION

1/4\"/>

GENERAL NOTES

GENERAL: ALL WORK TO CONFORM TO 2021 INTERNATIONAL BUILDING CODE AND ALL OTHER GOVERNING FEDERAL, STATE AND LOCAL REGULATIONS.

VERIFY AND/OR ESTABLISH ALL EXISTING CONDITIONS AND DIMENSIONS AT THE SITE BEFORE SUBMITTING SHOP DRAWINGS, ORDERING ANY MATERIAL OR COMMENCEMENT OF ANY WORK.

SHOP DRAWINGS FOR CONCRETE AND MASONRY REINFORCING, STRUCTURAL STEEL, STEEL JOISTS, STEEL DECK, AND EXTERIOR AND LOAD BEARING STRUCTURAL STEEL STUDS TO BE SUBMITTED AND REVIEWED PRIOR TO START OF FABRICATION.

CONTACT PROPER AUTHORITIES TO LOCATE EXISTING UNDERGROUND UTILITIES PRIOR TO EXCAVATION.

EXISTING BUILDING INFORMATION SHOWN WAS OBTAINED FROM LIMITED EXISTING BUILDING DRAWINGS FROM A PREVIOUS RENOVATION AND LIMITED FIELD VERIFICATION OF EXISTING FRAMING. CONTRACTOR SHALL CONTACT OWNER TO OBTAIN ACCESS TO AND REVIEW EXISTING BUILDING DRAWINGS. CONTACT A/E FOR DIRECTION IF EXISTING CONDITIONS DIFFER FROM AS SHOWN.

REMOVE AND RELOCATE ALL EXISTING UTILITIES, DUCTS, CONDUITS, ETC. THAT INTERFERE WITH INSTALLATION OF NEW FRAMING ELEMENTS.

SPECIAL STRUCTURAL INSPECTIONS: SPECIAL TESTING AND INSPECTIONS OF THE FABRICATION, INSTALLATION, ERECTION, AND PLACEMENT OF THE FOLLOWING BUILDING ELEMENTS ARE TO BE PROVIDED IN ACCORDANCE WITH CHAPTER 17 OF THE BUILDING CODE. SEE NOTES AND SPECIFICATIONS FOR ADDITIONAL TESTING AND INSPECTIONS REQUIRED.

1. PREPARED FILL: SITE PREPARATION, FILL PLACEMENT, AND EVALUATION OF IN PLACE DENSITY.
2. CONCRETE CONSTRUCTION: MATERIALS, REINFORCING STEEL, FOUNDATION SUBGRADE, AND CONCRETING OPERATIONS.
3. MASONRY CONSTRUCTION: MATERIALS, STRENGTH, AND CONSTRUCTION OPERATIONS.
4. STEEL CONSTRUCTION: FABRICATION, MATERIALS, AND ERECTION, INCLUDING BOLTING AND WELDING.

NO PROVISIONS FOR FUTURE EXPANSION HAVE BEEN INCORPORATED IN THIS STRUCTURE.

DESIGN GRAVITY LOADS:

ROOF DEAD LOAD	20 PSF
ROOF LIVE LOAD	30 PSF MINIMUM
DESIGN SNOW LOADS:	
GROUND SNOW LOAD	25 PSF
SNOW IMPORTANCE FACTOR	1.10
THERMAL FACTOR	1.1
EXPOSURE FACTOR	1.0

DESIGN WIND LOADS:

WIND SPEED	120 MPH
MEAN BUILDING HEIGHT	24 FT
WIND EXPOSURE CATEGORY	C
SEE TABLE FOR MINIMUM ULTIMATE WIND PRESSURES FOR COMPONENTS AND CLADDING	

DESIGN SEISMIC LOADS:

SPECTRAL ACCELERATIONS	Ss = 0.20 S1 = 0.05, SDS = 0.213, SD1 = 0.079
OCCUPANCY CATEGORY	III
SEISMIC DESIGN CATEGORY	B
SITE CLASS	0
SEISMIC IMPORTANCE FACTOR	1.25
RESPONSE MODIFICATION FACTOR	3.5

FOUNDATIONS: FOUNDATION WORK HAS BEEN DESIGNED AND ELEVATIONS ESTABLISHED ON THE BASIS OF THE REPORT OF WHITSTONE ASSOCIATES, INC. DATED JANUARY 7, 2026 WRITTEN FOR THE USE OF THE OWNER, ARCHITECT AND ENGINEER. FOOTINGS HAVE BEEN DESIGNED TO BEAR ON UNDISTURBED RESIDUAL SOILS OR COMPACTED STRUCTURAL FILL CAPABLE OF SAFELY SUPPORTING A NET PRESSURE OF 3000 PSF. IF FOUNDATION CONDITIONS PROVE UNACCEPTABLE AT ELEVATIONS SHOWN, FOOTING SHALL BE CARRIED DEEPER OR OTHER REDESIGN OF FOUNDATIONS WILL BE REQUIRED AT THE DIRECTION OF THE ARCHITECT. BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 3'-0" BELOW FINISHED GRADE.

THE OWNER WILL EMPLOY A TESTING AGENCY TO PERFORM INSPECTIONS AND TESTING OF FOOTING BEARING ELEVATIONS WITH REGARD TO THE DESIGN SOIL BEARING PRESSURE NOTED ABOVE.

NOTIFY THE TESTING AGENCY WITHIN 24 HOURS WHEN EXCAVATION OF THE BOTTOM OF FOOTINGS HAVE BEEN COMPLETED. DO NOT POUR FOOTINGS UNTIL THE INSPECTION AGENCY HAS VERIFIED THE BEARING VALUE.

BOTTOM OF NEW FOOTINGS ADJACENT TO EXISTING FOUNDATIONS SHALL BE AT THE SAME ELEVATION AS THE EXISTING FOUNDATION UNLESS A 2:1 MAXIMUM SLOPE (HORIZONTAL TO VERTICAL) IS MAINTAINED BETWEEN BOTTOM OF FOOTINGS.

EARTHWORK: ALL EARTHWORK, INCLUDING EXCAVATIONS, FILLS, PROOFROLLING, TESTING, COMPACTION, ETC., TO BE PERFORMED IN ACCORDANCE WITH RECOMMENDATIONS CONTAINED IN SOILS REPORT PREPARED BY WHITSTONE ASSOCIATES, INC. DATED JANUARY 7, 2026 AND THE SPECIFICATIONS. ALL EARTHWORK, INCLUDING FOUNDATION BEARING, SHALL BE INSPECTED FOR CONFORMANCE WITH THE SOILS REPORT AND CONTRACT DOCUMENTS AS REQUIRED BY THE SPECIFICATIONS.

CONTROLLED COMPACTED FILL: ALL INSITU SOFT MATERIAL THAT CANNOT BE COMPACTED AND MATERIAL THAT CANNOT SUPPORT THE REQUIRED THICKNESSES OF CONTROLLED COMPACTED FILL AND/OR FOUNDATION LOADS WITHOUT DETRIMENTAL SETTLEMENT MUST BE REMOVED PRIOR TO PROOFROLLING.

AFTER STRIPPING AND GENERAL EXCAVATIONS, THE ENTIRE BUILDING AREA SHALL BE COMPACTED WITH A HEAVY SMOOTH VIBRATORY DRUM ROLLER (MINIMUM 10-TON STATIC WEIGHT) WITH A MINIMUM OF TWO PASSES IN PERPENDICULAR DIRECTIONS TO DETECT CAVITIES, SINKHOLES, SOFT SPOTS OR IRREGULARITIES IN THE SUBGRADE. THE SURFACE IS TO BE PROOFROLLED WITH A LOADED TANDUM AXLE TRUCK IN THE PRESENCE OF THE TESTING LABORATORY REPRESENTATIVE TO OBSERVE AND MAKE RECOMMENDATIONS DURING PROOFROLLING OPERATION.

AFTER PROOFROLLING AND ALL CORRECTIVE MEASURES AS REQUIRED BY THE TESTING LABORATORY ARE ACCOMPLISHED, GENERAL FILL WITHIN THE BUILDING SHALL BE PLACED PRIOR TO CONSTRUCTION OF FOUNDATIONS. ALL FILL WITHIN AND 5'-0" BEYOND THE BUILDING LINES SHALL BE MAINTAINED NOMINALLY AT OPTIMUM MOISTURE CONTENT, AND PLACED AND COMPACTED TO A MINIMUM OF 95% OF MODIFIED PROCTOR DENSITY (ASTM D1557). FILL SHALL BE PLACED IN LAYERS OF APPROPRIATE THICKNESSES REQUIRED BY THE NATURE OF THE SOIL OR AS DIRECTED SO THAT THE MATERIAL WILL BE COMPACTED THROUGHOUT THE ENTIRE LAYER. LIFTS SHALL NOT EXCEED 9 INCHES (LOOSE THICKNESS) UNLESS OTHERWISE DIRECTED BY THE OWNER'S TESTING AND INSPECTION AGENCY. SPECIAL CARE SHALL BE TAKEN TO ADEQUATELY COMPACT FILL AND BACKFILL IN AREAS EXCAVATED FOR PIPE TRENCHES, FOOTINGS AND FOUNDATION WALLS.

THE OWNER WILL RETAIN THE SERVICES OF A SOILS CONSULTANT TO TEST PROPOSED FILL MATERIALS, ESTABLISH COMPACTION PROCEDURES, OBSERVE CONTROLLED COMPACTED FILL PLACEMENT ON A FULL-TIME BASIS WHENEVER FILL IS BEING PLACED, AND PERFORM IN-PLACE DENSITY TESTS TO CONFIRM THE ADEQUACY OF COMPACTION.

DEWATERING: PROVIDE FOR ANY DEWATERING AS REQUIRED DURING EXCAVATION AND CONSTRUCTION. THE SITE SOILS ARE MOISTURE SENSITIVE. SEE GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.

CONCRETE: MINIMUM 28 DAY CYLINDER STRENGTH, MAXIMUM SLUMP (PRIOR TO ADDITION OF SUPERPLASTICIZER) AND MAXIMUM WATER/CEMENT RATIO AS FOLLOWS:

	F'c (PSI)	SLUMP	W/C RATIO
FOOTINGS (5-7% AE)	3000	4"	
WALLS AND PIERS (5-7% AE)	4000	3"	
INTERIOR SLABS ON GROUND	4000	3"	
WORK EXPOSED TO WEATHER (5-7% AE)	4500	4"	0.45
OTHER WORK	3000	4"	

BEFORE PLACING ANY CONCRETE, SUBMIT MIX DESIGNS, ADMIXTURES, CURING COMPOUNDS, ETC. FOR REVIEW. MIX DESIGN TO BE IN ACCORDANCE WITH ACI 318, CHAPTER 5. NO CALCIUM CHLORIDE OR ADMIXTURE CONTAINING CHLORIDES SHALL BE USED IN ANY CONCRETE. ALL PUMPED CONCRETE AND CONCRETE FOR SLABS SHALL CONTAIN SUPERPLASTICIZER.

COARSE AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C33 SIZE #57.

REINFORCING STEEL ASTM A615, GRADE 60. WELDED WIRE FABRIC ASTM A185. LAPS 48 DIAMETERS UNLESS NOTED. ALL BARS SECURELY HELD IN ACCURATE POSITION BY SUITABLE ACCESSORIES, TIE BARS, SUPPORT BARS, ETC. ALL CONCRETE WORK TO COMPLY WITH ACI BUILDING CODE (ACI-318) AND DETAILING MANUAL (ACI-SP06). SHOP DRAWINGS SHOWING REINFORCING DETAILS INCLUDING BAR SIZES, SPACING, AND PLACEMENT SHALL BE SUBMITTED AND REVIEWED PRIOR TO FABRICATION.

SHOP DRAWINGS SHOWING ALL REINFORCING SHALL BE SUBMITTED AND REVIEWED PRIOR TO FABRICATION.

ALL FOOTINGS TO HAVE DOWELS THE SAME SIZE AND NUMBER AS VERTICAL REINFORCING IN WALLS, PIERS OR COLUMNS ABOVE, AND EXTEND A MINIMUM OF 48 BAR DIAMETERS ABOVE THE TOP OF FOOTING UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS IN EXTERIOR CORNERS OF WALLS, TO MATCH HORIZONTAL REINFORCING PER TYPICAL DETAILS.

THE OWNER WILL EMPLOY A TESTING AGENCY TO PERFORM INSPECTIONS AND TESTING OF CONCRETE AND REINFORCING STEEL. MAKE ONE SET OF FOUR CONCRETE CYLINDERS FROM EACH 50 CUBIC YARDS WITH A MINIMUM OF ONE SET FROM EACH DAYS OPERATION FOR EACH CLASS OF CONCRETE. TEST SPECIMENS SHALL BE MADE IN ACCORDANCE WITH ASTM C31. MAKE ONE SLUMP TEST ON THE CONCRETE AT THE BEGINNING OF THE POUR AND FOR EACH SET OF CYLINDERS TAKEN IN ACCORDANCE WITH ASTM C143. MAKE ONE ENTRAINMENT AIR TEST WITH EACH SLUMP TEST IN ACCORDANCE WITH ASTM C31. CYLINDER SHALL BE TESTED IN ACCORDANCE WITH ASTM C39, TWO CYLINDERS AT 7 DAYS AND TWO AT 28 DAYS OLD. INSTALLATION OF REBAR OR ANCHORS WITH EPOXY TO BE PERIODICALLY INSPECTED, INCLUDING DRILLING AND CLEANING OF HOLES PRIOR TO PLACEMENT OF EPOXY. THE TESTING AGENCY SHALL SUBMIT, WITHIN 48 HOURS, THE RESULTS OF THEIR INSPECTIONS AND TESTS TO THE ARCHITECT FOR REVIEW.

CURE FLATWORK AS SPECIFIED USING AN APPROVED CURING COMPOUND, OR BY MOIST CURING 7 DAYS MINIMUM. CURE FORMED SURFACES IN PLACE 7 DAYS MINIMUM, BY MOIST CURING OR BY APPLYING AN APPROVED CURING COMPOUND IF FORMS ARE REMOVED IN LESS THAN 7 DAYS.

SLAB ON GROUND: CONSTRUCTION AND SHRINKAGE JOINTS AS SHOWN. EXCEPT AS SHOWN, MINIMUM SLAB THICKNESS TO BE 4". SEE PLAN FOR CRUSHED STONE OR GRAVEL BASE COURSE THICKNESS AND SLAB REINFORCEMENT.

CONTRACTOR TO RECOGNIZE THE POTENTIAL PROBLEM OF CURLING AND SHRINKAGE CRACKING WHEN CASTING SLABS ON VAPOR BARRIER AND/OR SUBGRADE. EVERY EFFORT MUST BE MADE TO USE PROPER MIX, ADMIXTURES WITH LOW SHRINKAGE POTENTIAL AND SLUMP AS SPECIFIED. AFTER CASTING, CURE SLABS TO MAINTAIN THE MOISTURE CONTENT OF THE SLAB AS UNIFORM AS POSSIBLE, AND DO NOT ALLOW THE TOP OF THE SLAB TO BECOME DRIER THAN THE BOTTOM.

SEE ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS ITEMS OF CONCRETE WORK. SHOP DRAWINGS TO BE SUBMITTED AND APPROVED PRIOR TO FABRICATING MATERIAL.

UNIT MASONRY: CONSTRUCT ALL LOAD BEARING WALLS TO COMPLY WITH SPECIFICATION FOR MASONRY STRUCTURES, TMS 602. SEE SPECIFICATIONS FOR COLD AND HOT WEATHER CONSTRUCTION AND WALL PROTECTION REQUIREMENTS. SUBMIT FOR REVIEW PROPOSED MORTAR MIX DESIGNS, GROUT MIX DESIGNS, PRE-CONSTRUCTION TEST DATA, PRODUCT DATA, AND STEEL REINFORCING SHOP DRAWINGS PRIOR TO START OF CONSTRUCTION.

MASONRY WALLS HAVE BEEN DESIGNED ON THE BASIS OF THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH OF MATERIALS:

MINIMUM 28-DAY COMPRESSIVE STRENGTH (PSI)			
CMU	MORTAR	PRECAST	
(NET AREA)	TYPE & STRENGTH	GROUT	STRENGTH F'c
2150	S - 1800	3000	2000

ALL CONCRETE MASONRY UNITS, INCLUDING SPECIAL SHAPES, SHALL CONFORM TO ASTM C99 NORMAL WEIGHT AND SHALL MEET SCHEDULED COMPRESSIVE STRENGTH REQUIREMENTS. MASONRY UNITS SHALL BE STEAM CURED, A MINIMUM OF 28 DAYS OLD AT THE TIME OF DELIVERY AND CONTINUOUSLY PROTECTED FROM EXPOSURE TO RAIN OR OTHER SOURCES OF WATER FROM TIME OF CASTING TO FINAL PLACEMENT IN WALL. MASONRY UNITS SHALL BE DRY, FREE FROM SOIL, ICE AND FROST WHEN LAID IN WALL.

MORTAR: MORTAR SHALL CONFORM TO ASTM C-270 PROPORTION SPECIFICATION. MORTAR SHALL BE PROPORTIONED (BY VOLUME) AS FOLLOWS: ONE PART PORTLAND CEMENT, 1/4 TO 1/2 PARTS HYDRATED LIME OR LIME PUTTY, AND FINE AGGREGATE NOT LESS THAN 2-1/4 AND NOT MORE THAN 3 TIMES THE SUM OF THE VOLUMES OF CEMENT AND LIME USED. NO ADMIXTURES ARE PERMITTED. ADD WATER AS REQUIRED FOR PROPER CONSISTENCY.

GROUT: GROUT SHALL CONFORM TO ASTM C476 PROPORTION SPECIFICATION AND SHALL MEET SCHEDULED COMPRESSIVE STRENGTH REQUIREMENTS. USE FINE GROUT FOR FILLING OPENINGS OR CORE OPENINGS SMALLER THAN 4 INCHES IN LEAST DIMENSION. FINE GROUT SHALL CONSIST OF PORTLAND CEMENT, LIME OR LIME PUTTY, WATER, AND FINE AGGREGATE. USE COARSE GROUT FOR FILLING OPENINGS OR CORES WHERE LEAST DIMENSION OF OPENING IS 4 INCHES OR MORE. COARSE GROUT SHALL CONSIST OF PORTLAND CEMENT, HYDRATED LIME OR LIME PUTTY, WATER, FINE AGGREGATE AND COARSE AGGREGATE.

LOW LIFT GROUTING: WALLS SHALL BE CONSTRUCTED TO 5 FEET MAXIMUM HEIGHT BETWEEN GROUT POURS. GROUT POUR HEIGHT SHALL NOT EXCEED 5 FEET. CONSOLIDATE GROUT AT TIME OF PLACEMENT BY MECHANICAL VIBRATION AND RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED.

ALL MASONRY WALLS BELOW THE SLAB ON GRADE TO BE GROUTED 100% SOLID UNLESS OTHERWISE NOTED.

REINFORCEMENT: ALL REINFORCING STEEL SHALL BE ASTM A615, GRADE 60. MINIMUM LAP SHALL BE 48 BAR DIAMETERS. REINFORCING, METAL TIES AND ANCHORS SHALL BE PROTECTED FROM CONTACT WITH SOIL AND BEFORE BEING PLACED SHALL BE FREE FROM LOOSE RUST AND OTHER COATINGS THAT WILL DESTROY OR REDUCE THE BOND.

UNLESS OTHERWISE NOTED, REINFORCE WALLS WITH TWO VERTICAL #5 BARS AT CORNERS AND ENDS OF WALLS. A MINIMUM OF TWO HORIZONTAL BARS SHALL BE PLACED AT EACH FLOOR AND ROOF LEVEL. OPENINGS TO BE REINFORCED WITH TWO #5 MINIMUM AT HEAD, SILL, AND JAMBS, EXTENDING 2 FEET MINIMUM BEYOND EDGE OF OPENING.

SHOP DRAWINGS SHOWING ALL REINFORCEMENT IN ELEVATION (1/8" TO 1'-0" MINIMUM SCALE) SHALL BE SUBMITTED AND REVIEWED PRIOR TO FABRICATION.

PROVIDE, INSTALL AND REMOVE TEMPORARY BRACING REQUIRED TO ENSURE STABILITY DURING CONSTRUCTION.

OPENINGS: UNLESS OTHERWISE SHOWN OR DETAILED, PENETRATIONS, OPENINGS, AND RECESSES IN MASONRY WALLS ARE PERMITTED SUBJECT TO THE FOLLOWING:

1. THOSE INTERFERING WITH REINFORCING, OR THOSE WHOSE LARGEST DIMENSION IS 6" OR GREATER, OR THOSE IN AREAS SCHEDULED TO BE 100% SOLID ARE TO BE APPROVED BY ARCHITECT'S REPRESENTATIVE PRIOR TO CONSTRUCTION OF AFFECTED WALL.
2. ARCHITECT MAY REQUIRE OFFSETS IN REINFORCING, ADDITIONAL REINFORCING AND/OR GROUTING, OR RELOCATION AS REQUIRED.
3. THOSE WHOSE LARGEST DIMENSION IS LESS THAN 6" AND ARE LOCATED IN HOLLOW BEARING WALLS AND DO NOT INTERFERE WITH REINFORCING, MAY BE PLACED IN BEARING WALLS IF SPACING IS 2'-0" OR GREATER.

LINTELS: UNLESS OTHERWISE DETAILED, PROVIDE LINTELS OVER DOOR, WINDOW, DUCT, AND OTHER MASONRY OPENINGS FOR EACH 4" OF WALL THICKNESS, AS FOLLOWS:

OPG. WIDTH	BRICK (1-L)	* CONCRETE BLOCK **
TO 2'-8"	14X4X5/16	P/C UNIT, 1-44 T8B
2'-9" TO 4'-8"	14X4X5/16	14X3 1/2X5/16
4'-1" TO 5'-8"	16X4X5/16	15X3 1/2X5/16
5'-9" TO 6'-8"	16X4X3/8	16X3 1/2X5/16

* PLACE 4" LEG HORIZONTALLY.
** PLACE 3 1/2" LEG HORIZONTALLY.

PROVIDE 1" OF SOLID BEARING FOR EACH FOOT OF OPENINGS WIDTH (4" MIN.) EACH END. PROVIDE LINTELS FOR OPENINGS GREATER THAN 6'-8" AS ELSEWHERE DETAILED. STEEL LINTELS IN EXTERIOR MASONRY TO BE HOT-DIP GALVANIZED.

NOTE: WHERE CMU WALL THICKNESS IS 6", USE (1) 6" WIDE P/C LINTEL UNIT.

THE OWNER WILL EMPLOY AN APPROVED TESTING AND INSPECTION AGENCY TO INSPECT AND TEST SAMPLES OF ALL STRUCTURAL MASONRY WORK. TESTING AGENCY SHALL INSPECT EACH ASPECT OF MASONRY CONSTRUCTION INCLUDING LAYING OF CMU, PLACEMENT OF REINFORCING BARS, GROUTING AND PROTECTION OF REINFORCED MASONRY WALLS.

DURING CONSTRUCTION THE FOLLOWING MATERIALS SHALL BE TESTED FOR COMPLIANCE WITH MINIMUM STRENGTH REQUIREMENTS:

1. ONE MORTAR TEST PER ASTM C780 FOR EACH TYPE OF MORTAR INDICATED. PREPARE 3 SPECIMENS AND TEST 1 AT 7 DAYS AND 2 AT 28 DAYS.
2. ONE GROUT TEST PER ASTM C1819 FOR EACH STRENGTH OF GROUT INDICATED. PREPARE 3 SPECIMENS AND TEST 1 AT 7 DAYS AND 2 AT 28 DAYS.

TESTING FREQUENCY AND REPORTS: SPECIMENS SHALL BE PREPARED AND TESTS CONDUCTED FOR EACH TYPE AND STRENGTH OF MATERIAL AND FOR EACH 5000 SQUARE FOOT OF WALL AREA. THE TESTING AGENCY SHALL SUBMIT WITHIN 48 HOURS, THE RESULTS OF THEIR INSPECTIONS AND TESTS TO THE OWNER, ARCHITECT, ENGINEER AND CONTRACTOR.

STRUCTURAL STEEL: ALL FABRICATION AND ERECTION SHALL CONFORM TO AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", AND AISC "CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION". WIDE FLANGE AND TEE SECTIONS ASTM A992. STEEL ANGLES, CHANNELS, PLATES AND THREADED RODS ASTM A36. HEADED AND UNHEADED ANCHOR RODS ASTM F1554 GRADE 36. RECTANGULAR AND SQUARE HOLLOW STRUCTURAL SECTIONS (HSS) ASTM A500 GRADE B. ROUND HSS ASTM A500 GRADE B OR ASTM A501. WELDING ELECTRODES E70XX. SHOP CONNECTIONS BOLTED OR WELDED. FIELD CONNECTIONS BOLTED WITH A325-N BOLTS, INSTALLED SNUG TIGHT, UNLESS FULLY TENSIONED (F-T), SLIP-CRITICAL (S-C) OR WELDED CONNECTIONS ARE NOTED ON DRAWINGS.

SUBMIT FOR REVIEW WELDING PROCEDURE SPECIFICATIONS (WPSs) AND PROCEDURE QUALIFICATION RECORDS (PQRs) ACCORDING TO AWS D1.1 FOR EACH WELDED JOINT, WHETHER PREQUALIFIED OR QUALIFIED BY TESTING. SUBMIT WELDER'S CERTIFICATIONS FOR ALL PERSONNEL INVOLVED WITH FIELD WELDING.

ALL BEAM-TO-BEAM AND BEAM-TO-COLUMN CONNECTIONS SHALL BE DESIGNED FOR ASD SERVICE LOAD REACTIONS INDICATED ON PLAN USING ASD METHODOLOGY. WHERE NOT INDICATED, SIZE CONNECTION FOR 12 KIPS MINIMUM.

ALL STEEL SHALL BE CLEANED OF RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS, AND RECEIVE ONE SHOP COAT OF PRIMER PAINT AS REQUIRED BY SPECIFICATIONS. DO NOT PAINT TOP FLANGE OF COMPOSITE BEAMS, STEEL TO RECEIVE SPRAYED ON FIREPROOFING, OR STEEL WITHIN 3" OF BOLT HOLES AT SLIP-CRITICAL CONNECTIONS. STEEL AND HARDWARE EXPOSED TO THE ELEMENTS SHALL BE HOT DIP GALVANIZED PER ASTM A123 AND ASTM A153 AS APPLICABLE, UNLESS OTHERWISE NOTED TO BE PAINTED.

GUYS AND OTHER BRACING REQUIRED TO PROVIDE LATERAL STABILITY TO THE BUILDING SHALL BE ADEQUATELY SIZED AND ANCHORED. THIS BRACING SHALL REMAIN UNTIL THE PERMANENT BRACING SYSTEM IS IN PLACE.

THE FABRICATOR SHALL FURNISH CHECKED SHOP AND ERECTION DRAWINGS, AND OBTAIN APPROVAL PRIOR TO FABRICATING ANY STRUCTURAL STEEL.

THE OWNER WILL EMPLOY A TESTING AGENCY TO PERFORM SHOP AND FIELD INSPECTIONS OF STRUCTURAL STEEL CONNECTIONS, ERECTION, JOIST INSTALLATION AND METAL DECK ATTACHMENT. INSPECTING AGENCY SHALL PERFORM VISUAL OBSERVATIONS TO CONFIRM THAT WORK IS FABRICATED AND ERECTED IN ACCORDANCE WITH APPROVED SHOP DRAWINGS AND CONTRACT DOCUMENTS. PERFORM VISUAL OBSERVATIONS OF ALL SHOP/FIELD WELDS AND BOLTS, ULTRASONICALLY TEST ALL FULL-PENETRATION WELDS. TORQUE TEST AT LEAST 10% OF ALL BOLTS AND ALL SLIP CRITICAL BOLTS. SUBMIT WRITTEN REPORTS OF OBSERVATIONS AND RESULTS OF TESTS WITHIN TWO DAYS TO ARCHITECT AND ENGINEER. IMMEDIATELY NOTIFY ENGINEER WHEN DEVIATION FROM THE CONTRACT DOCUMENTS IS OBSERVED.

MISCELLANEOUS: SEE ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS ITEMS OF STRUCTURAL WORK. SHOP DRAWINGS TO BE SUBMITTED AND REVIEWED PRIOR TO FABRICATING MATERIAL.

STEEL JOISTS: JOISTS TO BE OPEN WEB STEEL JOISTS, K-SERIES OR SP SPECIAL JOISTS AS SHOWN, COMPLYING WITH LATEST SPECIFICATIONS OF SJI. FURNISH AND INSTALL ANGLE BRIDGING IN ACCORDANCE WITH SJI SPECIFICATIONS. DESIGN ALL FLAT ROOF JOISTS AND BRIDGING TO RESIST A NET ASD UPLIFT FORCE OF 16 PSF TYPICALLY (ZONE 1) AND 18 PSF AT LOCATIONS WITHIN 10 FEET OF BUILDING EDGES (ZONE 2) AND BUILDING CORNERS (ZONE 3). DESIGN ALL SLOPED ROOF JOISTS AND BRIDGING FOR THE UPLIFT FORCES SHOWN ON THE UPLIFT ROOF PART PLANS ON THE DRAWINGS.

SUBMIT SIGNED AND SEALED CALCULATIONS FOR REVIEW FOR SPECIAL JOISTS (SP) PRIOR TO FABRICATION OF SP JOISTS.

ATTACH JOISTS ON COLUMN LINES WITH ERECTION BOLTS EACH END AND WELD AFTER PLUMBING AND ALIGNING. ATTACH ALL OTHER JOISTS TO STEEL BEAMS AND BEARING PLATES BY WELDING. PROVIDE ANGLE EXTENSIONS AT BOTTOM CHORDS OF JOISTS FRAMING INTO COLUMNS AND GIRDERS AS DETAILED. ALL JOISTS TO RECEIVE SHOP COAT OF PAINT AS REQUIRED BY SPECIFICATIONS.

ERECTION OF JOISTS SHALL COMPLY WITH LATEST EDITION OF SJI STANDARD SPECIFICATIONS CONCERNING ERECTION STABILITY AND HANDLING, ESPECIALLY AS FOLLOWS: ONE END OF ALL JOISTS SHALL BE ATTACHED TO ITS SUPPORT BEFORE ALLOWING THE WEIGHT OF AN ERECTOR ON THE JOISTS. WHEN BOLTED CONNECTIONS ARE USED, THE BOLTS MUST BE SNUG TIGHTENED.

STEEL DECK: ALL MATERIALS AND INSTALLATION SHALL CONFORM TO AISI SPECIFICATION FOR THE "DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" AND TO RECOMMENDATIONS OF THE STEEL DECK INSTITUTE'S (SDI) DESIGN MANUAL FOR FORM DECKS AND ROOF DECKS.

STEEL ROOF DECK TO BE 1-1/2 INCH DEEP, 20 GAUGE, GALVANIZED WIDE RIB DECK COMPLYING WITH LATEST SPECIFICATION OF SDI. SHEETS TO BE AS LARGE AS POSSIBLE, SPANNING A MINIMUM OF 4 SUPPORTS. ATTACH TO SUPPORTS BY WELDING 6" O.C. AT ENDS AND END LAPS, AND 12" O.C. AT INTERMEDIATE SUPPORTS. SIDE LAPS TO BE SCREW-FASTENED AT MID-SPAN, AND AT 36" ON CENTER, MAXIMUM.

ACOUSTICAL STEEL ROOF DECK TO BE 3 1/2" DEEP, DOVETAIL RIB 20 GAUGE GALVANIZED EPICORE ROOF DECK. SHEETS TO BE AS LARGE AS POSSIBLE, SPANNING A MINIMUM OF 3 SUPPORTS. ATTACH TO SUPPORTS PER TYPICAL DETAIL ON S203.

THE OWNER WILL EMPLOY A TESTING AGENCY TO PERFORM FIELD INSPECTIONS OF METAL DECK INSTALLATION. INSPECTION AGENCY SHALL VISUALLY INSPECT TYPE AND GAGE OF DECK, BEARING LENGTHS, SIDELAP FASTENING AND FASTENING TO SUPPORTS. DO NOT INSTALL ROOFING MATERIALS UNTIL ALL CORRECTIONS REQUIRED BY THE INSPECTION AGENCY HAVE BEEN MADE AND HAVE BEEN RE-INSPECTED.

EXTERNAL AND LOAD BEARING METAL STUDS: ALL MATERIALS AND INSTALLATION METHODS SHALL CONFORM TO ASTM STANDARDS A-653, C-955, C-1807, AND AISI "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS."

ALL STUDS, JOISTS AND ACCESSORIES 16 GA. OR HEAVIER SHALL BE FORMED FROM STEEL WITH A MINIMUM YIELD STRENGTH OF 50 KSI. 18 GA. MEMBERS SHALL BE FORMED FROM STEEL WITH A MINIMUM YIELD STRENGTH OF 33 KSI. ALL STUDS, JOISTS, AND ACCESSORIES SHALL BE GALVANIZED PER ASTM A-924, CLASS G-60.

PROVIDE METAL STUDS AND JOISTS WITH 1 5/8" MINIMUM STIFFENED FLANGES AND MINIMUM PROPERTIES CALCULATED IN ACCORDANCE WITH LATEST AISI SPECIFICATION.

SUBMIT MANUFACTURER'S PRODUCT DATA, SHOP DRAWINGS AND FIELD PLACEMENT DRAWINGS FOR REVIEW FOR ALL STUDS AND ACCESSORIES PRIOR TO ORDERING ANY MATERIAL. DRAWINGS SHALL INDICATE ALL FASTENING DETAILS, BRACING, AND PANELIZATION, IF USED.

PREPARE AND SUBMIT DESIGN ENGINEERING CALCULATIONS FOR ALL EXTERIOR WALL METAL STUDS AND CONNECTIONS WITH SHOP AND FIELD PLACEMENT DRAWINGS. CALCULATIONS SHALL BE SEALED BY REGISTERED PROFESSIONAL ENGINEER. DESIGN OF METAL STUDS SHALL COMPLY WITH GRAVITY, WIND, AND SEISMIC LOADS IN ACCORDANCE WITH BUILDING CODE USING PARAMETERS DEFINED ABOVE UNDER DESIGN LOADS.

MAXIMUM PERMISSIBLE DEFLECTION OF STUDS UNDER WIND LOADS SHALL BE LIMITED TO 1/600 FOR WALL CONSTRUCTION WITH MASONRY VENEER, AND 1/360 ELSEWHERE.

STUDS SHALL BE INSTALLED SEATED SQUARELY (WITHIN 1/8") AGAINST THE WEB PORTION OF THE TOP AND BOTTOM TRACKS. TRACKS SHALL REST ON A CONTINUOUS, UNIFORM BEARING SURFACE. BRACE STUDS AT 4'-0" MAXIMUM VERTICALLY ON CENTER PER MANUFACTURER'S STANDARD BRIDGING DETAILS.

CUTTING OF STEEL FRAMING MEMBERS MAY BE ACCOMPLISHED WITH A SAW OR SHEAR. TORCH CUTTING OF LOAD BEARING MEMBERS IS NOT PERMITTED. SPLICING OF MEMBERS IS NOT PERMITTED.

TEMPORARY BRACING SHALL BE PROVIDED AS NECESSARY AND LEFT IN PLACE UNTIL WORK IS PERMANENTLY STABILIZED.

UNLESS NOTED OTHERWISE, ALL CONNECTIONS SHALL BE MADE USING #12-14 SELF TAPPING SCREWS. WELDED CONNECTIONS MAY BE USED IN LIEU OF SCREWS AT CONTRACTOR'S OPTION PROVIDED ALL WELDS ARE INDICATED ON SHOP AND FIELD DRAWINGS. ALL WELDS SHALL CONFORM TO AWS D1.3-B AND SHALL BE MADE BY OPERATORS QUALIFIED IN ACCORDANCE WITH SECTION 6.0 OF AWS CODE. TOUCH UP ALL WELDS WITH ZINC RICH PAINT.

ROUGH CARPENTRY: ALL MATERIALS AND WORKMANSHIP TO BE IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION" BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.

SAWN LUMBER SHALL BE SPRUCE-PINE-FIR OR EQUAL WITH THE FOLLOWING BASIC DESIGN VALUES:

ELEMENT	GRADE	Fb (PSI)	Fv (PSI)	Fc (PSI)	E (PSI)
JOISTS & RAFTERS	#1/#2	875	135	1,150	1,400,000
STUDS, PLATES, & BLOCKING	#1/#2	875	135	1,150	1,400,000

PLYWOOD FOR ROOF SHEATHING TO BE C-D/INT APA WITH EXTERIOR GLUE. MINIMUM THICKNESS 19/32". MINIMUM SPAN RATING 40/20. PROVIDE 1/8" MINIMUM GAP AT ALL PANEL JOINTS. FASTEN ROOF SHEATHING TO FRAMING USING 8d NAILS AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT ALL OTHER SUPPORTS. PROTECT PANELS FROM WEATHER UNTIL FULLY COVERED AND PERMANENTLY PROTECTED BY ROOFING MATERIALS.

PROVIDE GALVANIZED STEEL HURRICANE TIE DOWN ANCHORS WHERE INDICATED ON DRAWINGS.

ALL WOOD FRAMING ELEMENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR AS REQUIRED FOR ANY ANGULAR FIT AGAINST ABUTTING MEMBERS. MEMBER SHALL BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED.

CONTRACTOR SHALL BE RESPONSIBLE FOR ADEQUATE TEMPORARY BRACING OF ALL BUILDING ELEMENTS. TEMPORARY BRACING SHALL NOT BE REMOVED UNTIL THE STRUCTURE IS SECURELY TIED TOGETHER.

NAILS: NAILS INDICATED SHALL HAVE DIAMETERS AND LENGTHS AS FOLLOWS:

8d:	0.131" x 2.5"	10dX2.5: 0.148" x 2.5"
10d:	0.131" x 1.5"	10dX1.5: 0.148" x 1.5"
16d:	0.148" x 3.0"	16d: 0.162" x 3.5"

METAL FASTENERS IN WOOD TREATED WITH WOOD PRESERVATIVE: ALL FASTENERS (ANCHOR BOLTS, NUTS, WASHERS, SCREWS, LAGS, NAILS, ETC) IN CONTACT WITH PRESSURE-TREATED WOOD MUST BE EITHER TYPE 316 STAINLESS STEEL OR HOT DIPPED GALVANIZED (ASTM-A153) MATERIAL. THIS INCLUDES FASTENERS DRIVEN THROUGH SHEATHING OR CLADDING MATERIALS AND INTO THE PRESSURE-TREATED STOCK, NAILS SECURING STUDS TO MUDDLIS, AND CLIPS AND NAILS FASTENING RIM JOISTS TO MUDDLIS. ALL SHEET METAL FRAMING PRODUCTS INCLUDING JOIST HANGERS, FRAMING ANGLES, STRAPS, HOLD-DOWNS, ETC. IN CONTACT WITH PRESSURE-TREATED WOOD SHALL BE HOT DIPPED GALVANIZED (ASTM-A653) MEETING THE REQUIREMENTS FOR COATING DESIGNATION G185 OR BETTER. STAINLESS STEEL AND HOT-DIPPED GALVANIZED FASTENERS AND HARDWARE SHALL NOT BE MIXED.

ENGINEERED WOOD PRODUCTS: LAMINATED VENEER LUMBER OR PARALLAM STRAND LUMBER
EXTREME FIBER STRESS IN BENDING, EDGWISE: 2600 PSI FOR 12-INCH NOMINAL DEPTH MEMBERS
MODULUS OF ELASTICITY: EDGWISE 2,000,000 PSI

MINIMUM DESIGN WIND PRESSURES (ULT.) FOR EXTERIOR WALL CLADDING (PSF)		
TRIBUTARY AREA (Sq. Ft.)	TYPICAL	CORNER
<20	34.7	46.4
21-50	33.1	43.3
51-100	31.0	39.1
101-200	29.5	36.0
201-500	27.9	32.9
>500	25.8	28.8

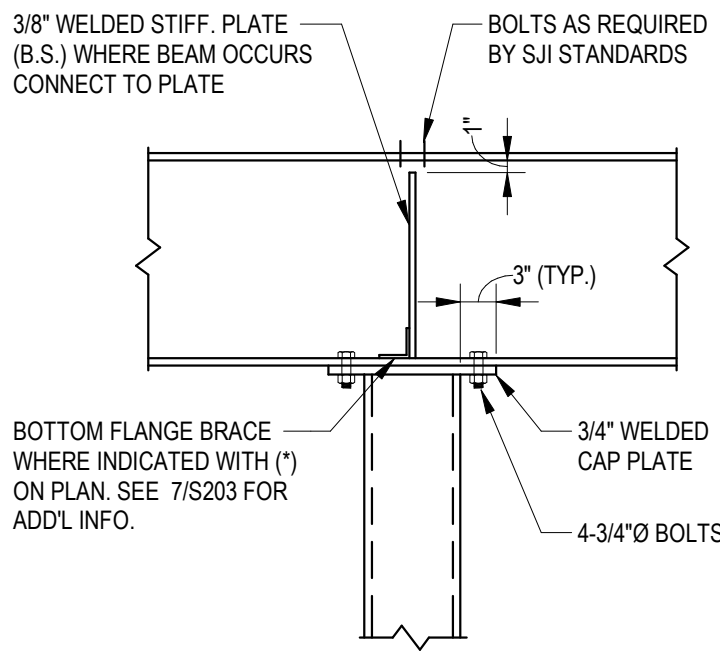
NOTES:
1. CORNER PRESSURES APPLY WITH IN 10'-0" OF EVERY EXTERIOR BUILDING CORNER

PROJECT TEAM

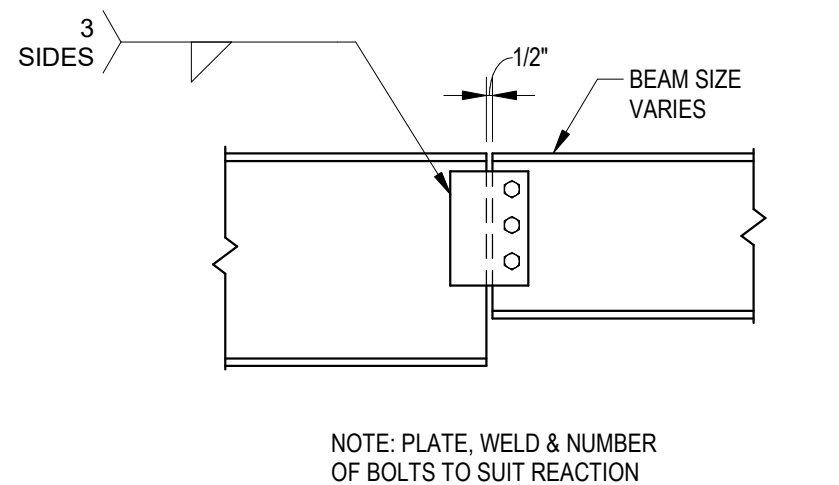
CLIENT
Center for Student Learning
Charter School at Pennsbury
345 Lakeside Drive
Levittown, PA 19054
215-269-7390
ARCHITECTURAL
Heckendorn Shiles Architects
347 East Conestoga Road
Wayne, PA 19087
610-994-3500

STRUCTURAL ENGINEER
A. W. Lookup Corporation
5

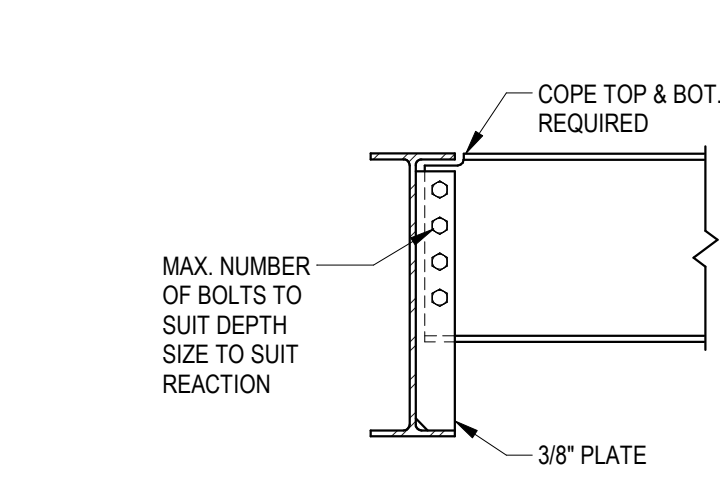
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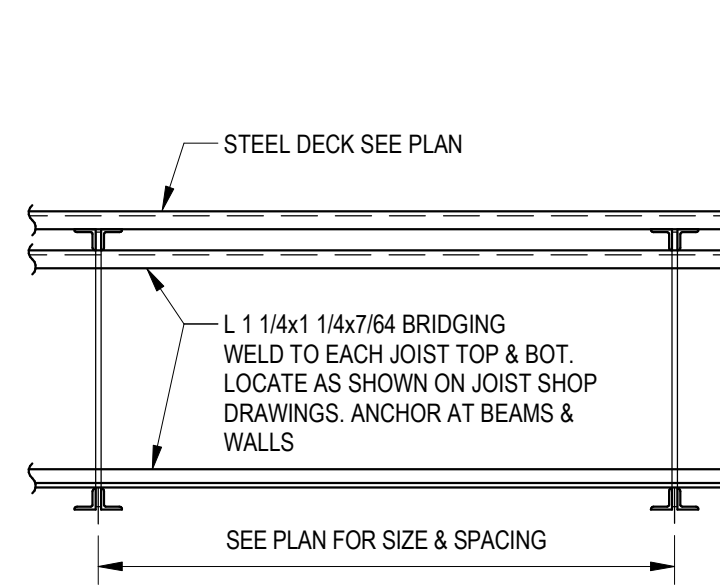
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S203 / NO SCALE
TYPICAL ROOF BEAM OVER COLUMN



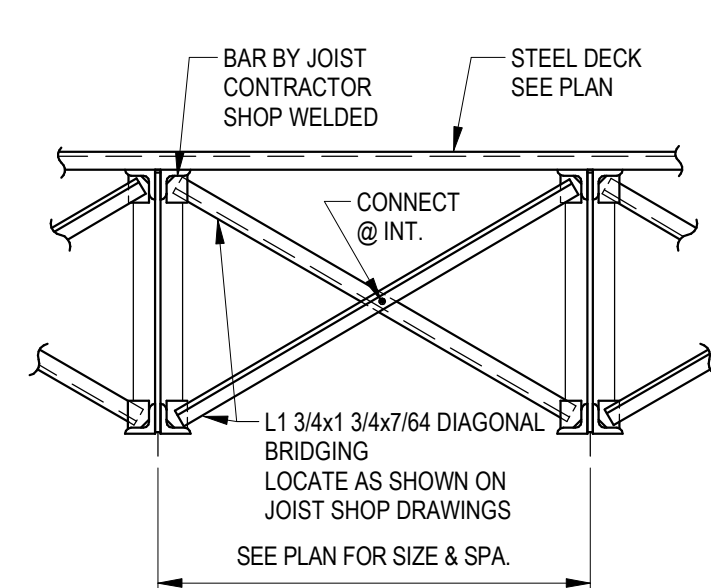
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S203 / NO SCALE
TYPICAL BEAM SPLICE



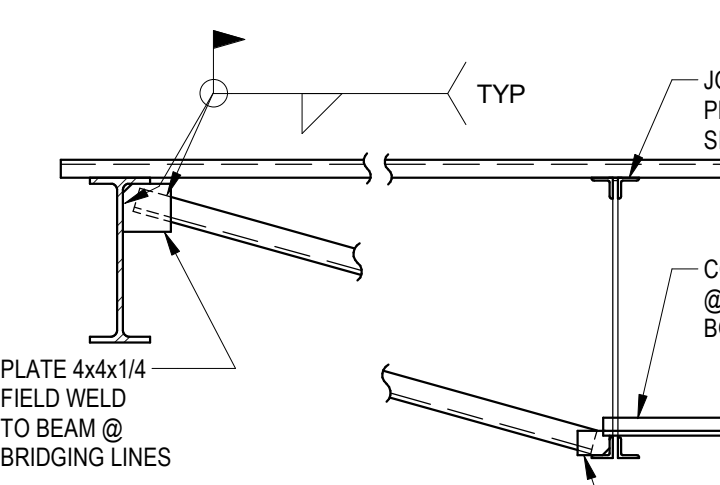
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S203 / NO SCALE
TYPICAL FULL DEPTH CONNECTION WHERE BEAM DEPTHS VARY



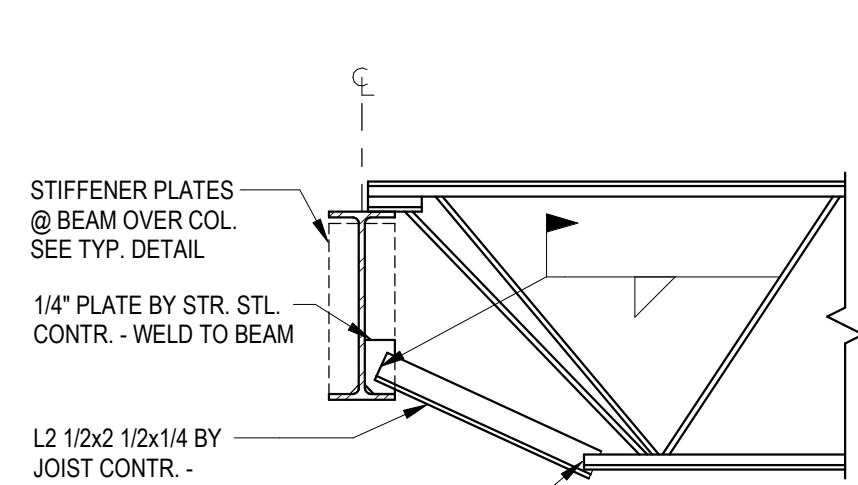
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S203 / 3/4\"/>



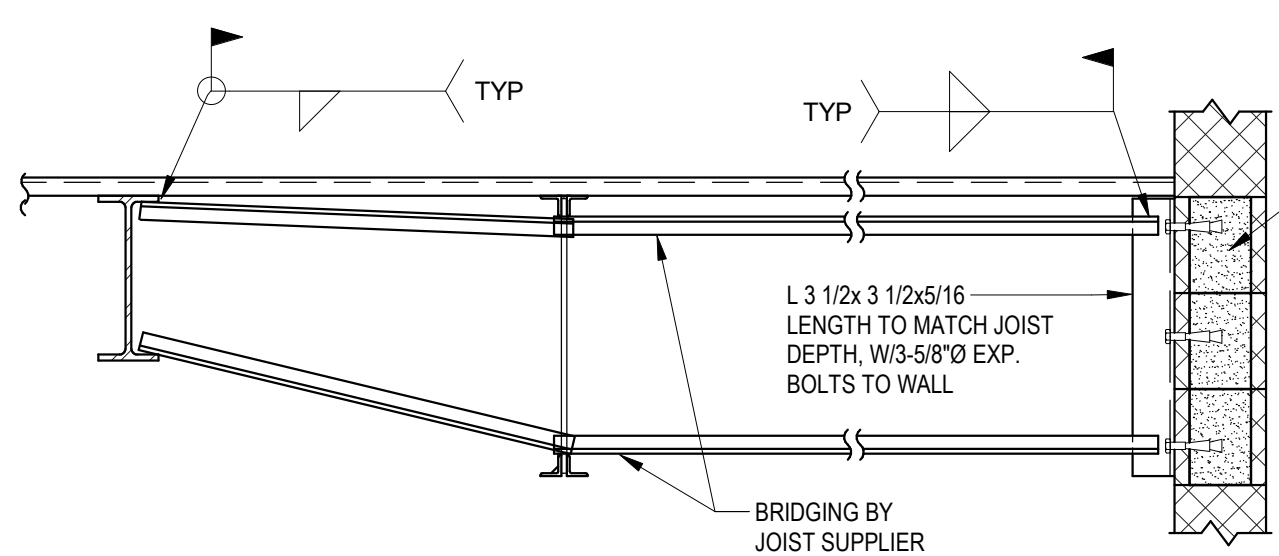
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S203 / 3/4\"/>



6
S203 / NO SCALE
TYPICAL UPLIFT BRIDGING

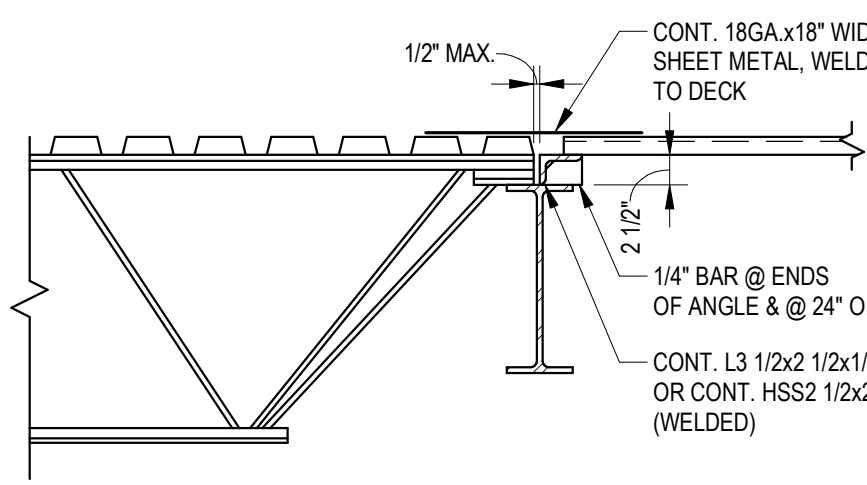


7
S203 / NO SCALE
TYPICAL BOTTOM FLANGE BRACE



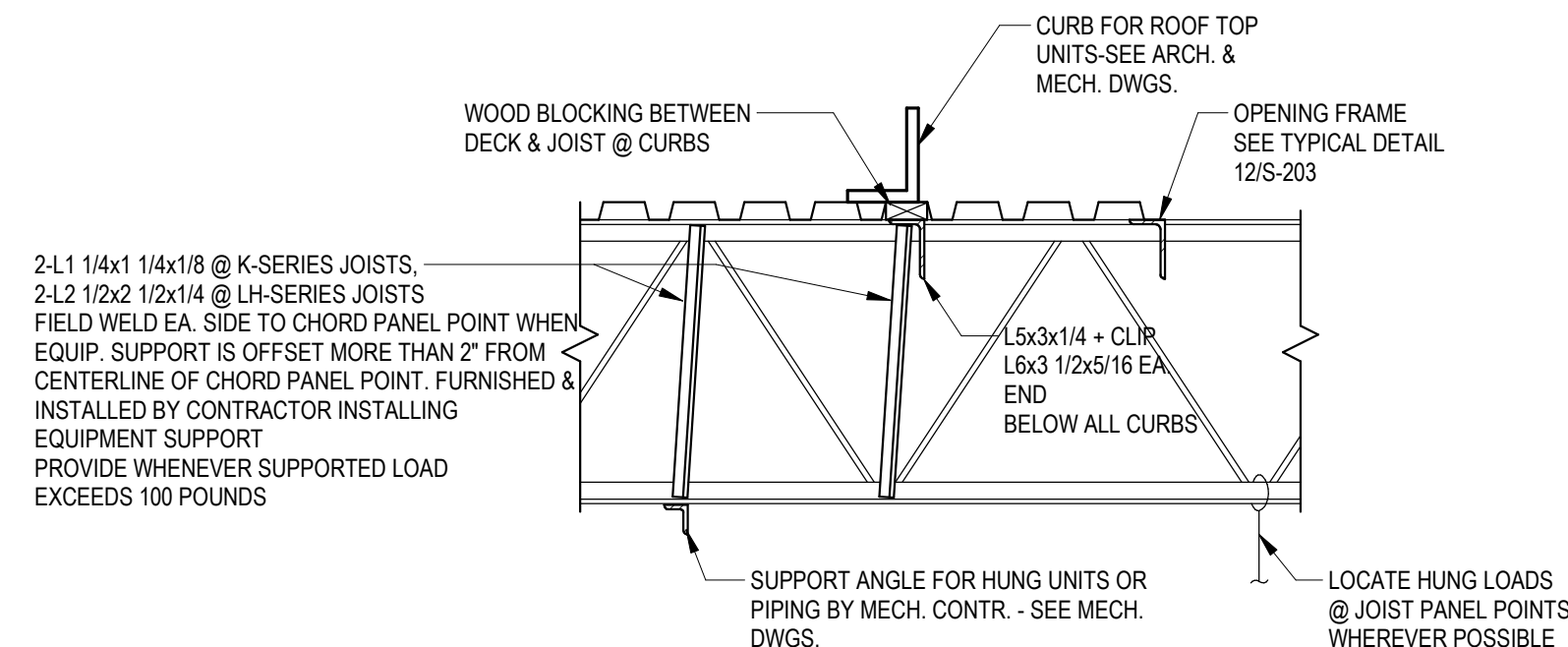
CONDITION AT BEAM

8
S203 / 3/4\"/>

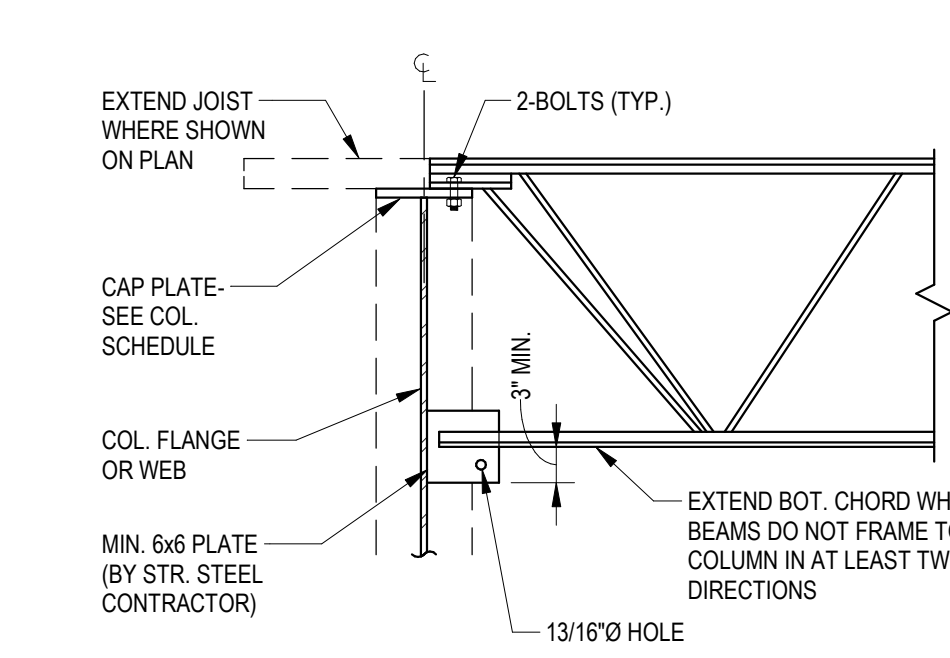


CONDITION AT WALL

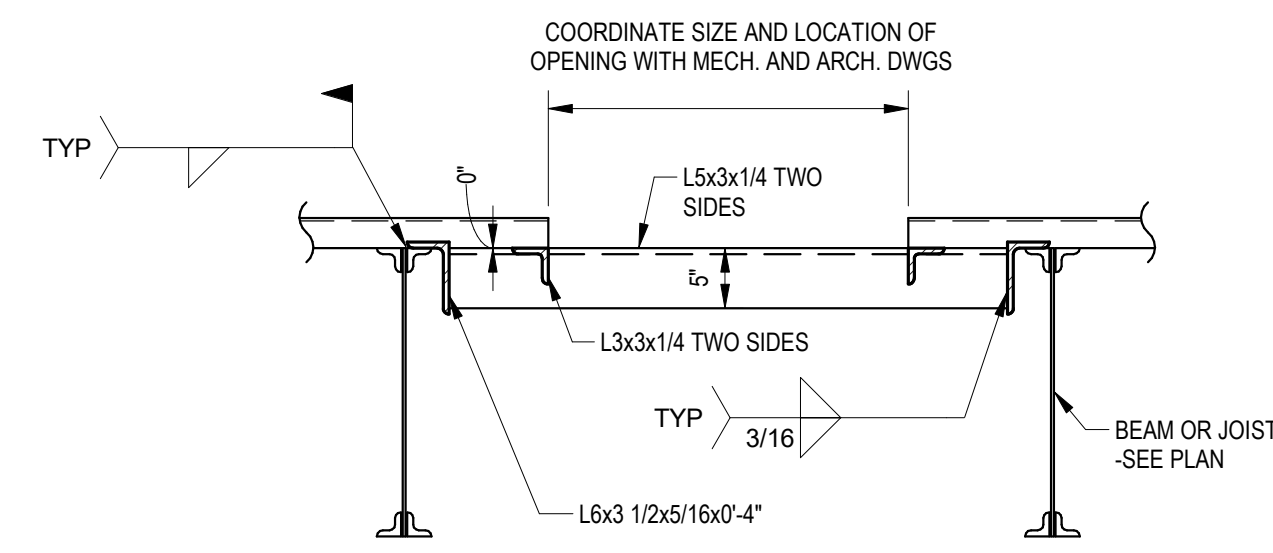
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S203 / NO SCALE
TYPICAL CHANGE IN ROOF DECK DIRECTION



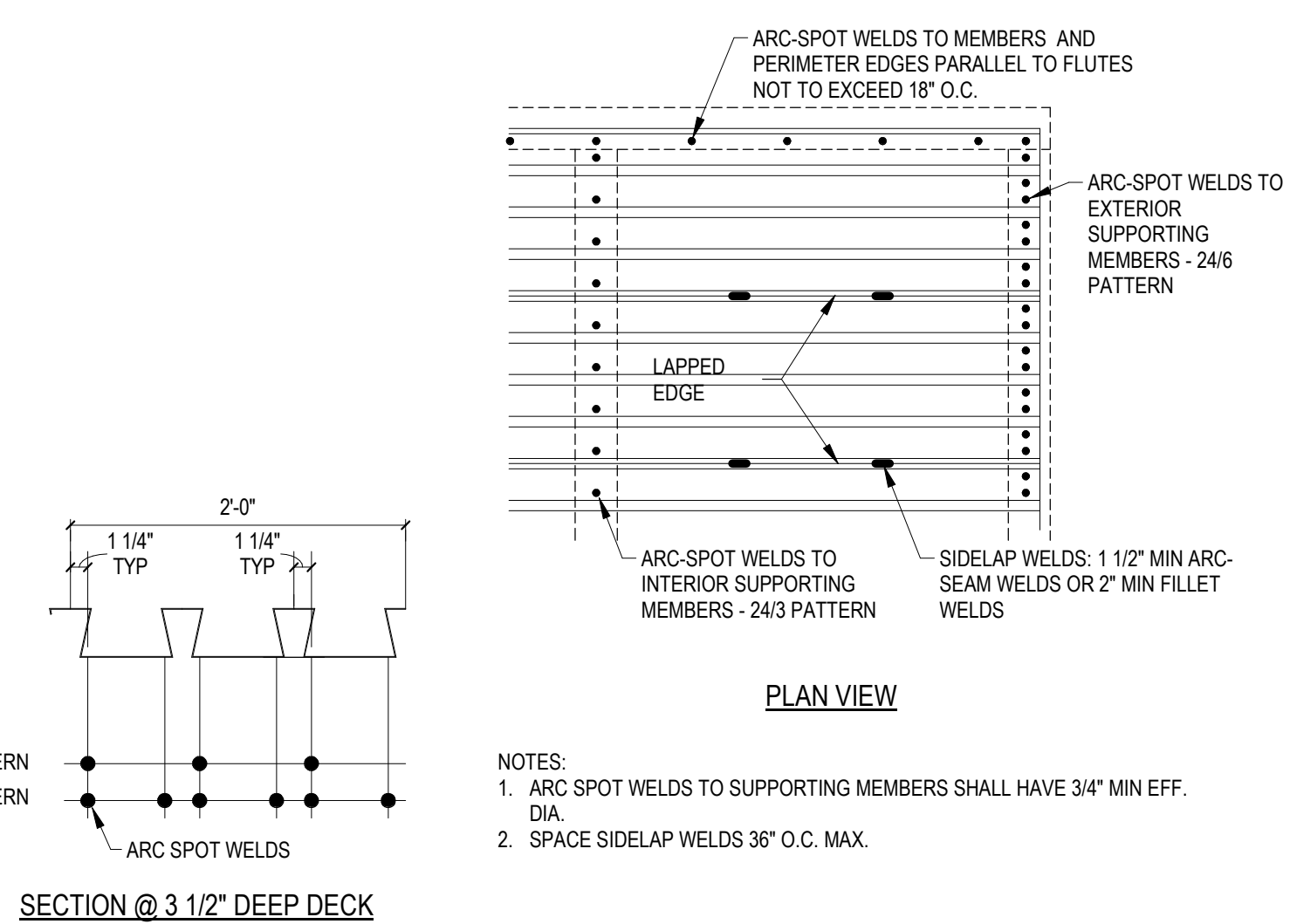
10
S203 / NO SCALE
TYPICAL JOIST REINFORCING AT MECHANICAL & OTHER EQUIPMENT



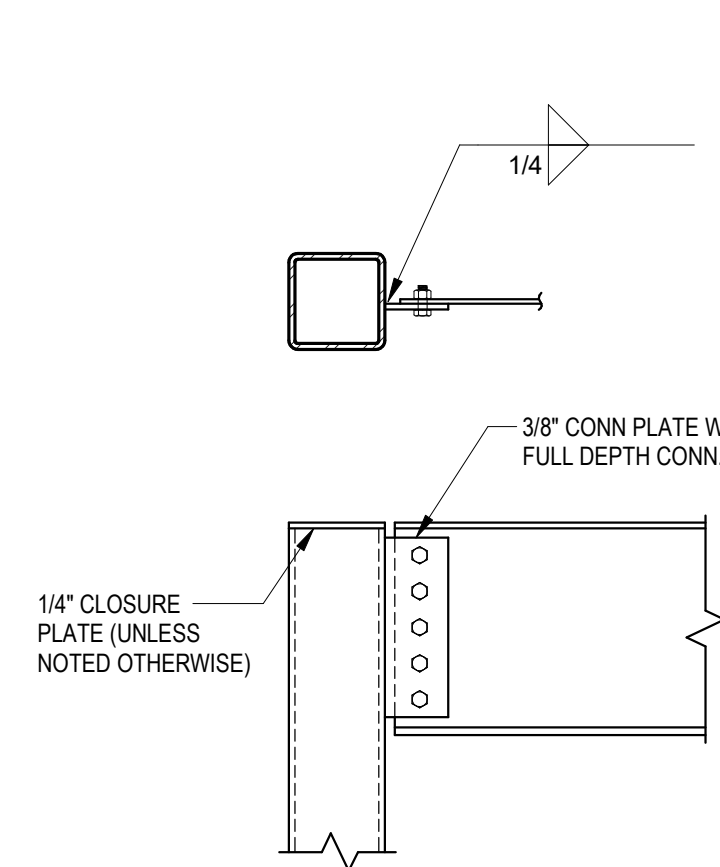
11
S203 / NO SCALE
TYPICAL JOIST TO COLUMN CONNECTION



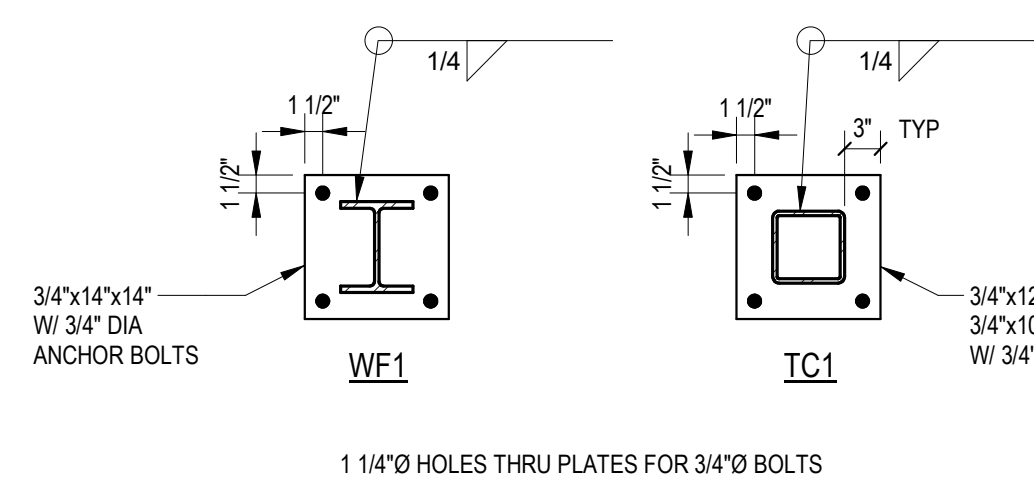
12
S203 / NO SCALE
TYPICAL FRAME AT OPENING IN JOIST & METAL DECK ROOF



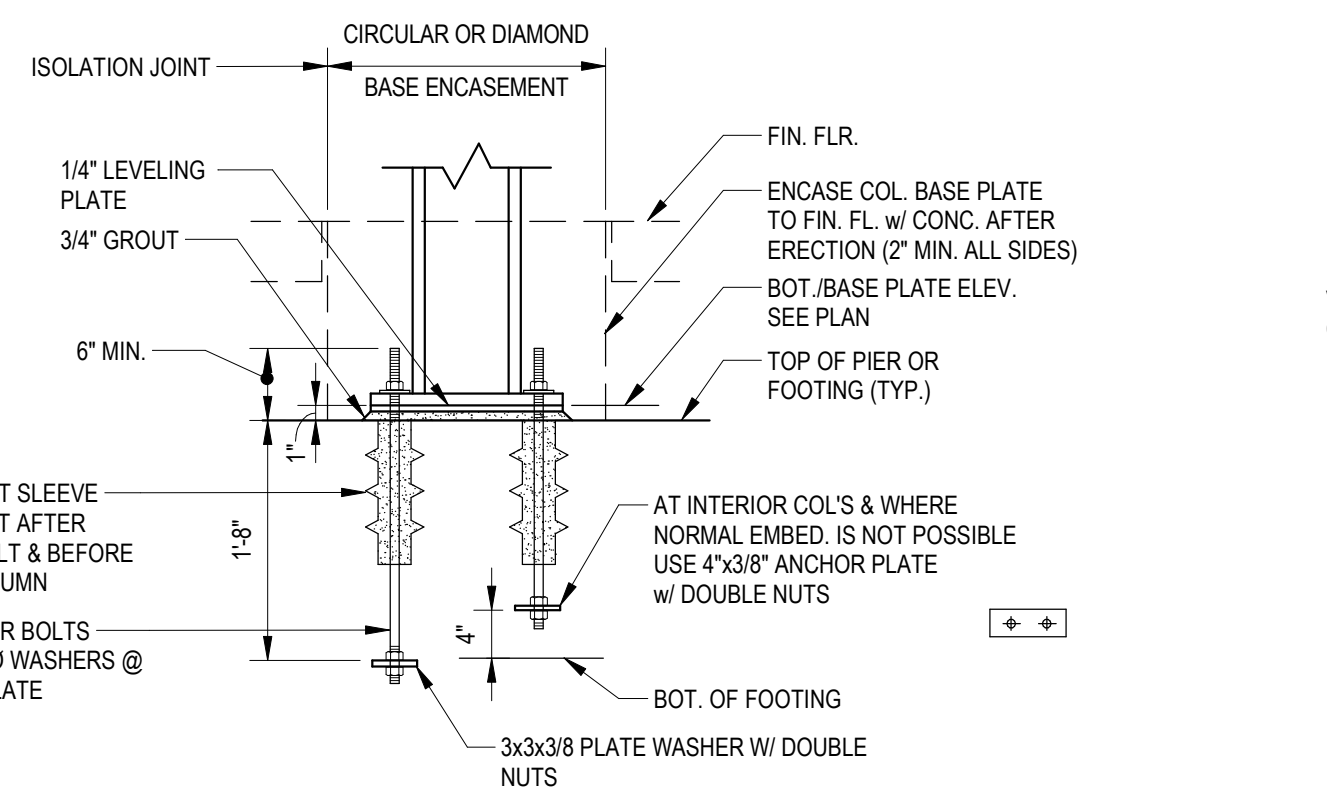
13
S203 / 1\"/>



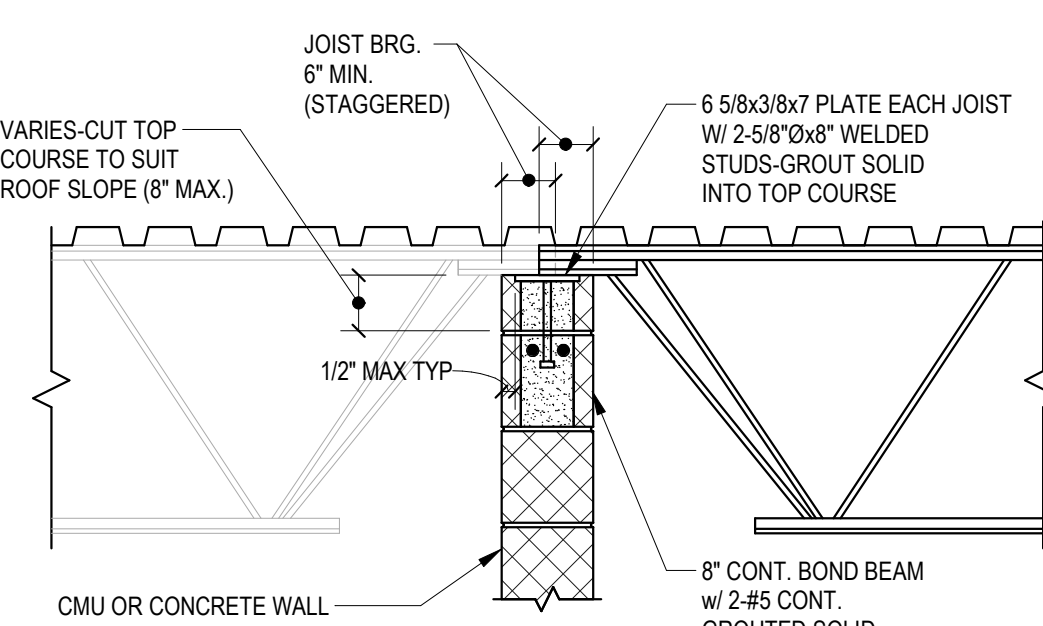
14
S203 / 3/4\"/>



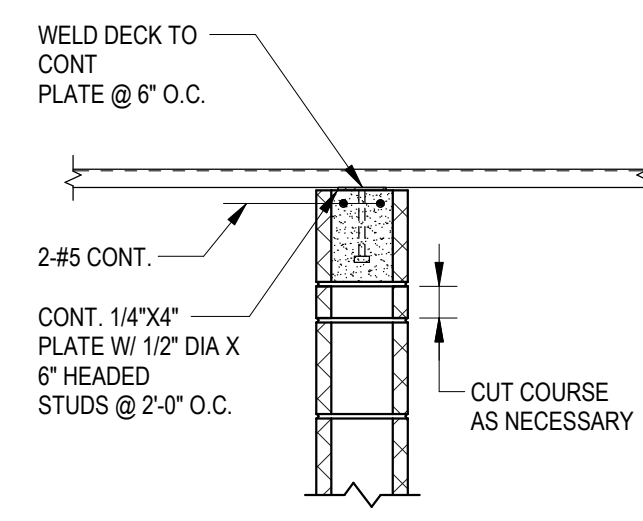
15
S203 / 3/4\"/>



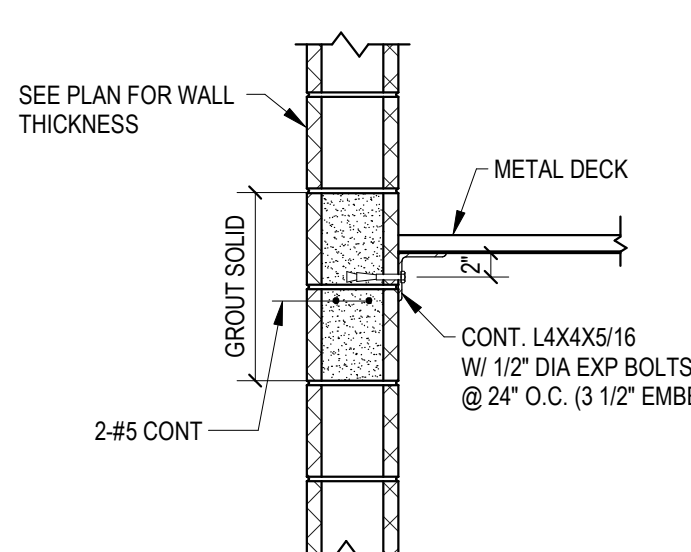
16
S203 / 3/4\"/>



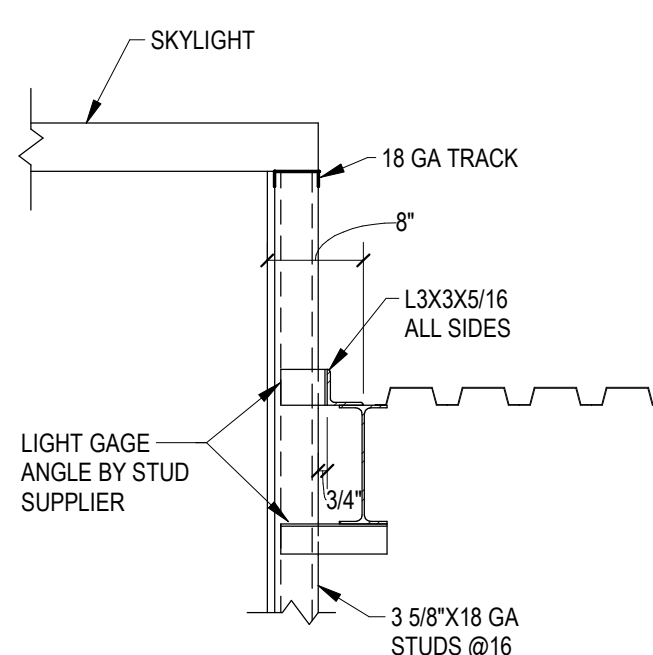
19
S203 / 3/4\"/>



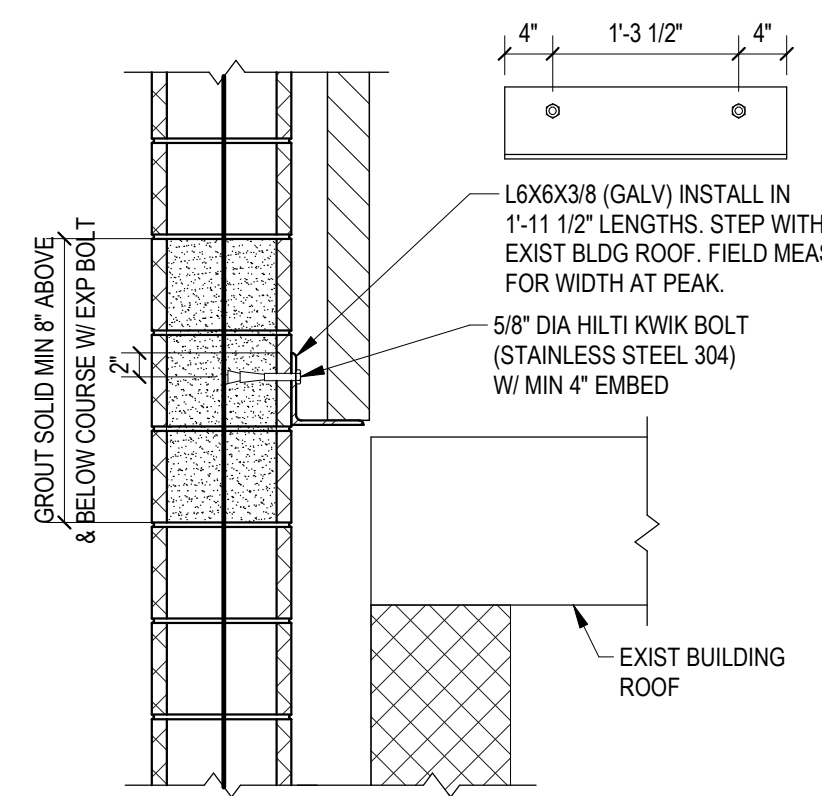
20
S203 / 3/4\"/>



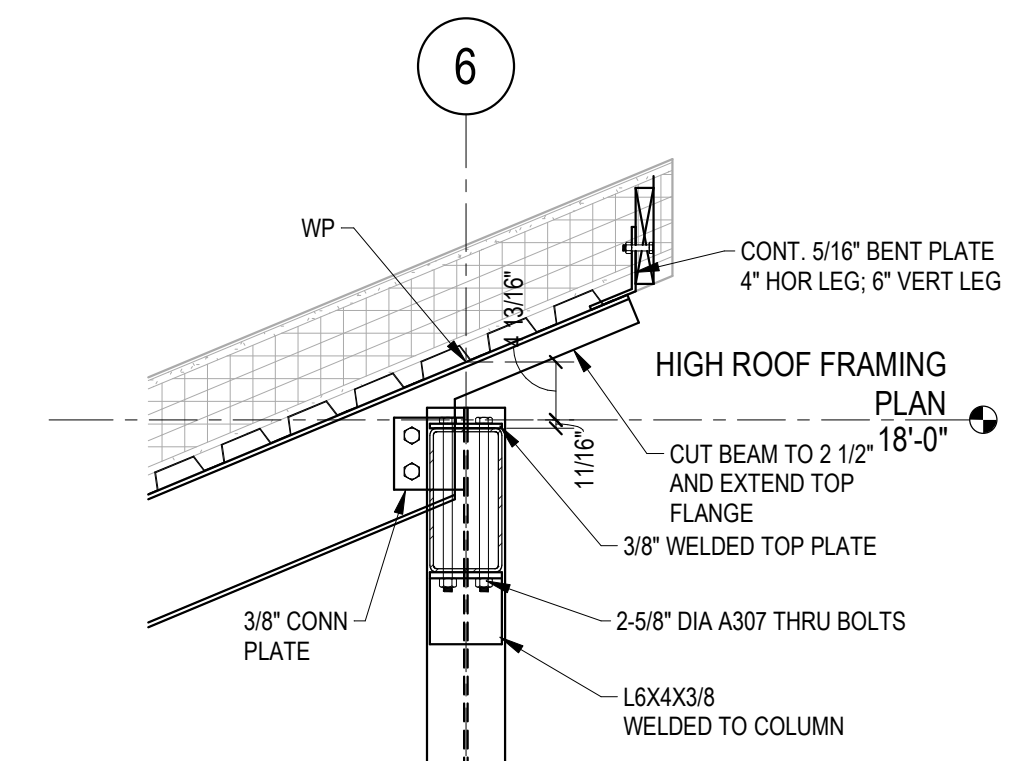
21
S203 / 3/4\"/>



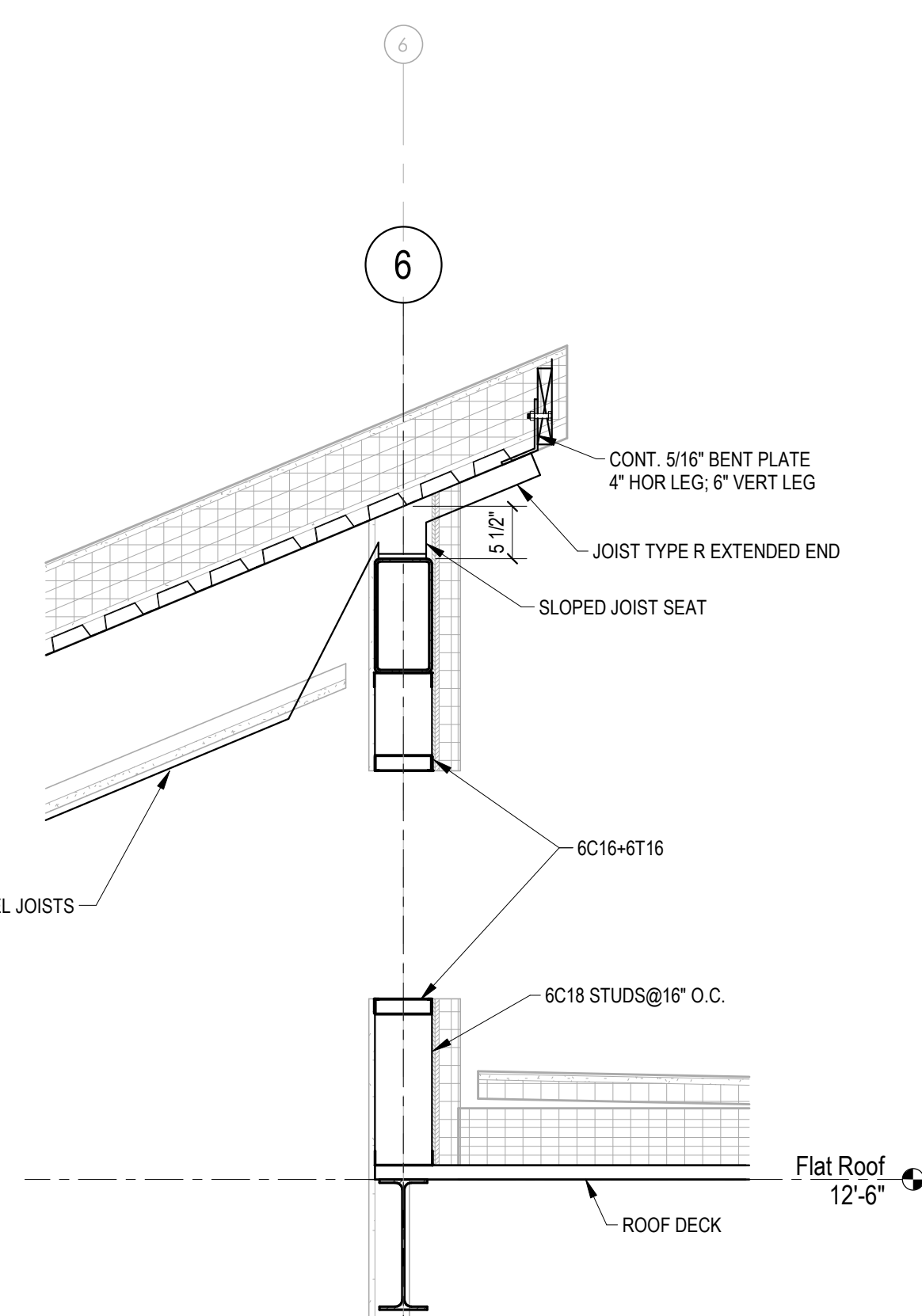
23
S203 / 3/4\"/>



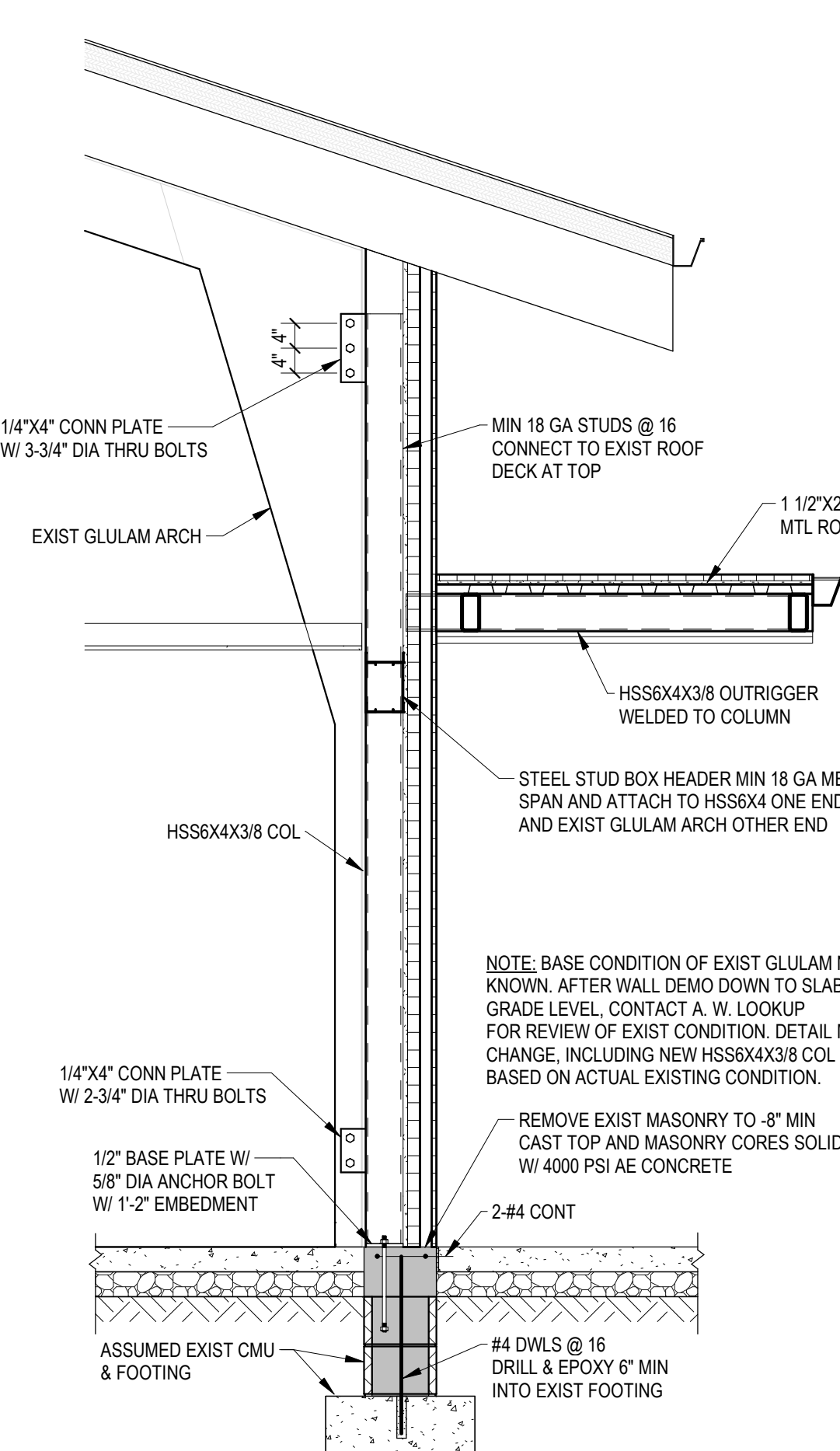
24
S203 / 3/4\"/>



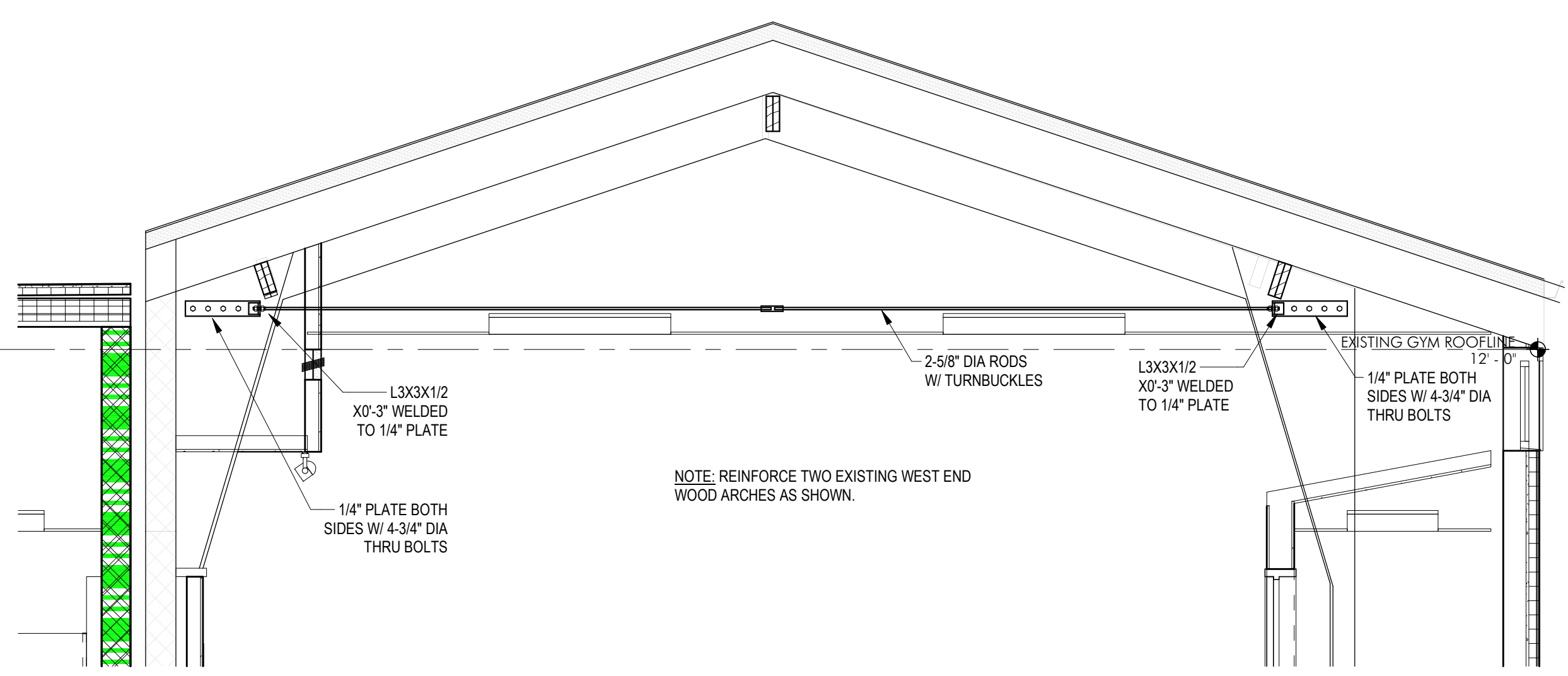
25
S203 / 3/4\"/>



22
S203 / 3/4\"/>



18
S203 / 1/2\"/>



17
S203 / 3/8\"/>