

DESIGN CRITERIA NOTES:

1. THE FOLLOWING CODES AND STANDARDS, INCLUDING ALL SPECIFICATIONS REFERENCED WITHIN, SHALL APPLY TO THE DESIGN, CONSTRUCTION, QUALITY CONTROL AND SAFETY OF ALL WORK PERFORMED ON THE PROJECT. USE THE LATEST EDITIONS UNLESS NOTED OTHERWISE.

INTERNATIONAL BUILDING CODE, 2018 EDITION
ASCE 7-16 - MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
ASCE 303-16 - SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
ASCE 303-16 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES
ACI 318-14 - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND MASONRY
TIA 402/602-16 - SPECIFICATION OF MASONRY STRUCTURES

2. DESIGN LOADS ARE AS LISTED BELOW.

LIVE LOADS:	
(UNIFORMLY DISTRIBUTED LIVE LOADS IN PSF)	
READING ROOMS	60 PSF
STACK ROOMS (BOOKS & SHELVING AT 65 PCF)	125 PSF
MECHANICAL ROOMS	100 PSF OR EQUIP. WT.
STAIRS, PUBLIC AREAS, COMPUTER ROOMS	100 PSF
STORAGE AREAS	125 PSF
ROOF	20 PSF

- SUPERIMPOSED DEAD LOADS:
- | | |
|---|-----------------------|
| MECHANICAL, ELECTRICAL, AND CEILING FINISHES WHERE SHOWN ON ARCHITECTURAL | 10 PSF
AS REQUIRED |
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SNOW LOADS:
GROUND SNOW LOAD $P_g = 30$ PSF
FLAT ROOF SNOW LOAD = 2.51 PSF + DRIFT
EXPOSURE FACTOR $C_e = 1.0$
IMPORTANCE FACTOR $I = 1.0$
THERMAL FACTOR $C_t = 1.0$

SEISMIC LOADS:
RISK CATEGORY = II
IMPORTANCE FACTOR = 1.25
MAPPED SPECTRAL RESPONSE ACCELERATIONS:
 $S_a = 0.18$ g
 $S_1 = 0.08$ g
SPECTRAL RESPONSE COEFFICIENTS:
 $SDS = 0.18$ g
 $SD1 = 0.08$ g
SITE CLASS = D
SEISMIC DESIGN CATEGORY = B
DESIGN BASE SHEAR = $0.002 W$
SEISMIC RESPONSE COEFFICIENT, $C_s = 0.002$
RESPONSE MODIFICATION FACTOR, $R = 3.5$
ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE

WIND LOADS:
BASIC WIND SPEED, V (MILE/H) = 123 MPH
RISK CATEGORY = III
WIND EXPOSURE = B
APPLICABLE INTERNAL PRESSURE COEFFICIENT $+/- 0.18$
WIND PRESSURES FOR COMPONENTS AND CLADDING (LOCATIONS PER ASCE 7)

COMPONENT AND CLADDING (ULT) WIND PRESSURE (psf)				
WALL PRESSURE				
ZONE 1609.6.2 (2)	Effective Wind Area (SF)	ROOF NEG.	ROOF POS.	
1	10	-43	16	
	20	-41	16	
	50	-37	16	
	100	-34	16	
2	10	-57	16	
	20	-54	16	
	50	-49	16	
	100	-45	16	
3	10	-78	16	
	20	-71	16	
	50	-61	16	
	100	-54	16	
ZONE 1609.6.2 (2)	Effective Wind Area (SF)	WALL NEG.	WALL POS.	
4	10	-27	25	
	20	-26	24	
	50	-24	22	
	100	-23	21	
5	10	-33	25	
	20	-31	24	
	50	-28	22	
	100	-26	21	

FOUNDATION NOTES:

1. IN THE ABSENCE OF A GEOTECHNICAL SOILS REPORT, THE FOUNDATION DESIGN IS BASED ON AN ASSUMED SOIL BEARING CAPACITY OF 2000 PSF. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD WHICH VARIES FROM THOSE CONDITIONS ASSUMED FOR DESIGN. STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.
2. SPREAD FOOTINGS ARE DESIGNED FOR THE ALLOWABLE NET SOIL BEARING PRESSURE OF 2,000 PSF. SOIL BEARING CAPACITY SHALL BE VERIFIED BY A LICENSED GEOTECHNICAL ENGINEER PRIOR TO FOUNDATION PLACEMENT.
3. BASEMENT WALLS SHALL NOT BE BACKFILLED UNTIL FLOORS AT TOP AND BOTTOM ARE IN PLACE AND HAVE REACHED REQUIRED DESIGN STRENGTH.
4. PROVIDE CRACK CONTROL JOINTS IN SLABS ON-GRADE AS INDICATED BY THE SPECIFICATIONS (TYPICAL DETAILS), WHERE SOIL SUPPORTED SLAB CONSTRUCTION OR CONTROL JOINTS ARE NOT SHOWN ON PLAN. JOINTS SHALL OCCUR IN EACH DIRECTION AT EACH COLUMN LINE AND RE-ENTRANT CORNER. ADDITIONAL JOINTS SHALL OCCUR IN EACH DIRECTION AT A SPACING NOT TO EXCEED 36 TIMES THE SLAB THICKNESS. AVOID EXCEEDING AN ASPECT RATIO OF 1.5 TO 1 FOR AREAS BOUNDED BY JOINTS. CUT JOINTS 25% OF THE DEPTH OF THE SLAB.
5. A VAPOR BARRIER PER THE ARCHITECTURAL SPECIFICATIONS SHALL BE INSTALLED DIRECTLY BENEATH THE SOIL SUPPORTED SLAB. TAPE AND LAP ALL JOINTS PER MANUFACTURER RECOMMENDATIONS.
6. FOUNDATIONS SHALL BEAR ON UNDISTURBED VIRGIN SOIL AND/OR SUPERVISED COMPACTED FILL, FREE OF FROST AND ANY ORGANIC MATERIALS.
7. ALL FOUNDATIONS AND EXCAVATIONS SHALL BE PROTECTED FROM FROST EXPOSURE DURING AND AFTER CONSTRUCTION. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 3'-0" BELOW EXTERIOR FINISHED GRADE, U.N.O.
8. DO NOT ALLOW SURFACE WATER TO ACCUMULATE AND/OR POND IN EXCAVATIONS. TEMPORARY DRAINAGE SYSTEM TO BE USED DURING CONSTRUCTION SHALL BE DESIGNED AND INSTALLED BY THE CONTRACTOR IN ACCORDANCE WITH THE RECOMMENDATIONS GIVEN IN THE GEOTECHNICAL REPORT AND THE REQUIREMENTS OF THE GOVERNING BUILDING CODE.
9. DO NOT BACKFILL AGAINST WALLS UNTIL SUPPORTING SLABS ARE IN PLACE AND HAVE ATTAINED REQUIRED STRENGTH.
10. WHERE PIPES OR UTILITIES PASS THROUGH FOOTING, DROP FOOTING SO UTILITIES PASS OVER THE TOP OF THE FOOTING OR DROP UTILITIES TO PASS BELOW FOUNDATION 6" (MIN.) PROVIDE SLEEVES AS NECESSARY.

CAST-IN-PLACE CONCRETE NOTES:

1. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE LATEST EDITIONS OF THE ACI BUILDING CODE (ACI 318), ACI DETAILING MANUAL (ACI 315), AND SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301).
2. CONCRETE, UNLESS OTHERWISE NOTED ON THE PLANS, SHALL BE NORMAL WEIGHT CONCRETE WITH THE FOLLOWING 28 DAY COMPRESSIVE STRENGTH (PSI):
- ALL CONC, U.N.O. = 3,000 N.W.C.
- SLAB-ON-GRADE = 3,500 N.W.C.
- EXTERIOR SLAB-ON-GRADE = 4,500 N.W.C. EXPOSURE CLASS F2
- FOOTINGS, FOUNDATION WALLS, & PIERS = 4,000 N.W.C.
- SLABS ON METAL DECK / TOPPING = 3,500 L.V.C.
- *N.W.C.-DENOTES NORMAL WEIGHT CONC. WITH A MAX. DRY DENSITY = 150 PCF
*L.V.C.-DENOTES LIGHTWEIGHT CONC. WITH A MAX. DRY DENSITY = 113 PCF
3. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE AS NOTED BELOW. SEE ACI 318 FOR CONDITIONS NOT NOTED.
- REINF. STEEL IN CONCRETE CAST AGAINST SOIL = 3"
- REINF. STEEL IN CONCRETE EXPOSED TO SOIL OR WEATHER:
#5 BARS AND SMALLER = 1.5"
#6 BARS AND LARGER = 2"
- CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
SLAB AND WALL REINF. = 0.75"
BEAMS AND COLUMNS = 1.5"
4. ALL CONCRETE EXPOSED TO WEATHER SHALL BE AIR ENTRAINED TO 5% $\pm$ 1%.
5. WORKABILITY SHALL NOT BE ACHIEVED THROUGH ADDITION OF WATER. WATER REDUCING ADMIXTURES (PLASTICIZERS) SHALL BE USED TO INCREASE WORKABILITY. SEE SPECIFICATIONS FOR CONCRETE SLUMP REQUIREMENTS.
6. HORIZONTAL CONSTRUCTION JOINTS SHALL BE PERMITTED ONLY WHERE SHOWN ON THE STRUCTURAL DRAWINGS.
7. CONTROL JOINTS FOR SLABS ON GRADE SHALL BE SAW CUT PER THE TYPICAL DETAILS ON THE STRUCTURAL DRAWINGS. DIAMOND LEAVE OUTS SHALL BE PROVIDED AT ALL COLUMNS.
8. CONTRACTOR SHALL SUBMIT PLAN SHOWING POUR SEQUENCE, INCLUDING TYPE AND LOCATION OF PROPOSED JOINTS IN SLABS AND WALLS FOR APPROVAL.
9. ALL CONCRETE PLACED IN COLD WEATHER SHALL CONFORM TO ACI 308-COLD WEATHER CONCRETING. ALL CONCRETE PLACED IN HOT WEATHER SHALL CONFORM TO ACI 305-HOT WEATHER CONCRETING.
10. CONTRACTOR SHALL COORDINATE THE LOCATION OF ALL SLEEVES, INSERTS, ANCHOR BOLTS AND OTHER EMBEDDED ITEMS AS REQUIRED BY OTHER TRADES.
11. TOP SURFACES OF CONCRETE BEAMS TO SUPPORT PRECAST CONCRETE MEMBERS NOT PLACED IN THE FORMS SHALL RECEIVE A TROWELED SMOOTH FINISH AND SHALL BE STRAIGHT, TRUE AND LEVEL.
12. ALL GRADE BEAMS SHALL BE FORMED ON SIDES.
13. ALL CONCRETE POURS SHALL BE TERMINATED BY FORMS. FOOTINGS MAY BE INFORMED PROVIDED THE TRENCH IS EXCAVATED AN ADDITIONAL 1'-0" ON ALL SIDES, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
14. ALL COLUMN FOOTINGS SHALL BE CENTERED UNDER COLUMN CENTERLINES, U.N.O.
15. GROUT ALL LEVELING AND BEARING PLATES WITH AN APPROVED NON-SHRINK GROUT.
16. CHAMFER CORNERS OF ALL EXPOSED CONCRETE AS DETAILED BY THE ARCHITECTURAL DRAWINGS.
17. A THIRD PARTY TESTING LABORATORY SHALL BE EMPLOYED BY THE OWNER FOR EVALUATION AND QUALITY CONTROL OF CONCRETE PLACED. FREQUENCY OF CONCRETE TESTING SHALL MEET THE REQUIREMENTS OF ACI 318 AT A MINIMUM UNLESS OTHERWISE REQUIRED BY THE LOCAL BUILDING CODE.

CONCRETE REINFORCEMENT NOTES:

1. CONCRETE REINFORCING BARS SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A615 GRADE 60, EXCEPT AS NOTED. FIELD BENT #3 DOWELS MAY BE ASTM A615, GRADE 40. REINFORCEMENT REQUIRED TO BE WELDED SHALL CONFORM TO ASTM A706, U.N.O.
2. WELDED WIRE REINFORCING SHALL CONFORM TO ASTM A185. THE FOLLOWING WELDED WIRE REINFORCING SHALL BE USED FOR AREAS SPECIFIED BELOW, UNLESS NOTED OTHERWISE ON THE DRAWINGS:
- | | |
|----------------------|-------------------|
| 5 INCH SLAB-ON-GRADE | 6 X 6 - W2 X W2 9 |
| 6 INCH SLAB-ON-GRADE | 6 X 6 - W2 X W2 9 |
| 8 INCH SLAB-ON-GRADE | 6 X 6 - W2 X W2 9 |
- FOR ADDITIONAL WWF, IF ANY, SEE FLOOR FRAMING PLANS.
3. HEADED STUDS AND DEFORMED BAR ANCHORS USED IN FABRICATION OF EMBEDDED ASSEMBLIES SHALL BE WELDED TO THOSE ASSEMBLIES USING A FULL FUSION PROCESS.
4. REINFORCING BARS MAY BE SPLICED ONLY AS SHOWN ON THE DRAWINGS EXCEPT THAT REINFORCING DESIGNATED AS "CONTINUOUS" SHALL HAVE A CLASS "B" LAP SPICE (ACI 318). LAP SPLICES OF CONTINUOUS REINFORCING SHALL BE MADE OVER SUPPORTS FOR BOTTOM BARS AND FOR INTERMEDIATE BARS AND AT MIDSPAN FOR TOP BARS. AT EXTERIOR SUPPORTS, TOP AND BOTTOM BARS SHALL BE WELDED AND INTERMEDIATE BARS SHALL EXTEND TO WITHIN 2' OF EXTERIOR FACE.
5. HORIZONTAL WALL REINFORCEMENT SHALL BE CONTINUOUS AND SHALL HAVE 90 DEGREE BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL WALL CORNER BAR PLACING DETAILS.
6. LAPS IN WELDED WIRE FABRIC SHALL BE TWO MESH AT SPLICES.
7. PROVIDE STANDARD BAR CHAIRS WITH PROTECTIVE TIPS AND SPACERS SPACED AS REQUIRED TO PROVIDE SPECIFIED CONCRETE PROTECTION FOR REINFORCEMENT BUT NOT TO EXCEED 3'-0" ON CENTER FOR SLABS, BEAMS, AND GRADE BEAMS. PLACE BAR CHAIRS LONGITUDINALLY IN BEAMS DIRECTLY BELOW THE STEEL BARS.
8. PLACE BARS NOTED AS "SECOND LAYER" BELOW THE PRIMARY TOP BARS (OR ABOVE THE PRIMARY BOTTOM BARS) AND PROVIDE A 2" SPACE BETWEEN THE TWO LAYERS.
9. ALL REINFORCING SHALL BE SECURELY WIRED TOGETHER IN THE FORMS PRIOR TO PLACING CONCRETE.
10. THE CONTRACTOR SHALL SUBMIT AND RECEIVE APPROVAL OF REINFORCING STEEL SHOP DRAWINGS PRIOR TO FABRICATION OR SHIPMENT OF MATERIAL. SHOP DRAWINGS SHALL CONTAIN INFORMATION FOR DETAILING, SPLICING, LAPPING, BENDING, SHAPES, QUANTITIES AND DIMENSIONS OF ALL BAR REINFORCEMENT INCLUDING SUPPORTING AND SPACING DEVICES.

SPECIALTY ENGINEERED PRODUCTS:

1. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED SHOP DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. EXTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:
- A. LIGHT GAUGE METAL INCLUDING BUT NOT LIMITED TO: SOFFITS, CLADDING, CEILING, STRUCT, ETC.
- B. MISCELLANEOUS METALS INCLUDING STEEL STAIRS, HAND RAILS, GUARD RAILS & SAFETY RAILS, MECHANICAL EQUIPMENT SUPPORTS, FRAMES THAT SUPPORT MACHINES, PIPES OR OTHER STRUCTURAL METAL USED FOR SUPPORT OF MECHANICAL SYSTEMS.
- C. MISCELLANEOUS HANGERS, CHANDELIERS, CABINETS, METAL FRAMES, LADDERS, RIGGING, HANGING WALLS, RAILINGS, GLAZING FRAMES, CLADDING SUCH AS STONE, PRECAST, ALUMINUM, METAL PANELS, CABLE BARRIER SYSTEMS, ETC. OR ANY OTHER MISCELLANEOUS PRODUCT REQUIRED BY ANY OF THE CONSTRUCTION DOCUMENTS.
- D. IN ADDITION TO THE LOADS SHOWN IN THE DESIGN LOAD SCHEDULE, THE SPECIALTY ENGINEER SHALL DESIGN FOR THE WEIGHT OF ALL MECHANICAL, PLUMBING AND ELECTRICAL EQUIPMENT AND FIXTURES, AS WELL AS CHANDELER FIXTURES, BAR CABINETS, AND ART WORK, MOBILES. GENERAL CONTRACTOR TO INCLUDE IN THEIR BID THE COST OF THE ABOVE NOTED SPECIALTY ENGINEERING.

STEEL NOTES:

1. STRUCTURAL STEEL
1. STRUCTURAL STEEL CONSTRUCTION HAS BEEN DESIGNED IN ACCORDANCE WITH A.I.S.C. 360, "STEEL CONSTRUCTION MANUAL."
2. STRUCTURAL STEEL SHAPES, PLATES, ETC. SHALL CONFORM TO THE FOLLOWING ASTM DESIGNATIONS, U.N.O.
- FLOOR/ROOF BEAMS AND GIRDERS ASTM A992-50
- COLUMNS, WEB DOUBLER PLATES ASTM A992-50
- CHANNELS, TEES, ANGLES, BARS, PLATES, ASTM A36
- STEEL TUBING (HSS SECTIONS) (F_y = 46 KSI) ASTM A500-GR. B
- STEEL PIPE TYPE "E" OR "S" ASTM A501 OR A53
- ANCHOR BOLTS ASTM F1554 GR. 36 ASTM F1554 GR. 55
3. CONNECTION BOLTS SHALL CONFORM TO ASTM A325. USE BEARING TYPE BOLTS WITH THREAD ALLOWED ACROSS THE SHEAR PLANE (TYPE N) AT TYPICAL BEAM SHEAR CONNECTIONS, U.N.O. USE TYPE "SC" BOLTS WITH EITHER DIRECT TENSION INDICATOR OR LOAD INDICATOR WASHERS AT ALL BOLTED SLIP CRITICAL CONNECTIONS.
4. A LISTING OF CONNECTIONS CONSIDERED "SLIP CRITICAL" IS AS FOLLOWS:
- A. BOLTED CONNECTIONS OF TENSION MEMBERS
- B. BOLTS USED IN MOMENT CONNECTIONS
- C. BOLTED SPLICES OF TRUSS TOP AND BOTTOM CHORDS.
5. STEEL BEAM CONNECTIONS NOT DETAILED ON THE DRAWINGS SHALL BE DESIGNED BY THE STRUCTURAL STEEL FABRICATOR. BEAM CONNECTIONS SHALL DEVELOP THE END REACTIONS GIVEN ON THE DRAWINGS. WHERE END REACTIONS ARE NOT SPECIFIED, THE BEAM CONNECTION SHALL DEVELOP 50% OF THE BEAMS WEB ALLOWABLE SHEAR CAPACITY. A MINIMUM CONNECTION CAPACITY OF 12 KIPS SHALL BE PROVIDED FOR ALL BEAMS, UNLESS NOTED OTHERWISE BY SPECIFIED REACTION. THE STRUCTURAL STEEL FABRICATOR SHALL PROVIDE CERTIFICATION BY A PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF THE PROJECT, THAT THE CONNECTION DESIGN IS IN ACCORDANCE WITH ALL APPLICABLE CODES AND SPECIFICATIONS.
6. FOR ALL HIGH STRENGTH BOLTS, HARDENED WASHERS SHALL BE PROVIDED.
7. GALVANIZING OF STEEL MEMBERS SHALL CONFORM TO ASTM A123. GALVANIZE ALL STEEL PERMANENTLY EXPOSED TO WEATHER.
8. ALL STRUCTURAL STEEL SHALL BE SHOP PAINTED WITH A RUST INHIBITIVE PRIMER. DO NOT PRIME STEEL WHICH SHALL HAVE SPRAY ON FIREPROOFING APPLIED. STEEL WHICH IS TO BE FIREPROOFED IS INDICATED ON THE ARCHITECTURAL DRAWINGS. ALL EXPOSED STEEL AND DETAILS IN EXTERIOR WALLS SHALL BE HOT-DIP GALVANIZED.
9. HEADED STUDS AND DEFORMED BAR ANCHORS USED IN FABRICATION OF EMBEDDED ASSEMBLIES SHALL BE WELDED TO THOSE ASSEMBLIES USING A FULL FUSION PROCESS.
10. STEEL BEAMS SHALL BE ERECTED WITH NATURAL CAMBER UP.
11. ANCHOR BOLTS HAVE NOT BEEN DESIGNED FOR ANY SPECIFIC ERECTION FORCES. THE ERECTOR IS RESPONSIBLE FOR ANY AND ALL LIFTING AND BRACING REQUIRED TO ERECT THE BUILDING.
12. THE RESPONSIBILITY FOR ANY TEMPORARY SHORING OR BRACING DURING THE CONSTRUCTION PHASE BEFORE COMPLETION OF CONNECTION AND POURING OF FLOOR SLAB IS ADDRESSED IN THE SPECIFICATIONS AND IS THE RESPONSIBILITY OF THE CONTRACTOR.
13. IF NOT SHOWN ON DRAWINGS, SUPPORT OF METAL DECK AROUND COLUMN CLOSURE, SCREED PLATES AROUND THE OPENINGS AND EDGE SLAB SHALL BE PROVIDED BY THE CONTRACTOR.
14. DURING CONSTRUCTION, THE ERECTED STRUCTURAL STEEL SHALL NOT PROCEED HIGHER THAN THE CONCRETE CORE CONSTRUCTION. THE CONTRACTOR SHALL MAKE SAFE PROVISIONS FOR STABILIZING THE STEEL STRUCTURE BOTH HORIZONTALLY AND VERTICALLY. THE STABILITY OF THE FRAME DURING ERECTION IS THE CONTRACTOR'S RESPONSIBILITY.
15. STRUCTURAL STEEL MEMBERS SHALL NOT BE SPLICED OR HAVE PENETRATIONS UNLESS INDICATED ON THE STRUCTURAL CONTRACT DOCUMENTS OR AS REVIEWED BY THE STRUCTURAL ENGINEER.
16. ALL TUBE STEEL SHALL BE SEAL WELDED. ADEQUATE DRAINAGE HOLES SHALL BE DRILLED TO ELIMINATE WATER TRAPS. THE PREFERRED HOLE DIAMETER, 1/2 INCH, SHALL BE SHOWN ON SHOP DRAWINGS AND SUBJECT TO THE APPROVAL OF THE ARCHITECT.
17. WELDING
1. WELDED CONSTRUCTION SHALL CONFORM TO THE AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE" D1.1, AWS D1.3-SHEET STEEL, AND AWS D1.4 "REINFORCING STEEL WELDING CODE".
2. ELECTRODES FOR FIELD AND SHOP WELDS OF STRUCTURAL STEEL SHALL BE E70XX, U.N.O.
3. ELECTRODES FOR WELDING OF REINFORCING STEEL SHALL BE E80XX.
4. ELECTRODES FOR WELDING OF SHEET STEEL SHALL CONFORM TO AWS D1.3.
5. WHEN WELDS ARE NOT CALLED-OUT ON DRAWINGS, THEY ARE MINIMUM SIZE CONTINUOUS FILLET WELDS IN ACCORDANCE WITH AWS D1.1. FILLET WELDS NOT SPECIFIED AS TO LENGTH SHALL BE CONTINUOUS.
6. UNLESS NOTED OTHERWISE ON THE DRAWINGS, ALL GROOVE WELDS SHALL BE FULL PENETRATION.
7. ONLY LOW HYDROGEN ELECTRODES SHALL BE USED ON REINFORCING STEEL AND ASTM A992 STEEL.
8. PROVIDE FILLET WELDS AT ALL CONTACT JOINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE STRENGTH OF THE SMALLER MEMBER AT THE JOINT UNLESS DETAILED OTHERWISE ON THE DRAWINGS.

METAL DECK NOTES:

1. COMPOSITE METAL DECK SHALL BE GALVANIZED AND SHALL BE PLACED WITH CONTINUOUS SPANS OF THREE OR MORE WHERE POSSIBLE. IN NO CASE SHALL UNHORED METAL DECK SPANS EXCEED THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS OR DEFLECTION CRITERIA OF SPAN DIVIDED BY 240. METAL DECK SHALL PROVIDE THE FOLLOWING MINIMUM PROPERTIES:
- TYPICAL FLOOR AND LOW ROOF COMPOSITE METAL DECK
- 1 1/2"-20 GA. (50 KSI)
P = 0.217 IN/4FT.
SP = 0.229 IN/4FT.
SN = 0.197 IN/4FT.
SN = 0.224 IN/3FT.
2. FLOOR DECK MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL FLOOR DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS. DECK UNITS SHALL BE LAPPED ONLY OVER SUPPORTS.
3. SHORING SHALL REMAIN UNDER THOSE DECK UNITS REQUIRING SHORING UNTIL CONCRETE CYLINDER COMPRESSIVE STRENGTH IS A MINIMUM OF 2500 PSI.
4. COMPOSITE SHEAR STUDS TO BE WELDED THROUGH STEEL DECK. STUD WORK TO BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE NELSON STUD WELDING. SHEAR STUDS MUST COMPLY WITH ASCE A 106 (F_y=50 KSI).
5. ROOF DECK OVER STEEL BEAMS SHALL BE WIDE RIBBED AND GALVANIZED METAL ROOF DECK WITH THE FOLLOWING MINIMUM PROPERTIES:
- 1.58"-20 GA. (33 KSI)
P = 0.197 IN/4FT.
SP = 0.224 IN/4FT.
SN = 0.217 IN/4FT.
SN = 0.229 IN/3FT.
6. MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE. ROOF DECK MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.
7. WELD DECK TO SUPPORTING MEMBERS ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. DECK AND WELDS SHALL HAVE A MINIMUM DESIGN DAPPHRAGM SHEAR CAPACITY OF 300 PLF AND 364 (1) FASTENING PATTERN MINIMUM.
8. ALL ROOF DECK TO BE DESIGNED, MANUFACTURED, AND INSTALLED IN ACCORDANCE WITH LATEST FACTORY MUTUAL STANDARDS.
9. WELDING WASHERS ARE TO BE USED ON ALL CONNECTIONS OF STEEL DECK WITH METAL THICKNESS LESS THAN 22 GA. TO STRUCTURAL STEEL SUPPORTS.
10. IN AREAS OF WARPED ROOF DECK, SELF DRILLING SCREWS ARE TO BE USED ON CONNECTIONS OF STEEL ROOF DECK TO STRUCTURAL STEEL SUPPORTS. SCREW SIZES TO COMPLY WITH MANUFACTURERS AND FACTORY MUTUAL REQUIREMENTS. ATTACH DECK TO ALL SUPPORTING ROOF JOISTS.
11. LOADS EXCEEDING 50 LBS SHALL NOT BE PERMITTED TO BE HUNG FROM ANY METAL DECKING. ALL HANGERS FOR PIPING, DUCTWORK, CABLE TRAYS, ETC. SHALL BE HUNG DIRECTLY FROM STRUCTURAL STEEL MEMBERS AND/OR ANCHORS EMBEDDED DIRECTLY INTO THE CONCRETE.

CONCRETE MASONRY UNITS NOTES:

1. ALL CONCRETE MASONRY WALLS AND COLUMNS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENT FOR MASONRY STRUCTURES" (ACI 530/ASCE 5/TMS 402) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TSM 602) LATEST EDITIONS.
2. ALL MASONRY UNITS SHALL BE ASTM C80, TYPE 1 MEDIUM WEIGHT WITH MINIMUM COMPRESSIVE STRENGTH OF 1,900 PSI AT 28 DAYS ON THE NET AREA OF INDIVIDUAL UNITS. ALL CMU SHALL BE ERECTED IN RUNNING BOND, ON FULL MORTAR BEDS, UNLESS OTHERWISE NOTED.
3. CMU MORTAR SHALL BE PORTLAND CEMENT-LIME CONFORMING TO ASTM C270. USE TYPE "M" FOR MASONRY BELOW GRADE AND USE TYPE "M" OR "S" FOR ALL INTERIOR AND EXTERIOR WALLS ABOVE GRADE.
4. CMU GROUT, POURED OR PUMPED, SHALL MEET ASTM C476, AND HAVE MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.
5. PROVIDE HOT DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT (9 GA.) AT 16" ON CENTER VERTICAL IN ALL MASONRY WALLS WITH PREFABRICATED "T" AND CORNER PIECES. LAP SPLICED 6" MIN. PROVIDE AN ADDITIONAL ROW ABOVE AND BELOW OPENINGS AND EXTEND 2'-0" BEYOND JAMBS. STOP HORIZONTAL REINFORCING EACH SIDE OF CONTROL AND EXPANSION JOINTS. HORIZONTAL JOINT REINFORCING SHALL MEET ASTM A62.
6. SEE STRUCTURAL DRAWINGS FOR REINFORCING OF LOADBEARING CMU WALLS. ALL CMU WALLS 8" OR WIDER SHALL HAVE THE FOLLOWING MINIMUM REINFORCING UNLESS NOTED OTHERWISE: INTERIOR WALLS -#4 AT 48" O.C.; EXTERIOR WALLS -#5 AT 32" O.C.
7. IN ADDITION TO REINFORCING STEEL NOTED ON PLANS, SECTIONS, AND DETAILS, PROVIDE VERTICAL BARS (#4 MIN. U.N.O.) WITHIN 8" OF EACH SIDE OF WALL CONTROL JOINTS, WITHIN 8" OF THE ENDS OF WALLS, WITHIN 16" OF EACH SIDE OF OPENINGS, AND AT ALL CORNERS. DOWELS TO MATCH VERTICAL REINF. WHERE CMU BEARS ON EXISTING SLAB, PROVIDE DOWEL TO EXISTING SLAB WITH ADHESIVE ANCHOR SYSTEM PER TYP. DETAILS (3 MIN. EMBED.).
8. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF NON-LOAD BEARING CMU WALLS. BRACING SHALL BE PROVIDED AT THE TOP OF CMU WALLS TO RESIST 2 PSF OF LATERAL PRESSURE AND TO ALLOW FOR VERTICAL DEFLECTION OF THE STRUCTURE ABOVE.
9. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF MASONRY CONTROL JOINTS. IF NOT INDICATED ON THE DRAWINGS, CONCRETE MASONRY SHALL HAVE VERTICAL CONTROL JOINTS WITH A MAXIMUM SPACING OF 32'-0" O.C. AT EXTERIOR WALLS BUT NOT LESS THAN 8' FROM BEARING WALLS.
10. CONTROL JOINTS SHALL ALSO BE LOCATED AT ABRUPT CHANGES IN WALL HEIGHT, CHANGES IN WALL THICKNESS, WALL CORNERS, INTERSECTIONS OF WALLS WITH COLUMNS, PIERS AND PILASTERS, AND NO CLOSER THAN 2'-0" TO EDGE OF ANY WALL OPENING. REINFORCING BARS IN BOND BEAMS AT ROOF AND FLOOR ELEVATIONS SHALL BE CONTINUOUS ACROSS CONTROL JOINTS. WHERE DISCREPANCIES EXIST BETWEEN THE ARCH. DRAWINGS AND CONTROL JOINT SPACING REQUIREMENTS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND ENGINEER TO RESOLVE THE DISCREPANCY.
11. SPECIAL INSPECTION IS REQUIRED FOR CMU LOADBEARING WALLS.
12. ALL CORES THAT CONTAIN VERTICAL REINFORCING SHALL BE FILLED SOLID WITH GROUT. ALL CMU PIERS SHALL BE COMPLETELY FILLED WITH GROUT. FILLING CORES WITH MORTAR AS WORK PROCEEDS IS NOT ACCEPTABLE.
13. ALL CMU THAT HAS ONE OR MORE FACES BELOW GRADE SHALL BE GROUTED SOLID.
14. THE MINIMUM SPICE LENGTH FOR ALL VERTICAL BARS IS 48 BAR DIAMETERS UNLESS OTHERWISE NOTED. LAP SPLICES SHALL OCCUR DIRECTLY ABOVE FOOTINGS AND SLABS. NO SPLICES ARE ALLOWED AT MID-HEIGHT OF WALL.
15. REINFORCING BAR POSITIONERS SHALL BE USED TO HOLD BARS IN THE PROPER LOCATION. POSITIONERS SHALL BE PLACED AT A MAXIMUM VERTICAL SPACING OF 48" O.C.
16. ALL UNITS SHALL BE CONTINUOUSLY PROTECTED FROM EXPOSURE TO RAIN OR OTHER SOURCES OF WATER FROM TIME OF CASTING TO FINAL PLACEMENT OF THE WALL. ALL CONCRETE MASONRY UNITS SHALL BE DRY, FREE FROM SOIL, ICE, AND FROST WHEN LAID IN THE WALL.
17. LAY NO MASONRY WHEN THE TEMPERATURE OF OUTSIDE AIR IS BELOW 40 DEGREES FAHRENEIT, UNLESS SUITABLE MEANS ARE PROVIDED TO KEEP MATERIALS PROTECT WORK FROM COLD AND FROST, AND ENSURE THAT THE MORTAR WILL HARDEN WITHOUT FREEZING. COMPLY WITH THE COLD WEATHER REQUIREMENTS CONTAINED IN ACI 530.1.
18. ALL BEAMS SUPPORTING MASONRY, INCLUDING STEEL, PRECAST, AND MASONRY UNITS ARE TO BEAR ON 8" MIN. AND ON 3 COURSES GROUTED SOLID, MINIMUM.
19. UNLESS NOTED OTHERWISE, PROVIDE ANCHORAGE OF MASONRY WALLS TO THE STRUCTURE IN THE FOLLOWING MANNER:
- a. AT STEEL BEAMS - ADJUSTABLE MASONRY ANCHORS AT 16"
- b. AT STEEL COLUMNS - ADJUSTABLE MASONRY ANCHORS AT 16"
- c. AT CONCRETE ELEMENTS - LOVELET ANCHORS AT 16"
20. SPACE MASONRY VENER ANCHORS AT A MAXIMUM OF 32" HORIZONTALLY AND 16" VERTICALLY. PROVIDE ADDITIONAL ANCHORS AROUND OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS AROUND PERIMETER OF OPENING AT A MAXIMUM OF 32" O.C. PLACE ANCHORS WITHIN 12" OF OPENINGS.
21. ALL LOOSE ANGLE UNITS SHALL HAVE A MINIMUM BEARING OF 6" ON MASONRY. WHERE LOOSE ANGLES ARE EXPOSED TO WEATHER, ANGLES SHALL BE GALVANIZED. LOOSE UNITS SHALL BE PROVIDED AT OPENINGS 12" WIDE OR LARGER. UNLESS NOTED OTHERWISE, WIDTH OF UNTIL SHALL BE 1/2" LESS THAN WIDTH OF MASONRY BEARING SUPPORT (5/16" THICKNESS).
22. INSTALL FLASHING AS REQUIRED BY THE ARCH. DRAWINGS.

DEMOLITION NOTES:

1. REMOVAL AS DESCRIBED HEREIN SHALL BE ACCOMPLISHED WITHOUT STORING ON THE FLOOR EXCESSIVE QUANTITIES OF ANY MATERIALS, RUBBISH, DIRT, DEBRIS, OR WASTE OF ANY SORT RESULTING FROM THE REMOVAL OPERATIONS ON THE FLOOR.
2. ALL DEBRIS SHALL BE REMOVED FROM THE CONSTRUCTION SITE DAILY.
3. THE CONTRACTOR SHALL TAKE PRECAUTIONS TO MAINTAIN FREE PROTECTED ACCESS OF ALL TENANTS, SERVICE PERSONNEL AND THE PUBLIC THROUGH THE AREAS INVOLVED.
4. THE CONTRACTOR SHALL REMOVE ALL PIPE SLEEVES PROJECTING THROUGH SLAB, PATCH ALL PENETRATIONS, HOLES, ETC.
5. ALL PIPES AND CONDUITS IN WALLS THAT ARE TO BE DEMOLISHED ARE TO BE REMOVED AND/OR RELOCATED AS REQUIRED.
6. CONTRACTOR SHALL REVIEW WITH ARCHITECT'S/ENGINEER ANY AND ALL ITEMS OF DEMOLITION NOT IMPLIED OR SPECIFIED ON DRAWINGS OR SPECIFICATIONS AND TO INCLUDE SUCH COSTS IN BID UNLESS OTHERWISE ADVISED.
7. PROVIDE ALL LABOR, MATERIAL, EQUIPMENT AND SERVICES AND PERFORM ALL OPERATIONS REQUIRED FOR COMPLETE INTERIOR DEMOLITION AND RELATED WORK AS DESCRIBED AND SPECIFIED HEREIN, AND AS MAY BE REASONABLY IMPLIED AS NECESSARY TO COMPLETE WORK IN ALL RESPECTS.
8. JOBSITE INSPECTION MUST BE CONDUCTED TO EXAMINE EXISTING CONDITIONS, TO DETERMINE NATURE AND SCOPE OF WORK OR ANY DIFFICULTIES THAT MIGHT ARISE AT TIME OF WORK. IN ADDITION, EXAMINE ALL WORK THAT IS INTENDED TO REMAIN AS PART OF THE COMPLETED PROJECT AND REPORT ALL UNSATISFACTORY CONDITIONS TO ARCHITECT/ENGINEER PRIOR TO COMMENCEMENT OF WORK. EXERCISE EXTREME CARE DURING DEMOLITION SO AS NOT TO DAMAGE CONSTRUCTION AND OTHER STRUCTURES THAT ARE INTENDED TO REMAIN. ANYTHING DAMAGED AT WORK IS TO BE REPAIRED AND/OR REPLACED TO MATCH EXISTING CONSTRUCTION AT CONTRACTORS EXPENSE.
9. REFER TO DRAWINGS FOR EXISTING ITEMS/ SYSTEMS TO REMAIN.
10. CONTRACTOR TO PROVIDE DUST BARRIER FOR PROTECTION OF EXISTING AREAS TO REMAIN AS REQUIRED.
11. WHEN DEMOLITION TAKES PLACE, SHOULD ANY WORK AFFECT THE INTEGRITY OF THE STRUCTURE, WORK MUST STOP IMMEDIATELY, AND ARCHITECT/ENGINEER NOTIFIED. UNDER NO CIRCUMSTANCES SHALL REINFORCING OF ANY KIND BE DAMAGED, CUT OR BROKEN.
12. THE GENERAL CONTRACTOR SHALL PROVIDE SUFFICIENT FRAMING FOR ALL WALL OPENINGS FOR DUCTWORK, RETURN AIR OPENINGS, ACCESS PANELS AND GRILLE OPENINGS ABOVE AND BELOW HUNG CEILING. THESE ARE TO BE COORDINATED WITH H.V.A.C. ENGINEERING DRAWINGS AND THE GENERAL CONTRACTOR'S SHOP DRAWINGS AND THE GENERAL CONTRACTOR'S MECHANICAL CONTRACTOR'S SHOP DRAWINGS. ALL SPACES SHALL BE PROPERLY SEALED FOR SOUNDPROOFING AND VIBRATION.
13. PRIOR TO DEMOLITION OF LOAD BEARING MEMBERS, SUPPORTED MEMBERS SHALL BE SHORED.

MISCELLANEOUS NOTES:

1. THE DETAILS DESIGNATED AS "TYPICAL DETAILS" APPLY GENERALLY TO THE DRAWINGS IN AREAS WHERE CONDITIONS ARE SIMILAR TO THOSE DESCRIBED IN THE DETAILS, UNLESS NOTED OTHERWISE.
2. ALL DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS, AND DETAILS. DO NOT SCALE THE DRAWINGS.
3. THE STRUCTURAL DRAWINGS ARE PART OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES PROVIDE ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL CONSULT THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND COORDINATE THE INFORMATION CONTAINED IN THESE DRAWINGS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT. PRINCIPAL OPENINGS, CURBS, AND SLAB DEPRESSIONS ARE SHOWN ON THE DRAWINGS. SEE ARCHITECTURAL, MECH., ELEC., AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS, OTHER OPENINGS, AND SLAB DEPRESSIONS NOT SHOWN. THE CONTRACTOR SHALL PROVIDE FOR ALL OPENINGS, CURBS, AND SLAB DEPRESSIONS WHETHER SHOWN ON STRUCTURAL DRAWINGS OR NOT. SIZE AND LOCATION OF OPENINGS SHALL BE VERIFIED WITH THE MECHANICAL CONTRACTOR. ANY DEVIATION FROM OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER FOR APPROVAL PRIOR TO PROCEEDING WITH ANY WORK.
4. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL DRAWINGS WITH THE ARCH. MECH., ELEC., PLUMBING, AND CIVIL DRAWINGS TO CONFIRM ALL REQUIREMENTS OF THE WORK. REPORT ANY CONFLICTS/INCONSISTENCY BETWEEN THE DISCIPLINES TO THE ARCHITECT PRIOR TO FABRICATING OR INSTALLING STRUCTURAL ELEMENTS. BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS TO PROPERLY SIZE OR FIT THE WORK. NO EXTRA CHARGE OR COMPENSATION WILL BE ALLOWED BY THE OWNER RESULTING FROM THE CONTRACTOR'S FAILURE TO COMPLY WITH THIS REQUIREMENT.
5. THE HORIZONTAL AND VERTICAL DIMENSIONS OF EXISTING STRUCTURES SHALL BE VERIFIED BEFORE WORK IS BEGUN. ANY VARIATION BETWEEN DIMENSIONS SHOWN AND EXISTING DIMENSIONS SHALL BE REPORTED TO THE ARCHITECT.
6. THE CONTRACTOR SHALL INSURE THAT CONSTRUCTION LOADS DO NOT EXCEED THE DESIGN LIVE LOADS INDICATED ON THE STRUCTURAL DRAWINGS AND THAT THESE LOADS ARE NOT PUT ON THE STRUCTURAL MEMBERS PRIOR TO THE TIME THAT THE CONCRETE REACHES THE FULL DESIGN STRENGTH AND ALL FRAMING MEMBERS AND THEIR CONNECTIONS ARE IN PLACE.
7. ALL STRUCTURES HAVE BEEN DESIGNED TO RESIST THE DESIGN LOADS LISTED ONLY AS COMPLETED STRUCTURES. THE GENERAL CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT WORK IN PROGRESS UNTIL THE STRUCTURES ARE COMPLETED. THE GENERAL CONTRACTOR SHALL ALSO INSURE THAT ITS OPERATIONS AND PROCEDURES PROVIDE NO LOADING GREATER THAN THE DESIGN LOADS LISTED ON ANY MEMBER.
8. PROVIDE CHAMFERS AS SPECIFIED AND/OR DETAILED ON THE ARCHITECTURAL DRAWINGS. CHAMFERS HAVE NOT BEEN SHOWN ON THE STRUCTURAL DRAWINGS.
9. ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

SUBMITTAL AND SHOP DRAWING REQUIREMENT NOTES:

1. THE CONTRACTOR SHALL SUBMIT FOR REVIEW BY THE ARCHITECT AND THE ENGINEER ALL INFORMATION REQUIRED BY THE CONTRACT DOCUMENTS INCLUDING THE SPECIFICATIONS.
2. SHOP DRAWINGS SHALL BE PREPARED, SUBMITTED AND REVIEWED PRIOR TO PROCEEDING WITH FABRICATION AND/OR INSTALLATION OF THE ASSOCIATED WORK. REVIEW PERIOD SHALL BE A MINIMUM OF TWO (2) WEEKS.
3. THE CONTRACTOR SHALL SUBMIT FOR REVIEW, DRAWINGS AND CALCULATIONS FOR ALL PERFORMANCE ASSEMBLIES IDENTIFIED IN THE GENERAL NOTES AND LISTED BELOW. THE DESIGN OF THESE ASSEMBLIES IS THE RESPONSIBILITY OF THE CONTRACTOR'S ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION. ALL SUBMITTALS SHALL BEAR CONTRACTOR'S ENGINEER'S SEAL AND SIGNATURE. REVIEW SHALL BE FOR GENERAL CONFORMANCE WITH THE PROJECT REQUIREMENTS AS INDICATED ON THE DRAWINGS AND IN GENERAL NOTES.
- A. NON-LOAD BEARING STUD WALL AND CURTAIN WALL SYSTEMS AND RELATED CONNECTIONS
- a. DESIGN SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CODES. BACK-UP SYSTEM AND STUD WALLS SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION OF 1/800 OF THE SPAN, OR 3/8" WHICHEVER IS LESS. AT THE APPLICABLE DESIGN WIND LOAD, CURTAIN WALLS SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION AS INDICATED BY ASCE REQUIREMENTS UNLESS OTHERWISE NOTED IN SPECIFICATIONS.
- B. METAL STAIRS AND METAL RAILINGS
- a. DESIGNS SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CODES. WHEREAS HEADERS OR OTHER TYPES



1 DATUM ELEVATION OF "D" = 140'-0"

2 USE 50 SERIES FOR GENERAL NOTES AND 83 SERIES FOR TYPICAL DETAILS.

3 "X" INDICATES TOP OF EXISTING FOOTING ELEV. TOP OF ALL EXISTING FOOTING ELEV. = "+X" U.N.O. TOP OF ALL EXISTING

4 FOOTING ELEV. = "+X" U.N.O.

5 BOTTOMS OF ALL PERMITS FOOTINGS SHALL BE 3'-0" MIN. BELOW GRADE.

6 NEW FOOTING ELEVATIONS SHALL MATCH THE EXISTING ADJACENT FOOTING ELEVATIONS. VERIFY FOOTING ELEVATIONS IN FIELD

7 AND AT ALL PERMITS.

8 COORDINATE ALL SLAB ON GRADE RECESSES, CURBS, AND LEDGES WITH THE REQUIREMENTS OF THE ARCH. DWGS.

9 VERIFY ALL DIMENSIONS OF RECESSES AND LEDGES WITH THE ARCH. DWGS.

10 VERIFY SLAB DIMENSIONS WITH ARCH. DWGS. REFER TO ARCH. DRAWINGS FOR SLAB EDGE DIMENSIONS NOT INDICATED.

11 PERIMETER FINISHES SHALL MATCH EXISTING PERIMETER FINISHES.

12 CONCRETE SHALL BE CAST IN PLACE. ALL CONCRETE SHALL BE PLATE CAST. U.N.O. TO PERIOR ELEVATION SHALL = "-6" U.N.O.

13 ALL DIMENSIONS SHALL BE IN FEET AND INCHES. ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE NOTED.

14 ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE NOTED.

15 ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE NOTED.

16 ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE NOTED.

17 ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE NOTED.

18 ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE NOTED.

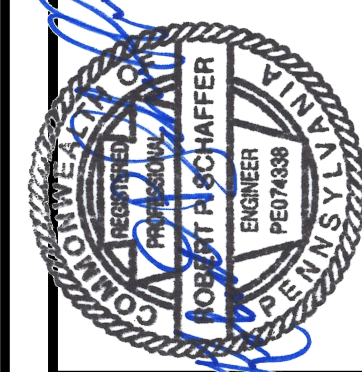
19 ALL DIMENSIONS SHALL BE TO THE CENTERLINE UNLESS OTHERWISE NOTED.



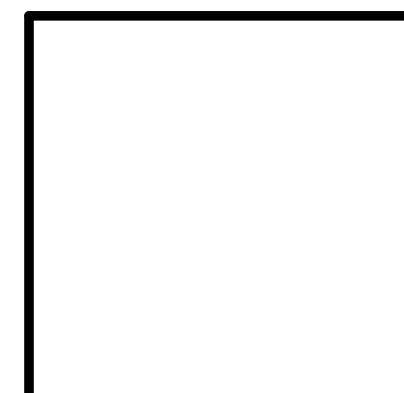
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CONSTRUCTION
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SCALE: AS NOTED

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KIMMEL BOGRETTE

Architecture + Site

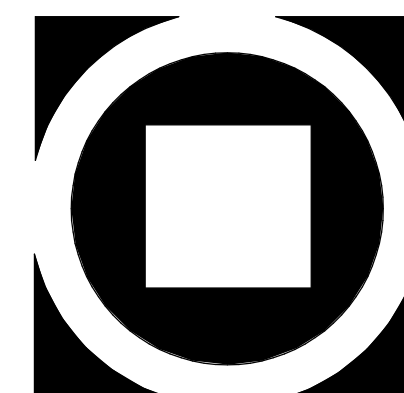
Blue Bell PA 19422

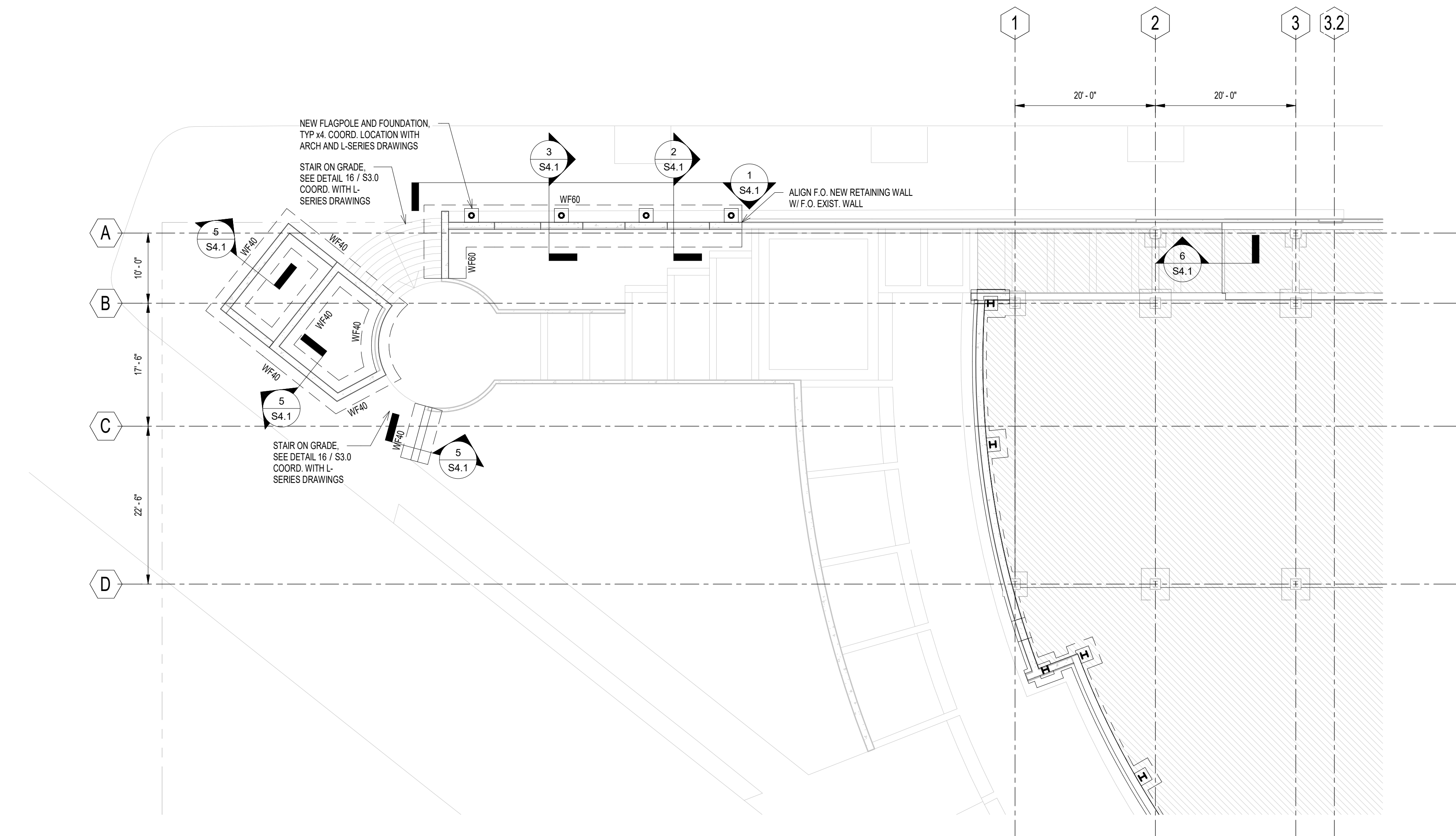
182 Norristown Rd., Suite 200

Phone: 610.834.7805

Facsimile: 610.834.7819

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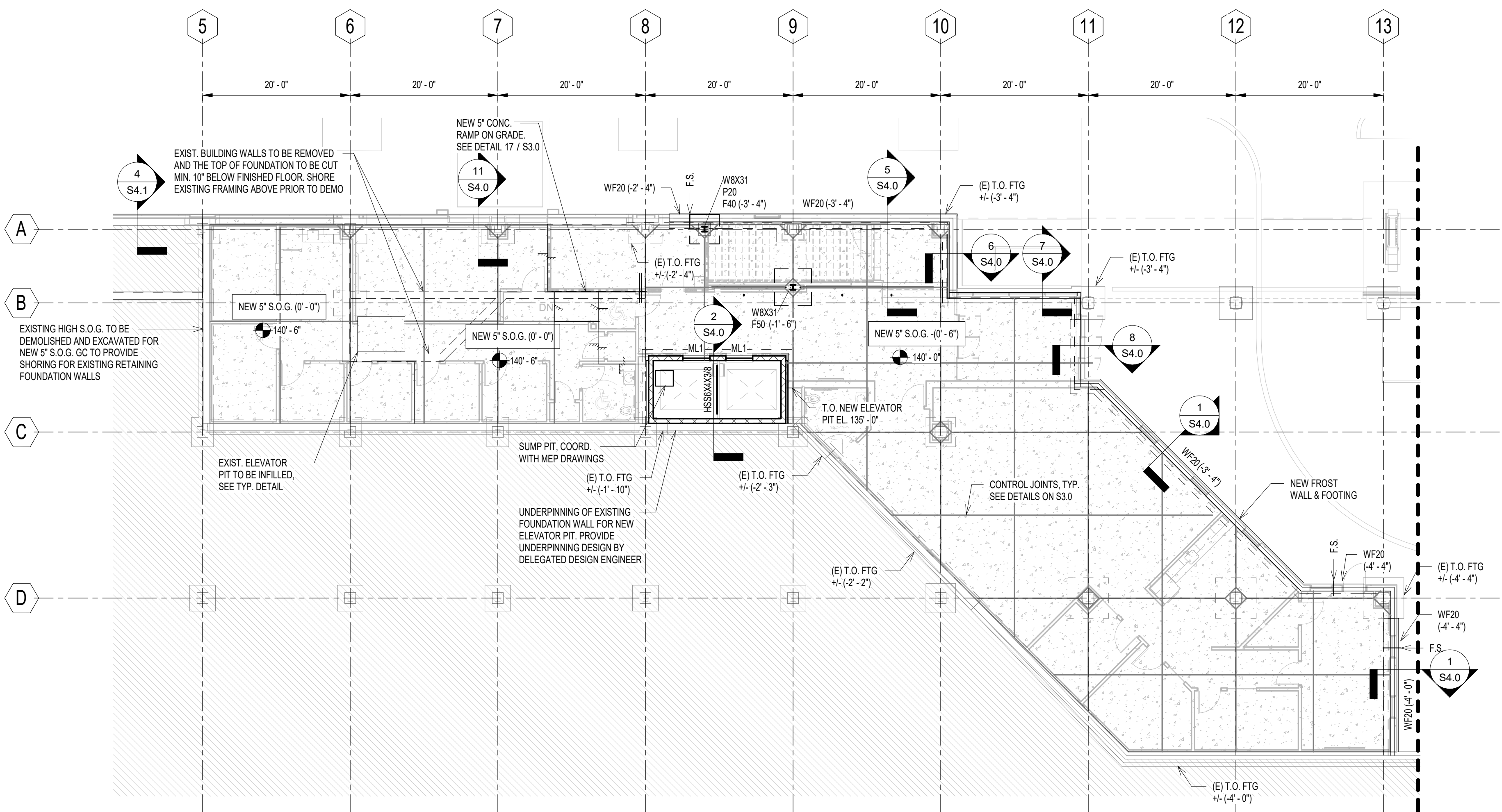




1 SITE WALLS PART PLAN
3/32" = 1'-0"

FOUNDATION PLAN NOTES:

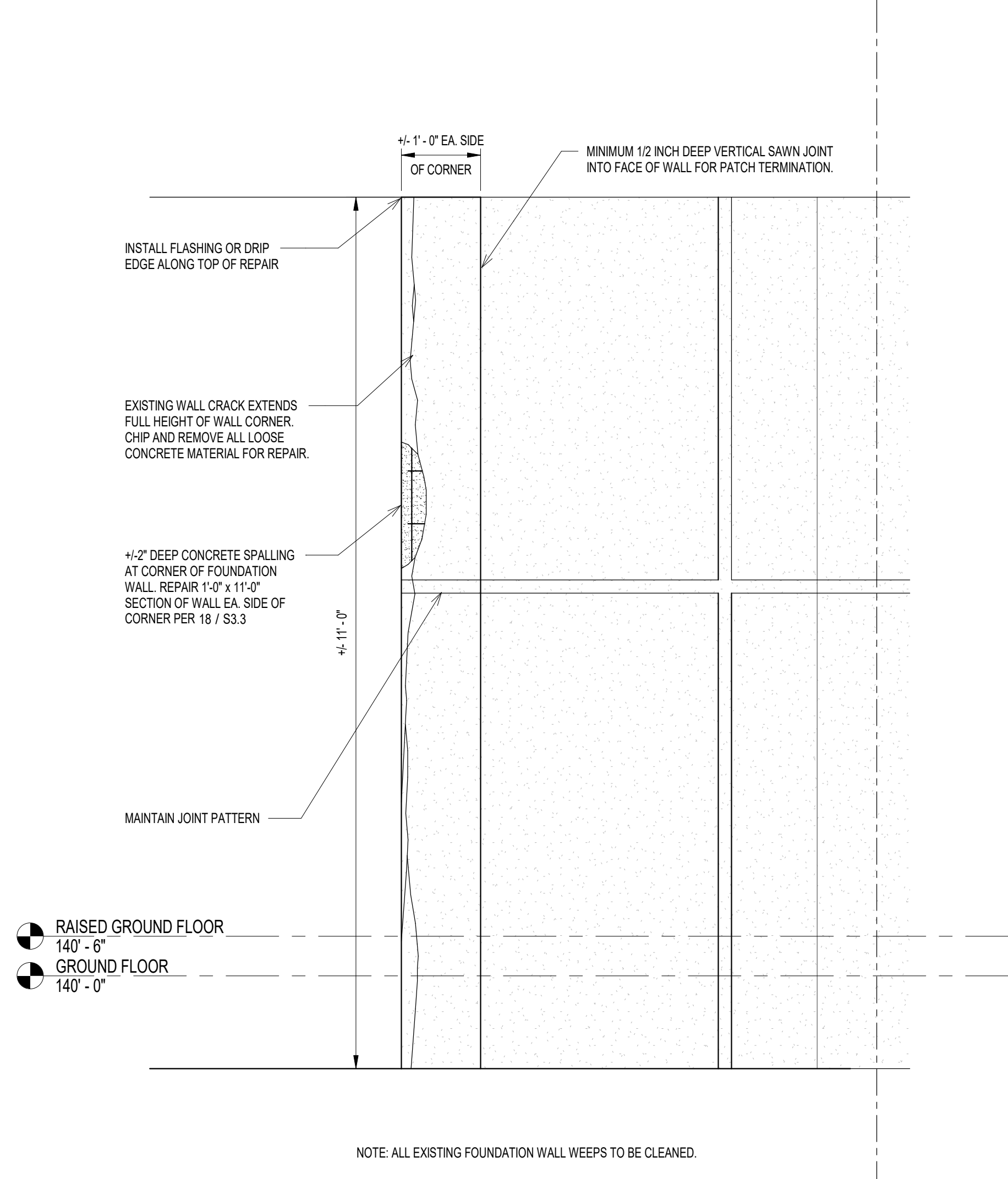
- DATUM ELEVATION 0'-0" = 140'-0"
- SEE S3 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- [X-XX] INDICATES TOP OF FOOTING ELEVATION. TOP OF ALL INTERIOR FOOTING ELEV. = -1'-0" U.N.O. TOP OF ALL EXTERIOR FOOTING ELEV. = -2'-0" U.N.O.
- BOTTOMS OF ALL PERIMETER FOOTINGS SHALL BE 3'-0" MIN. BELOW GRADE.
- NEW FOOTING ELEVATIONS SHALL MATCH THE EXISTING ADJACENT FOOTING ELEVATIONS. VERIFY FOOTING ELEVATIONS IN FIELD.
- SLAB ON GRADE SHALL BE REINFORCED PER THE GENERAL NOTES.
- COORDINATE ALL SLAB ON GRADE RECESSES, CURBS, AND LEDGES WITH THE REQUIREMENTS OF THE ARCH. DWGS.
- COORDINATE ALL SIZES AND LOCATIONS OF HOUSE KEEPING PADS WITH THE ARCH. AND MECH. DWGS.
- VERIFY SLAB DIMENSIONS WITH ARCH. DWGS. REFER TO ARCH. DRAWINGS FOR SLAB EDGE DIMENSIONS NOT INDICATED.
- PX INDICATES PIER CALL OUT. BPX INDICATES BASE PLATE CALL OUT. T.O. PIER ELEVATION SHALL = -0'-0" U.N.O.
- FOUNDATIONS, COLUMNS, AND PIERS ARE CENTERED ON THE GRID LINES. U.N.O.; ALL BEAMS TO BE EQUALLY SPACED BETWEEN COLUMN LINES. U.N.O.
- ALL EXISTING FOUNDATION WALL WEEPS TO BE CLEANED.



2 GROUND FLOOR LOBBY PART PLAN
3/32" = 1'-0"

FOUNDATION PLAN NOTES:

- DATUM ELEVATION 0'-0" = 140'-0"
- SEE S3 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- [X-XX] INDICATES TOP OF FOOTING ELEVATION. TOP OF ALL INTERIOR FOOTING ELEV. = -1'-0" U.N.O. TOP OF ALL EXTERIOR FOOTING ELEV. = -2'-0" U.N.O.
- BOTTOMS OF ALL PERIMETER FOOTINGS SHALL BE 3'-0" MIN. BELOW GRADE.
- NEW FOOTING ELEVATIONS SHALL MATCH THE EXISTING ADJACENT FOOTING ELEVATIONS. VERIFY FOOTING ELEVATIONS IN FIELD.
- SLAB ON GRADE SHALL BE REINFORCED PER THE GENERAL NOTES.
- COORDINATE ALL SLAB ON GRADE RECESSES, CURBS, AND LEDGES WITH THE REQUIREMENTS OF THE ARCH. DWGS.
- COORDINATE ALL SIZES AND LOCATIONS OF HOUSE KEEPING PADS WITH THE ARCH. AND MECH. DWGS.
- VERIFY SLAB DIMENSIONS WITH ARCH. DWGS. REFER TO ARCH. DRAWINGS FOR SLAB EDGE DIMENSIONS NOT INDICATED.
- PX INDICATES PIER CALL OUT. BPX INDICATES BASE PLATE CALL OUT. T.O. PIER ELEVATION SHALL = -0'-0" U.N.O.
- FOUNDATIONS, COLUMNS, AND PIERS ARE CENTERED ON THE GRID LINES. U.N.O.; ALL BEAMS TO BE EQUALLY SPACED BETWEEN COLUMN LINES. U.N.O.
- ALL EXISTING FOUNDATION WALL WEEPS TO BE CLEANED.



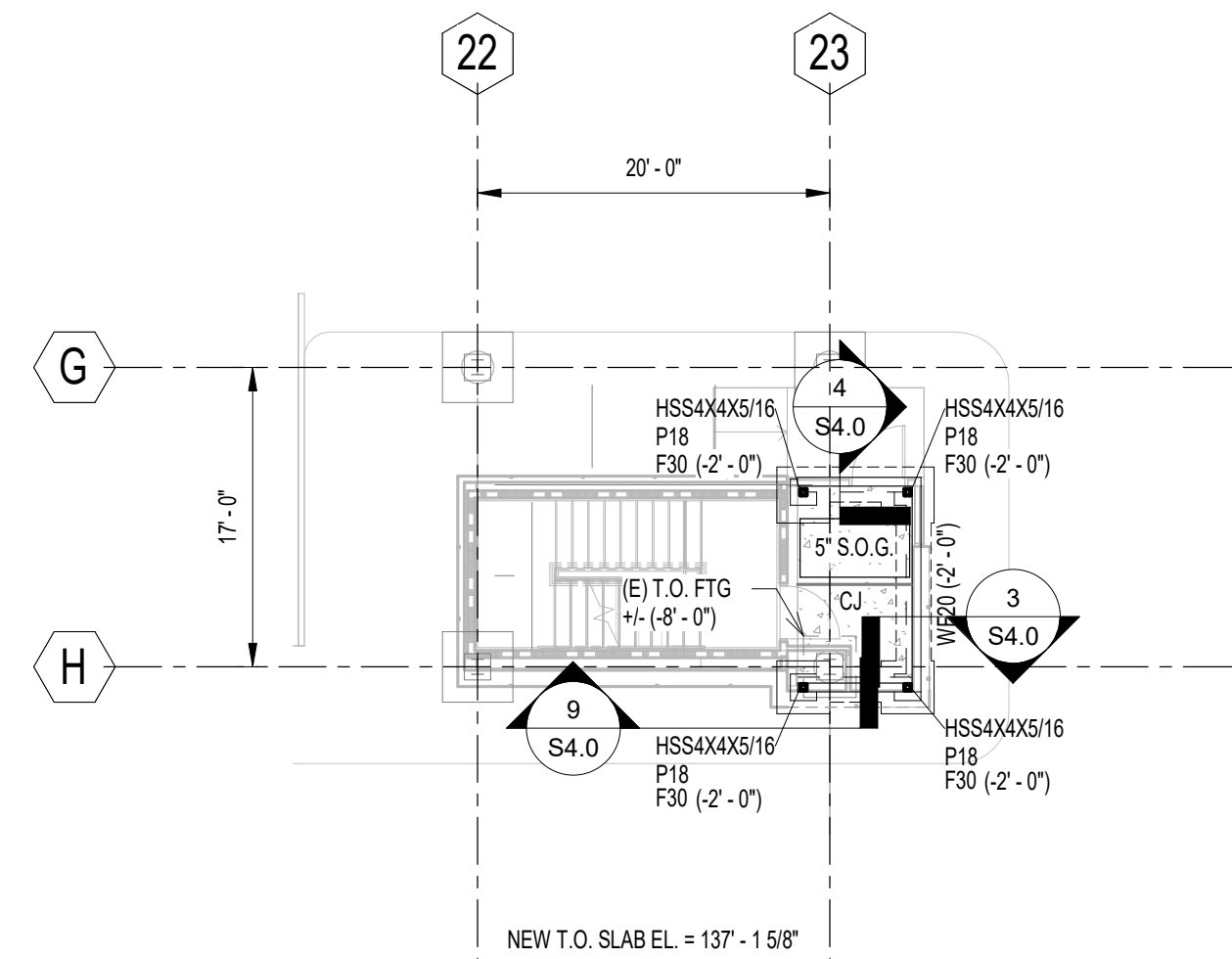
4 FOUNDATION WALL REPAIR
3/4" = 1'-0"

WALL FOOTING SCHEDULE				
MARK	DIMENSIONS		CONTINUOUS	TRANSVERSE
	WIDTH	DEPTH	REIN.	REIN.
WF20	2'-0"	1'-0"	2#5	#5 @ 12
WF60	6'-0"	1'-2"	6#5 TOP & BOT.	#5 @ 12

PIER SCHEDULE				
MARK	DIMENSIONS	VERT. REIN.	TIES	
P18	1'-0"X1'-0"	8#5	#3@8"	
P20	1'-0"X1'-0"	8#7	#3@8"	
P24	2'-0"X2'-0"	8#7	#3@8"	

SPREAD FOOTING SCHEDULE				
MARK	DIMENSIONS			REINFORCEMENT
	WIDTH	LENGTH	DEPTH	(EACH WAY BOTTOM, U.N.O.)
F30	3'-0"	3'-0"	1'-0"	(4)-#4 BOT.
F40	4'-0"	4'-0"	1'-0"	(5)-#4 BOT.
F50	5'-0"	5'-0"	1'-0"	(6)-#5 BOT.

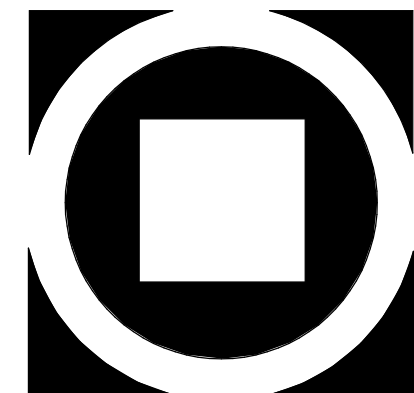
BASE PLATE SCHEDULE					
MARK	COLUMN	B (in)	D(in)	t(in)	ANCHOR BOLTS
BP1	HSS4X4	12	12	3/4	(4)-3/4" DIA ASTM GRADE 36 (0'-0" EMBED)
BP2	W18X31	16	16	3/4	(4)-3/4" DIA ASTM GRADE 36 (0'-0" EMBED)
BP3	W10X49	18	18	3/4	(4)-3/4" DIA ASTM GRADE 36 (0'-0" EMBED)
BP4	HSS10X6	12	12	3/4	(2)-3/4" DIA H.L.TI KWIK-HUS-EZ (0'-6 1/4" EMBED) SEE DETAIL 14 / S4.2



3 GROUND FLOOR VESTIBULE PART PLAN
3/32" = 1'-0"

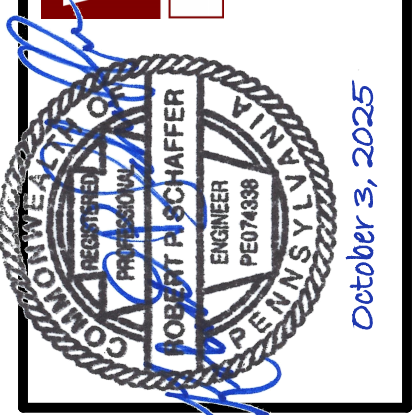
FOUNDATION PLAN NOTES:

- DATUM ELEVATION 0'-0" = 137'-1 5/8"
- SEE S3 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- [X-XX] INDICATES TOP OF FOOTING ELEVATION. TOP OF ALL INTERIOR FOOTING ELEV. = -1'-0" U.N.O. TOP OF ALL EXTERIOR FOOTING ELEV. = -2'-0" U.N.O.
- BOTTOMS OF ALL PERIMETER FOOTINGS SHALL BE 3'-0" MIN. BELOW GRADE.
- NEW FOOTING ELEVATIONS SHALL MATCH THE EXISTING ADJACENT FOOTING ELEVATIONS. VERIFY FOOTING ELEVATIONS IN FIELD.
- SLAB ON GRADE SHALL BE REINFORCED PER THE GENERAL NOTES.
- COORDINATE ALL SLAB ON GRADE RECESSES, CURBS, AND LEDGES WITH THE REQUIREMENTS OF THE ARCH. DWGS.
- COORDINATE ALL SIZES AND LOCATIONS OF HOUSE KEEPING PADS WITH THE ARCH. AND MECH. DWGS.
- VERIFY SLAB DIMENSIONS WITH ARCH. DWGS. REFER TO ARCH. DRAWINGS FOR SLAB EDGE DIMENSIONS NOT INDICATED.
- PX INDICATES PIER CALL OUT. BPX INDICATES BASE PLATE CALL OUT. T.O. PIER ELEVATION SHALL = -0'-0" U.N.O.
- FOUNDATIONS, COLUMNS, AND PIERS ARE CENTERED ON THE GRID LINES. U.N.O.; ALL BEAMS TO BE EQUALLY SPACED BETWEEN COLUMN LINES. U.N.O.
- ALL EXISTING FOUNDATION WALL WEEPS TO BE CLEANED.



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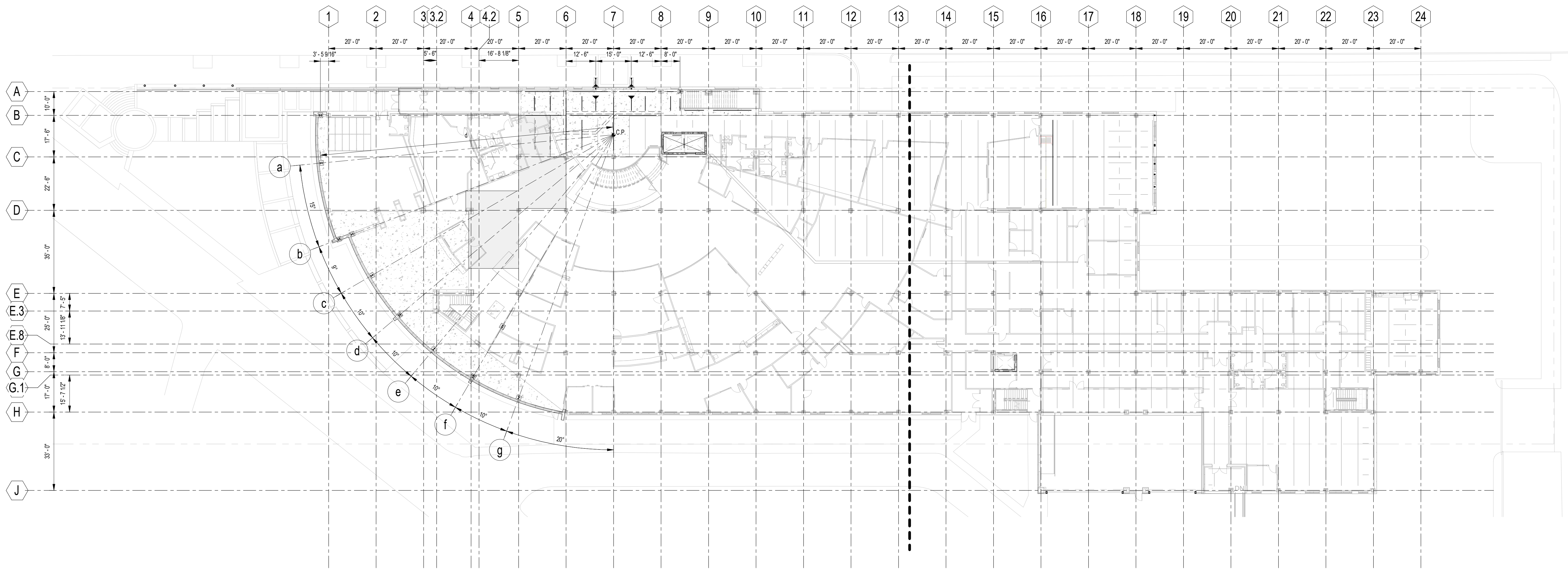


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GROUND FLOOR PART PLANS		DATE
NO.	DESCRIPTION	DATE
1	SCHEMATIC DESIGN	10/23/24
2	DESIGN DEVELOPMENT	02/21/25
3	75% GMP	08/22/25
4	BID & PERMIT	10/03/25

S1.0A



1 FIRST FLOOR OVERALL PLAN
3/64" = 1'-0"

FOUNDATION PLAN NOTES:

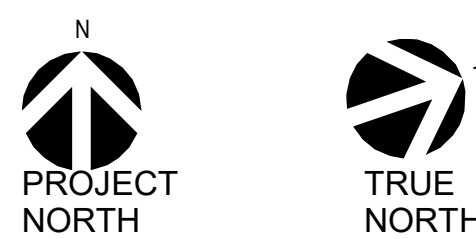
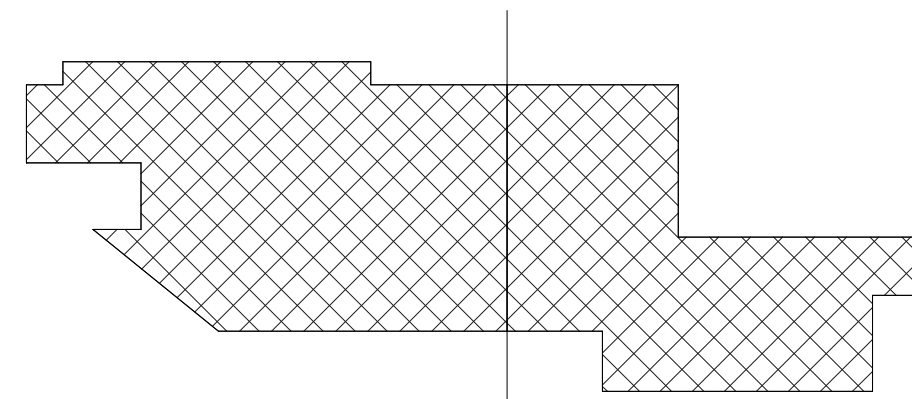
- DATUM ELEVATION 0'-0" = 142'-0"
- SEE S3 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- [X-X'] INDICATES TOP OF FOOTING ELEVATION, TOP OF ALL INTERIOR FOOTING ELEV. = 1'-0" U.N.O. TOP OF ALL EXTERIOR FOOTING ELEV. = 2'-0" U.N.O.
- BOTTOMS OF ALL PERIMETER FOOTINGS SHALL BE 3'-0" MIN. BELOW GRADE.
- NEW FOOTING ELEVATIONS SHALL MATCH THE EXISTING ADJACENT FOOTING ELEVATIONS. VERIFY FOOTING ELEVATIONS IN FIELD.
- SLAB ON GRADE SHALL BE REINFORCED PER THE GENERAL NOTES.
- COORDINATE ALL SLAB ON GRADE RECESSES, CURBS, AND LEDGES WITH THE REQUIREMENTS OF THE ARCH. DWGS.
- COORDINATE ALL SIZES AND LOCATIONS OF HOUSE KEEPING PADS WITH THE ARCH. AND MECH. DWGS.
- VERIFY SLAB DIMENSIONS WITH ARCH. DWGS. REFER TO ARCH. DRAWINGS FOR SLAB EDGE DIMENSIONS NOT INDICATED.
- PI INDICATES PIER CALL OUT. BPI INDICATES BASE PLATE CALL OUT. T.O. PIEN ELEVATION SHALL = 0'-0" U.N.O.
- FOUNDATIONS, COLUMNS, AND PIERS ARE CENTERED ON THE GRID LINES. U.N.O.; ALL BEAMS TO BE EQUALLY SPACED BETWEEN COLUMN LINES. U.N.O.
- ALL EXISTING FOUNDATION WALL WEEPS TO BE CLEANED.

FRAMING PLAN NOTES:

- REFER TO S3 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - RAISED GROUND FLOOR TOP OF SLAB ELEV. = 0'-0"
 - FIRST FL. T.O. STEEL ELEV. = 1'-3" - 2'-13/16"; TOP OF SLAB ELEV. = 1'-3" - 7'-9/16"
 - SECOND FL. T.O. STEEL ELEV. = 1'-2" - 2'-13/16"; TOP OF SLAB ELEV. = 1'-2" - 7'-9/16"
 - ROOF T.O. STEEL ELEV. = 1'-42" - 2'-11/16"; TOP OF DECK ELEV. = 1'-42" - 3'-9/16"
- ELEVATIONS INDICATED (XXX) ARE RELATIVE TO EXIST. FLOOR T.O. STEEL ELEVATION.
- VERIFY SLAB DIMS. WITH ARCH. DWGS.
- SEE ARCH. DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES. U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS. U.N.O.
- REF. ARCH. FOR TOP OF FINISHED FLOOR AND CANOPY ELEVATION.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS TO COORDINATE OPENINGS.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

S-1
DENOTES SPAN OF 4'-3/4" COMPOSITE SLAB.
SLAB SHALL BE 3'-11/4" L.W.G. OVER 1'-0"
COMPOSITE METAL DECK. SEE GENERAL
NOTES FOR ADDITIONAL DETAILS.

COLUMN LOCATIONS	
GRIDLINE	DISTANCE FROM C.P. "ft"
a	123'-0"
b	123'-0"
c	117'-10"
d	117'-10"
e	117'-10"
f	117'-10"
g	117'-10"



FIRST FLOOR OVERALL PLAN	
NO.	DATE
DESIGN DEVELOPMENT	02/21/25
75% GMP	09/22/25
BID & PERMIT	10/03/25

S1.1

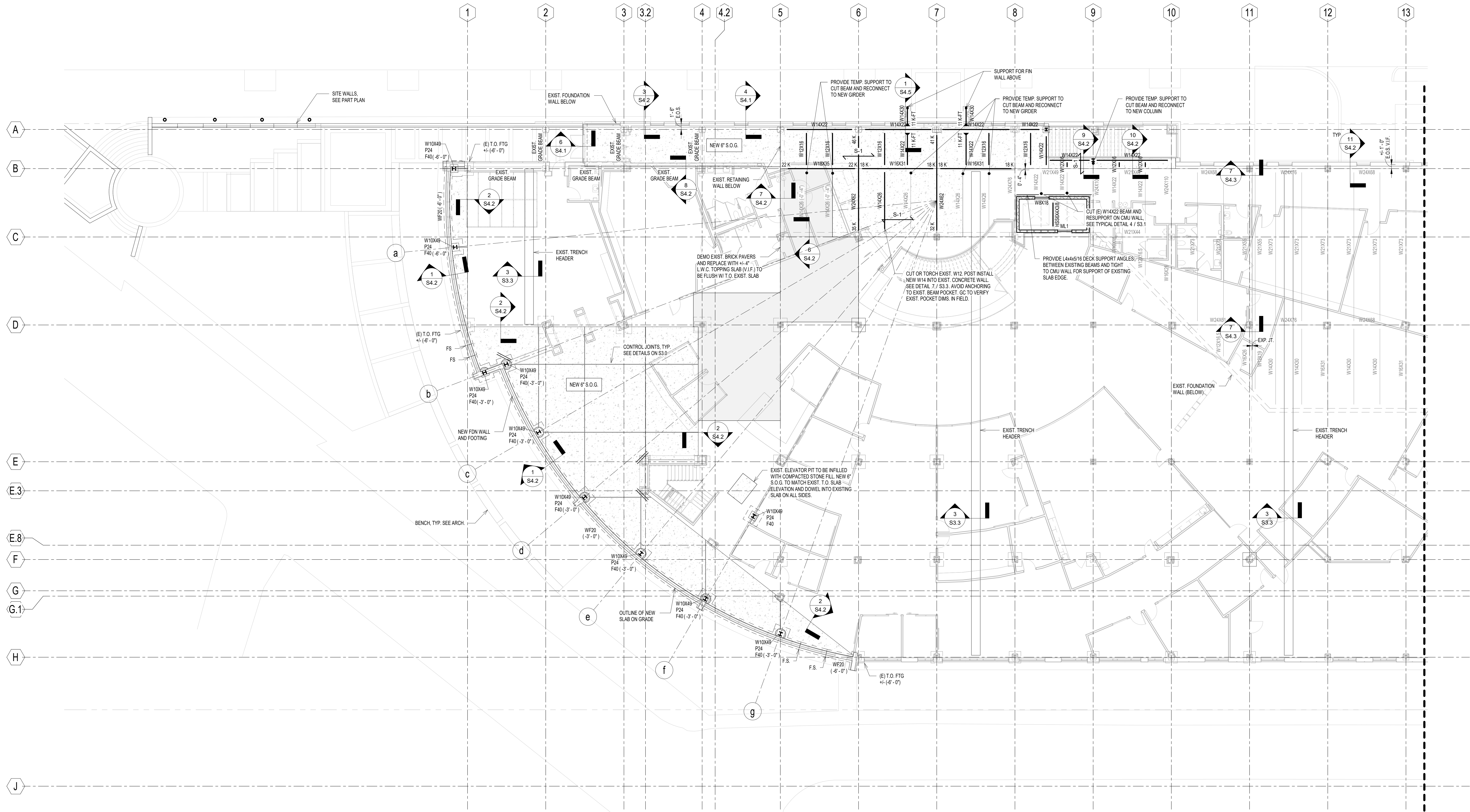
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1 FIRST FLOOR PLAN - AREA A
3/32" = 1'-0"

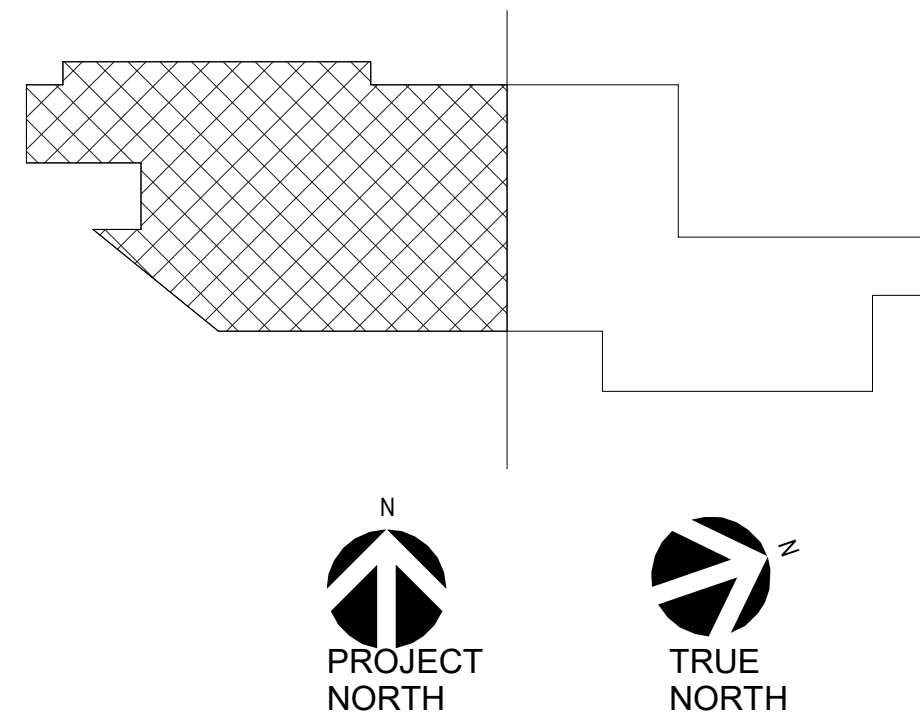
FOUNDATION PLAN NOTES:

- DATUM ELEVATION 0'-0" = 140'-8"
- SEE S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- 1'-X'-XX" INDICATES TOP OF FOOTING ELEVATION. TOP OF ALL INTERIOR FOOTING ELEV. = 1'-0" U.N.O. TOP OF ALL EXTERIOR FOOTING ELEV. = 2'-0" U.N.O.
- BOTTOMS OF ALL PERIMETER FOOTINGS SHALL BE 3'-0" MIN. BELOW GRADE.
- NEW FOOTING ELEVATIONS SHALL MATCH THE EXISTING ADJACENT FOOTING ELEVATIONS. VERIFY FOOTING ELEVATIONS IN FIELD.
- SLAB ON GRADE SHALL BE REINFORCED PER THE GENERAL NOTES.
- COORDINATE ALL SLAB ON GRADE RECESSES, CURBS, AND LEDGES WITH THE REQUIREMENTS OF THE ARCH. DWGS.
- COORDINATE ALL SIZES AND LOCATIONS OF HOUSE KEEPING PADS WITH THE ARCH. AND MECH. DWGS.
- VERIFY SLAB DIMENSIONS WITH ARCH. DWGS. REFER TO ARCH. DRAWINGS FOR SLAB EDGE DIMENSIONS NOT INDICATED.
- PX INDICATES PIER CALL OUT. BPX INDICATES BASE PLATE CALL OUT. T.O. PIER ELEVATION SHALL = 2'-8" U.N.O.
- FOUNDATIONS, COLUMNS, AND PIERS ARE CENTERED ON THE GRID LINES. U.N.O.; ALL BEAMS TO BE EQUALLY SPACED BETWEEN COLUMN LINES. U.N.O.
- ALL EXISTING FOUNDATION WALL WEEPS TO BE CLEANED.

FRAMING PLAN NOTES:

- REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- FLOOR ELEVATIONS (U.N.O. V.I.P.):
 - A. RAISED GROUND FLOOR TOP OF SLAB ELEV. = 10'-0"
 - B. FIRST FL. T.O. STEEL ELEV. = 1'-3" - 2 13/16", TOP OF SLAB ELEV. = 1'-3" - 7 9/16"
 - C. SECOND FL. T.O. STEEL ELEV. = 1'-0" - 2 13/16", TOP OF SLAB ELEV. = 1'-0" - 7 9/16"
 - D. ROOF T.O. STEEL ELEV. = 1'-2" - 2 11/16", TOP OF DECK ELEV. = 1'-2" - 3 9/16"
- ELEVATIONS INDICATED (XX") ARE RELATIVE TO EXIST. FLOOR T.O. STEEL ELEVATION.
- VERIFY SLAB DIMS WITH ARCH. DWGS.
- SEE ARCH. DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES. U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS. U.N.O.
- REF. ARCH. FOR TOP OF FINISHED FLOOR AND CANOPY ELEVATION.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS TO COORDINATE OPENINGS.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

S-1 DENOTES SPAN OF 4 3/4" COMPOSITE SLAB. SLAB SHALL BE 3 1/4" L.W.C. OVER 1 1/2" COMPOSITE METAL DECK. SEE GENERAL NOTES FOR ADDITIONAL DETAILS.



FIRST FLOOR PLAN - AREA A

NO.	DESCRIPTION	DATE
1	SCHEMATIC DESIGN	10/23/24
2	DESIGN DEVELOPMENT	02/21/25
3	75% GMP	08/22/25
4	BID & PERMIT	10/03/25

S1.1A

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1 FIRST FLOOR PLAN - AREA B
3/32" = 1'-0"

FRAMING PLAN NOTES:

- REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - RAISED GROUND FLOOR TOP OF SLAB ELEV. = [0'-0"]
 - FIRST FL. T.O. STEEL ELEV. = [1'-2 13/16"]; TOP OF SLAB ELEV. = [1'-3 7/8"]
 - SECOND FL. T.O. STEEL ELEV. = [2'-2 13/16"]; TOP OF SLAB ELEV. = [2'-3 7/8"]
 - ROOF T.O. STEEL ELEV. = [4'-2 2 1/16"]; TOP OF DECK ELEV. = [4'-2 3/16"]ELEVATIONS INDICATED (XX) ARE RELATIVE TO EXIST. FLOOR T.O. STEEL ELEVATION.
- VERIFY SLAB DIMS. WITH ARCH'L DWGS.
- SEE ARCH'L DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES, U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS, U.N.O.
- REFER ARCH'L FOR TOP OF FINISHED FLOOR AND CANOPY ELEVATION.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS TO COORDINATE OPENINGS.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

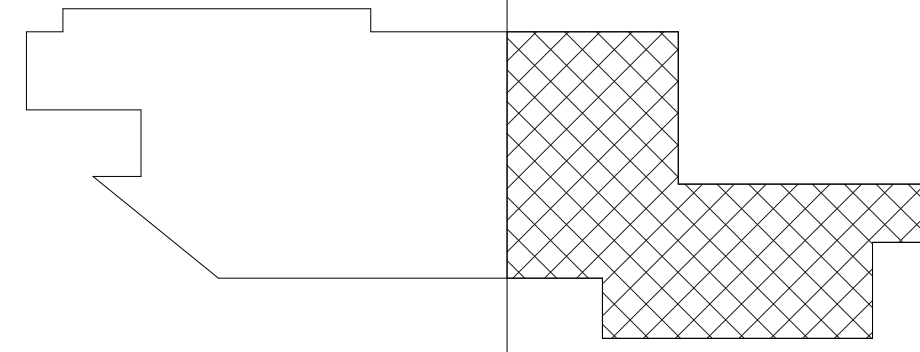
S-1 DENOTES SPAN OF 4 3/4" COMPOSITE SLAB.
SLAB SHALL BE 3 1/4" U.N.C. OVER 1 1/2"
COMPOSITE METAL DECK. SEE GENERAL
NOTES FOR ADDITIONAL DETAILS.

ROOF FRAMING PLAN NOTES:

- REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - RAISED GROUND FLOOR TOP OF SLAB ELEV. = [0'-0"]
 - FIRST FL. T.O. STEEL ELEV. = [1'-2 13/16"]; TOP OF SLAB ELEV. = [1'-3 7/8"]
 - SECOND FL. T.O. STEEL ELEV. = [2'-2 13/16"]; TOP OF SLAB ELEV. = [2'-3 7/8"]
 - ROOF T.O. STEEL ELEV. = [4'-2 2 1/16"]; TOP OF DECK ELEV. = [4'-2 3/16"]ELEVATIONS INDICATED (XX) ARE RELATIVE TO EXIST. FLOOR T.O. STEEL ELEVATION.
- VERIFY DIMS. WITH ARCH'L DWGS.
- SEE ARCH'L DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES, U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS, U.N.O.
- COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND SLEEVES WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- COORDINATE HOIST BEAM AND LIFT SAFETY BEAM REQUIREMENTS WITH ELEVATOR MANUFACTURER.
- EXIST. EDGE OF BENT PLATE = E.O.B.P.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

R-1 DENOTES SPAN OF 1 1/2" METAL ROOF
DECK. SEE GENERAL NOTES FOR
ADDITIONAL DETAILS.

2 VESTIBULE ROOF PART PLAN
3/32" = 1'-0"



FIRST FLOOR PLAN - AREA B

NO.	DESCRIPTION	DATE
1	SCHEMATIC DESIGN	10/23/24
2	DESIGN DEVELOPMENT	02/21/25
3	75% GMP	08/22/25
4	BID & PERMIT	10/03/25

S1.1B

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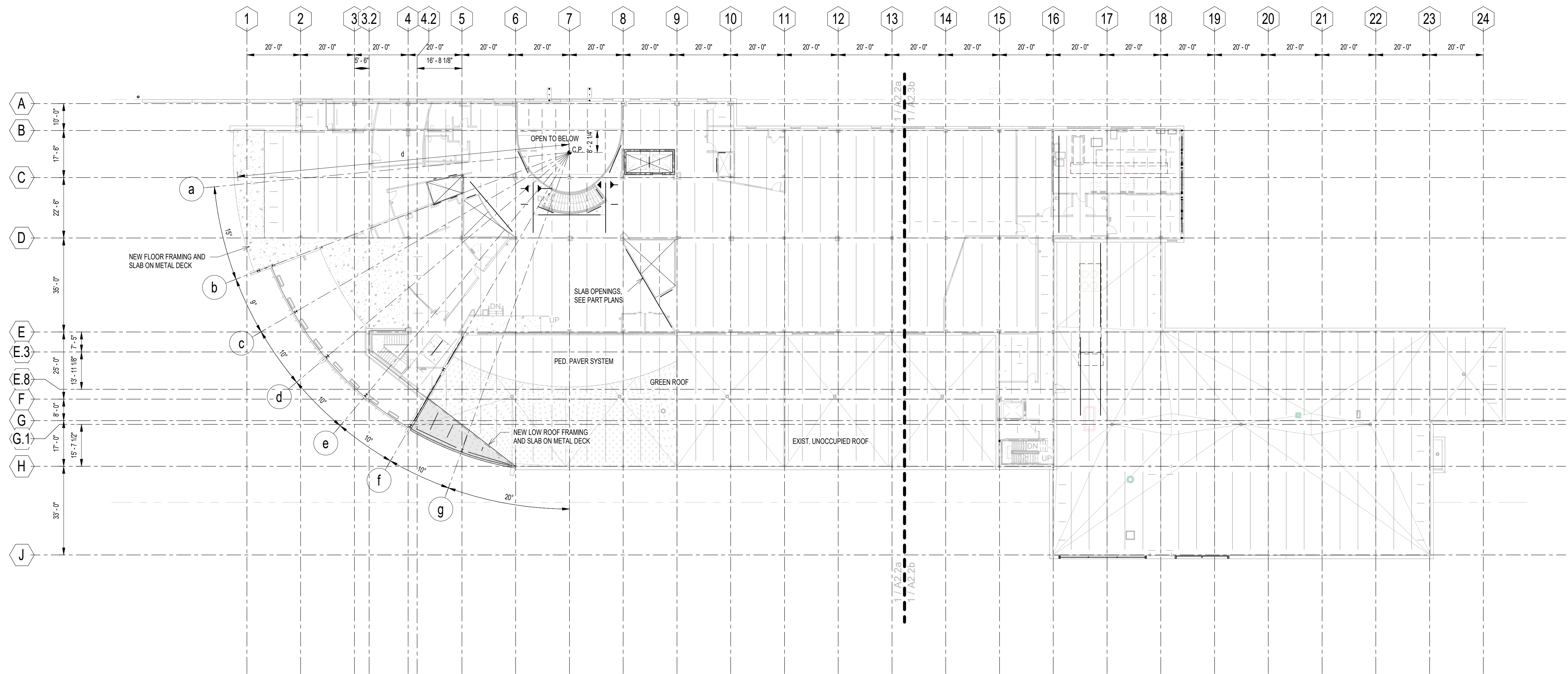
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DRAWN BY:	EMS
CHECKED BY:	RPS
SCALE:	AS NOTED



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1 SECOND FLOOR OVERALL PLAN
3/64" = 1'-0"

FRAMING PLAN NOTES:

- REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - RAISED GROUND FLOOR TOP OF SLAB ELEV. = (+13'-0")
 - FIRST FL. T.O. STEEL ELEV. = (+13'-2 13/16"), TOP OF SLAB ELEV. = (+13'-7 9/16")
 - SECOND FL. T.O. STEEL ELEV. = (+27'-2 13/16"), TOP OF SLAB ELEV. = (+27'-7 9/16")
 - ROOF T.O. STEEL ELEV. = (+42'-2 11/16"), TOP OF DECK ELEV. = (+42'-3 9/16")ELEVATIONS INDICATED (XXX) ARE RELATIVE TO EXIST. FLOOR T.O. STEEL ELEVATION.
- VERIFY SLAB DIMS. WITH ARCH'L DWGS.
- SEE ARCH'L DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES, U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS, U.N.O.
- REF. ARCH. FOR TOP OF FINISHED FLOOR AND CANOPY ELEVATION.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS TO COORDINATE OPENINGS.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

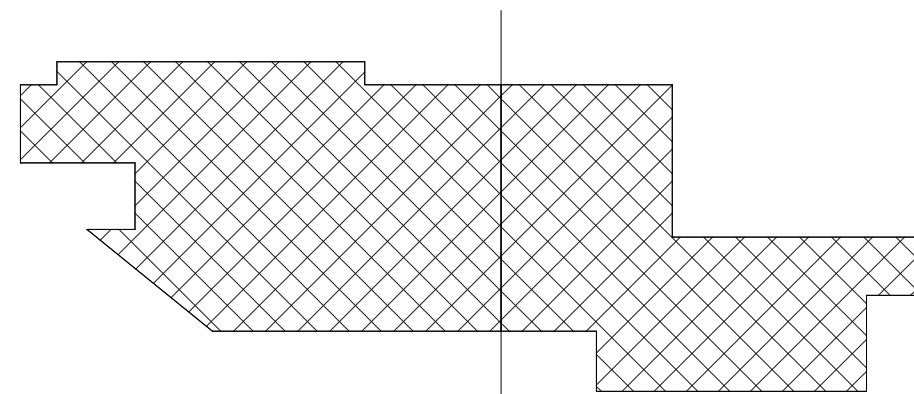
S-1 DENOTES SPAN OF 4 3/4" COMPOSITE SLAB.
SLAB SHALL BE 3 1/4" L.W.C. OVER 1 1/2" ROOF FRAMING PLAN NOTES: DECK, SEE GENERAL NOTES FOR ADDITIONAL DETAILS.

- REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.

- FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - RAISED GROUND FLOOR TOP OF SLAB ELEV. = (+13'-0")
 - FIRST FL. T.O. STEEL ELEV. = (+13'-2 13/16"), TOP OF SLAB ELEV. = (+13'-7 9/16")
 - SECOND FL. T.O. STEEL ELEV. = (+27'-2 13/16"), TOP OF SLAB ELEV. = (+27'-7 9/16")
 - ROOF T.O. STEEL ELEV. = (+42'-2 11/16"), TOP OF DECK ELEV. = (+42'-3 9/16")ELEVATIONS INDICATED (XXX) ARE RELATIVE TO EXIST. FLOOR T.O. STEEL ELEVATION.
- VERIFY DIMS. WITH ARCH'L DWGS.
- SEE ARCH'L DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES, U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS, U.N.O.
- COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND SLEEVES WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- COORDINATE HOIST BEAM AND LIFE SAFETY BEAM REQUIREMENTS WITH ELEVATOR MANUFACTURER.
- EXIST. EDGE OF BENT PLATE = E.O.B.P.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

R-1 DENOTES SPAN OF 1 1/2" METAL ROOF DECK. SEE GENERAL NOTES FOR ADDITIONAL DETAILS.

COLUMN LOCATIONS	
GRIDLINE	DISTANCE FROM C.P. "d"
a	123'-9"
b	123'-9"
c	117'-10"
d	117'-10"
e	117'-10"
f	117'-10"
g	93'-4"



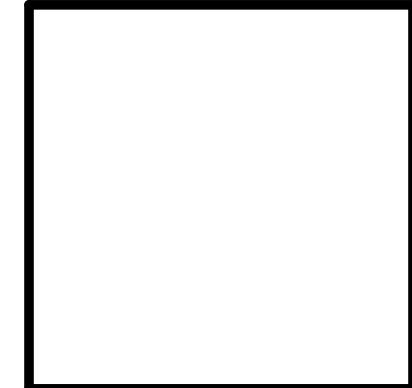
SECOND FLOOR OVERALL PLAN	
NO.	DESCRIPTION
1	DESIGN DEVELOPMENT
2	75% GMP
3	BID & PERMIT

CONSTRUCTION	Issue Date
ISSUE DATE:	02/21/25
DRAWN BY:	EMS
CHECKED BY:	RPS
SCALE:	AS NOTED

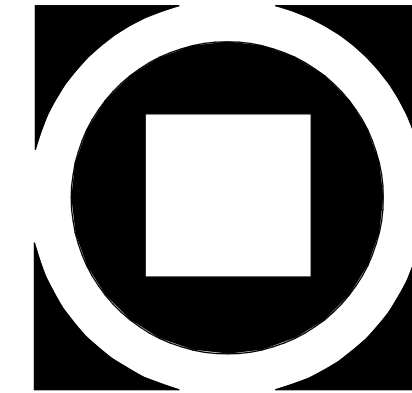
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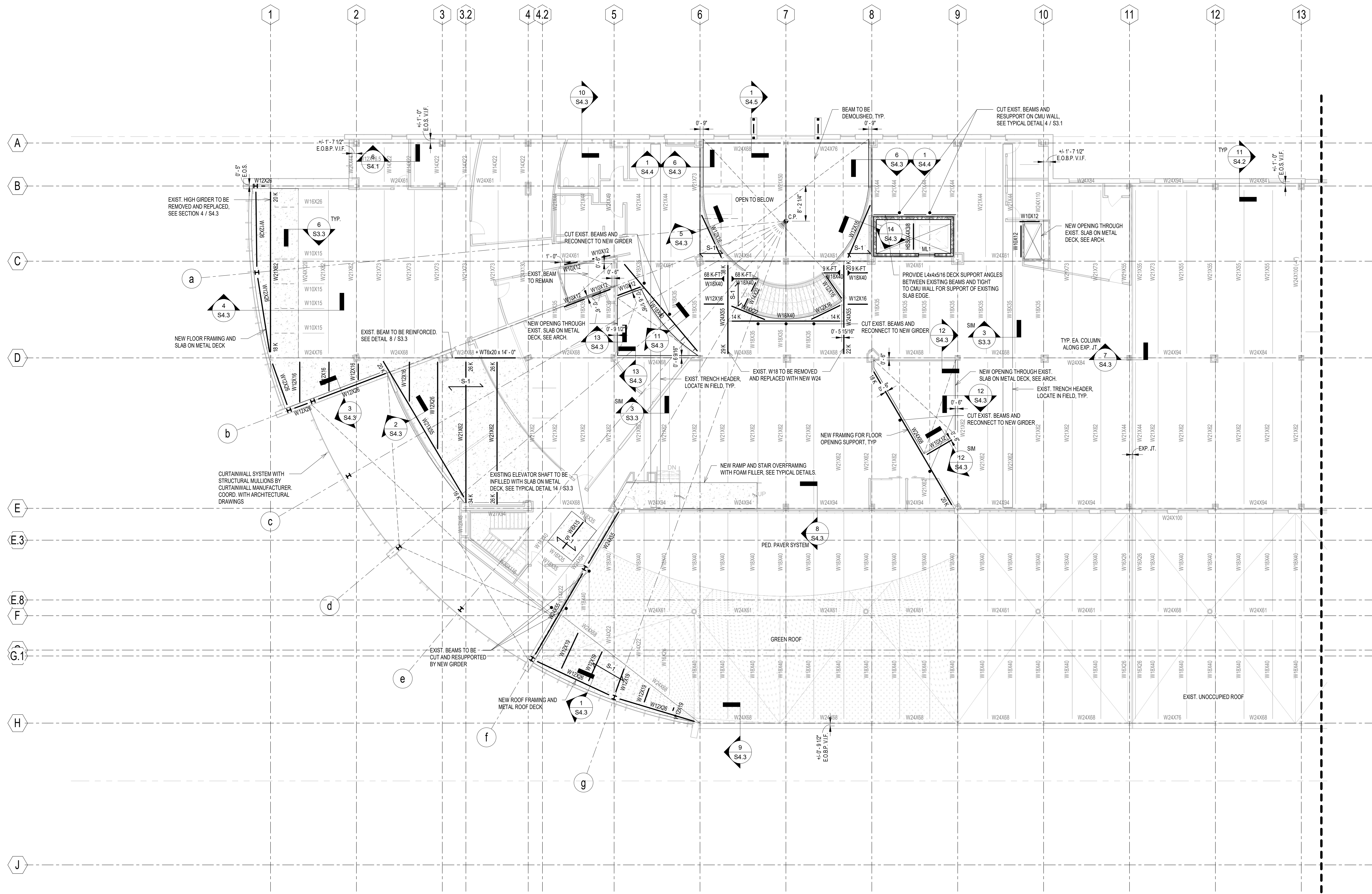


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1 SECOND FLOOR PLAN - AREA A
3/32" = 1'-0"

FRAMING PLAN NOTES:

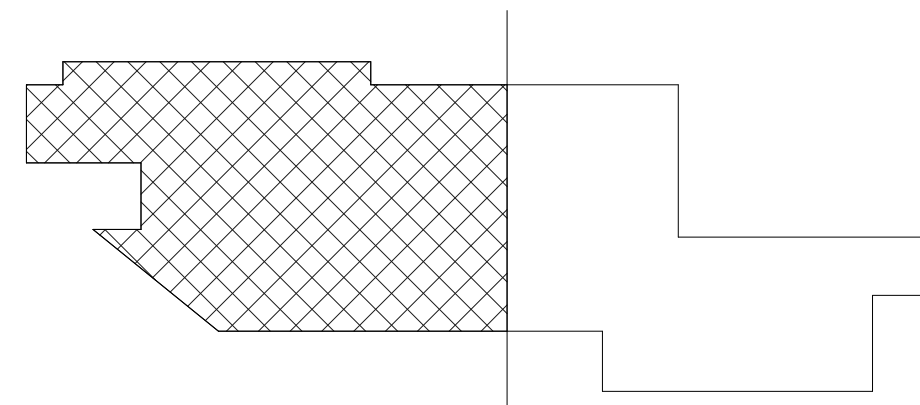
1. REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
2. FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - A. RAISED GROUND FLOOR TOP OF SLAB ELEV. = [0'-0"]
 - B. FIRST FL. T.O. STEEL ELEV. = [+1'-2 13/16"] TOP OF SLAB ELEV. = [+1'-7 9/16"]
 - C. SECOND FL. T.O. STEEL ELEV. = [+2'-2 13/16"] TOP OF SLAB ELEV. = [+2'-7 9/16"]
 - D. ROOF T.O. STEEL ELEV. = [+4'-2 11/16"] TOP OF DECK ELEV. = [+4'-2'-3 9/16"]
3. ELEVATIONS INDICATED (XXX) ARE RELATIVE TO EXIST. FLOOR T.O. STEEL ELEVATION. VERIFY SLAB DIMS. WITH ARCH. DWGS.
4. SEE ARCH. DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
5. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
6. ALL COLUMNS SHALL BE CENTERED ON GRID LINES, U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS, U.N.O.
7. REF. ARCH. FOR TOP OF FINISHED FLOOR AND CANOPY ELEVATION.
8. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS TO COORDINATE OPENINGS.
9. ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

← S-1 →
DENOTES SPAN OF 4 3/4" COMPOSITE SLAB.
SLAB SHALL BE 3 1/4" I.W.C. OVER 1 1/2"
COMPOSITE METAL DECK. SEE GENERAL
NOTES FOR ADDITIONAL DETAILS.

ROOF FRAMING PLAN NOTES:

1. REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
2. FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - A. RAISED GROUND FLOOR TOP OF SLAB ELEV. = [0'-0"]
 - B. FIRST FL. T.O. STEEL ELEV. = [+1'-2 13/16"] TOP OF SLAB ELEV. = [+1'-7 9/16"]
 - C. SECOND FL. T.O. STEEL ELEV. = [+2'-2 13/16"] TOP OF SLAB ELEV. = [+2'-7 9/16"]
 - D. ROOF T.O. STEEL ELEV. = [+4'-2 11/16"] TOP OF DECK ELEV. = [+4'-2'-3 9/16"]
3. ELEVATIONS INDICATED (XXX) ARE RELATIVE TO EXIST. FLOOR T.O. STEEL ELEVATION. VERIFY DIMS. WITH ARCH. DWGS.
4. SEE ARCH. DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
5. ALL COLUMNS SHALL BE CENTERED ON GRID LINES, U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS, U.N.O.
6. COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND SLEEVES WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
7. COORDINATE HOIST BEAM AND LIFE SAFETY BEAM REQUIREMENTS WITH ELEVATOR MANUFACTURER.
8. EXIST. EDGE OF BENT PLATE = E.O.B.P.
9. ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

← R-1 →
DENOTES SPAN OF 1 1/2" METAL ROOF
DECK. SEE GENERAL NOTES FOR
ADDITIONAL DETAILS.



SECOND FLOOR PLAN - AREA A			
NO.	DESCRIPTION	DATE	
1	SCHEMATIC DESIGN	10/23/24	
2	DESIGN DEVELOPMENT	02/21/25	
3	75% GMP	08/22/25	
4	BID & PERMIT	10/03/25	

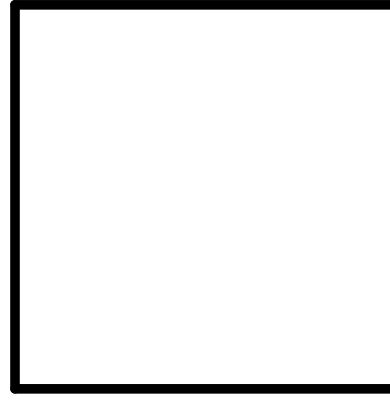
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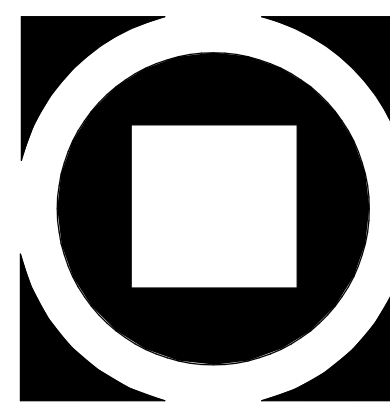
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1 SECOND FLOOR PLAN - AREA B
3/32" = 1'-0"

FRAMING PLAN NOTES:

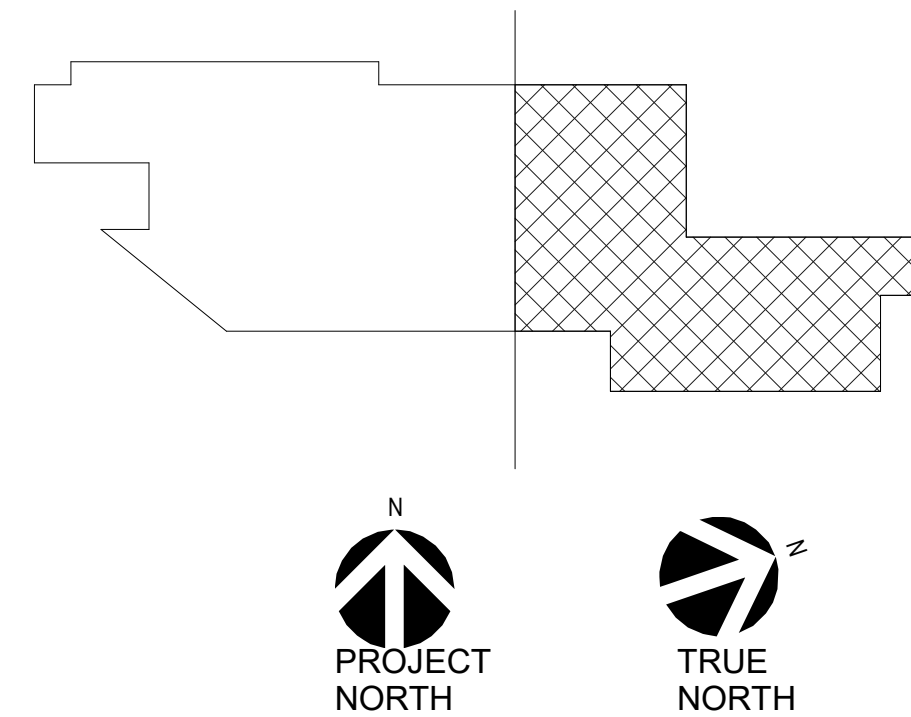
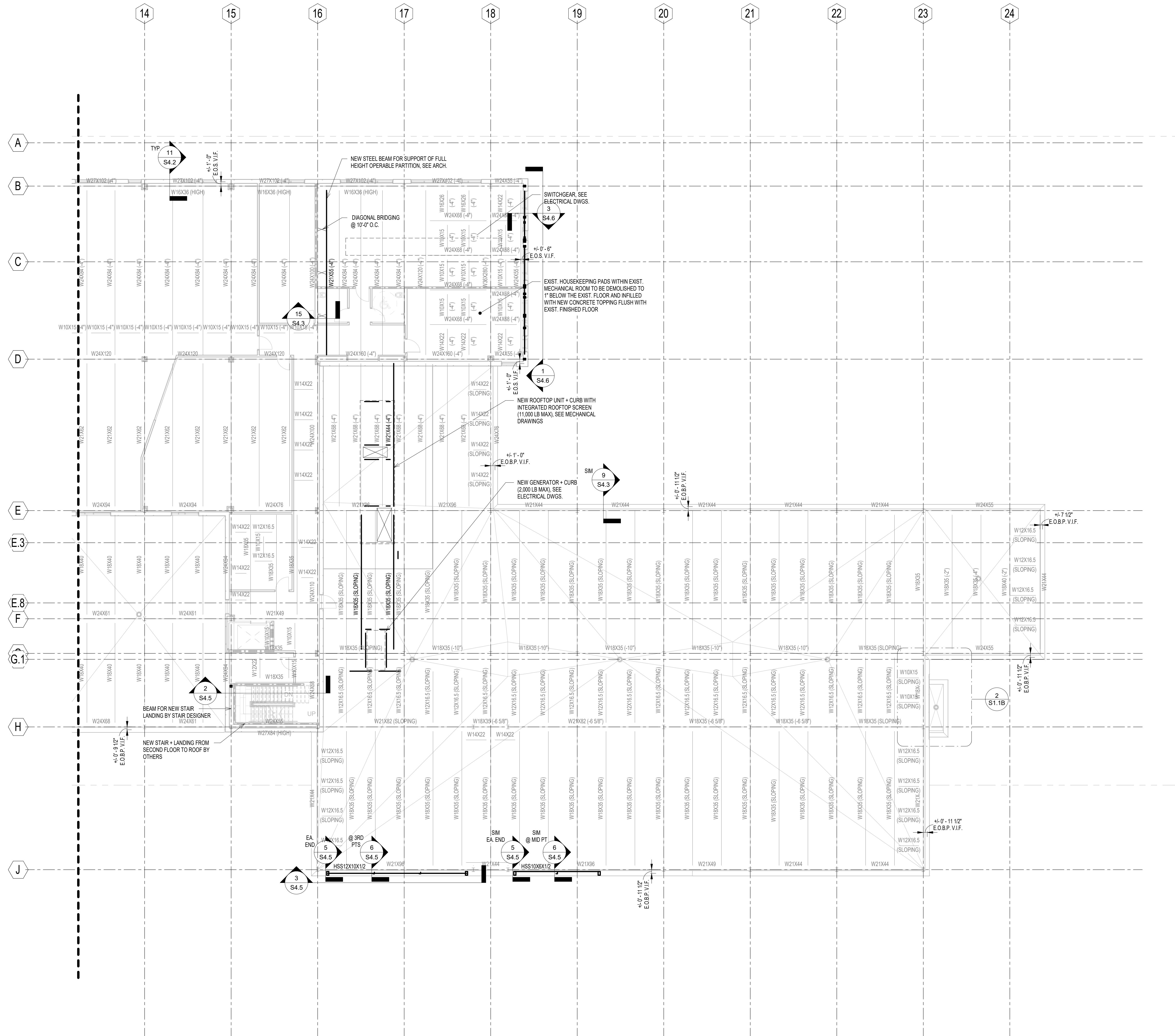
- REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - RAISED GROUND FLOOR TOP OF SLAB ELEV. = [0'-0"]
 - FIRST FL. T.O. STEEL ELEV. = [1'-3" - 2 13/16"] TOP OF SLAB ELEV. = [1'-3" - 7 9/16"]
 - SECOND FL. T.O. STEEL ELEV. = [4'-7" - 2 13/16"] TOP OF SLAB ELEV. = [4'-7" - 7 9/16"]
 - ROOF T.O. STEEL ELEV. = [4'-2" - 2 11/16"] TOP OF DECK ELEV. = [4'-2" - 3 9/16"]
- VERIFY SLAB DIMS. WITH ARCH'L DWGS.
- SEE ARCH'L DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES, U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS, U.N.O.
- REF. ARCH. FOR TOP OF FINISHED FLOOR AND CANOPY ELEVATION.
- REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS TO COORDINATE OPENINGS.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

S-1 DENOTES SPAN OF 4 3/4" COMPOSITE SLAB.
SLAB SHALL BE 3 1/4" L.W.C. OVER 1 1/2" COMPOSITE METAL DECK. SEE GENERAL NOTES FOR ADDITIONAL DETAILS.

ROOF FRAMING PLAN NOTES:

- REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - RAISED GROUND FLOOR TOP OF SLAB ELEV. = [0'-0"]
 - FIRST FL. T.O. STEEL ELEV. = [1'-3" - 2 13/16"] TOP OF SLAB ELEV. = [1'-3" - 7 9/16"]
 - SECOND FL. T.O. STEEL ELEV. = [4'-7" - 2 13/16"] TOP OF SLAB ELEV. = [4'-7" - 7 9/16"]
 - ROOF T.O. STEEL ELEV. = [4'-2" - 2 11/16"] TOP OF DECK ELEV. = [4'-2" - 3 9/16"]
- VERIFY DIMS. WITH ARCH'L DWGS.
- SEE ARCH'L DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES, U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS, U.N.O.
- COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND SLEEVES WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- COORDINATE HOST BEAM AND LIFE SAFETY BEAM REQUIREMENTS WITH ELEVATOR MANUFACTURER.
- EXIST. EDGE OF BENT PLATE = E.O.B.P.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

R-1 DENOTES SPAN OF 1 1/2" METAL ROOF DECK. SEE GENERAL NOTES FOR ADDITIONAL DETAILS.



SECOND FLOOR PLAN - AREA B			
NO.	DESCRIPTION	DATE	
1	SCHEMATIC DESIGN	10/23/24	
2	DESIGN DEVELOPMENT	02/21/25	
3	75% GMP	08/22/25	
4	BID & PERMIT	10/03/25	

S1.2B

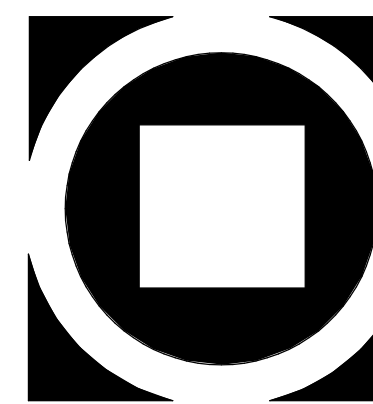
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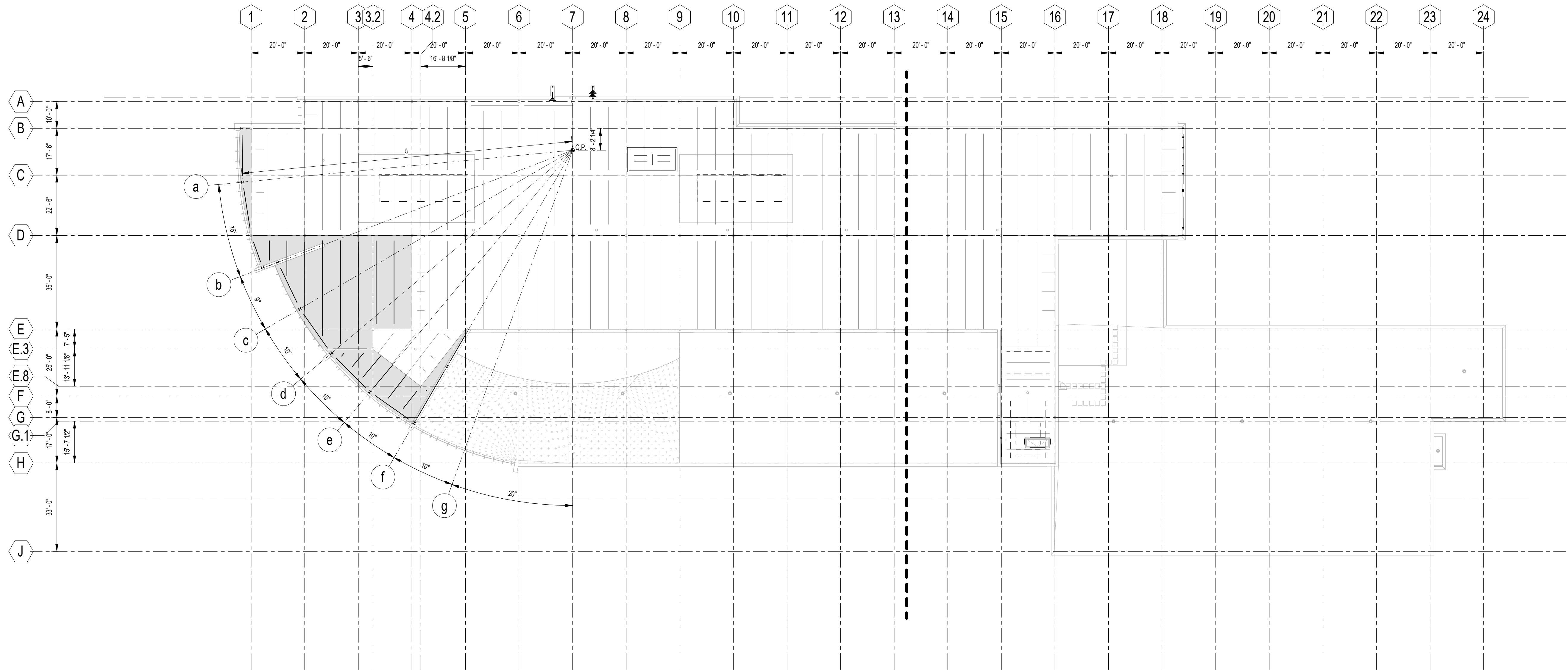
CONSTRUCTION	Issue Date
ISSUE DATE:	
DRAWN BY: EMS	
CHECKED BY: RPS	
SCALE: AS NOTED	



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482 Norristown Rd., Suite 200
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Phone 610.834.7865
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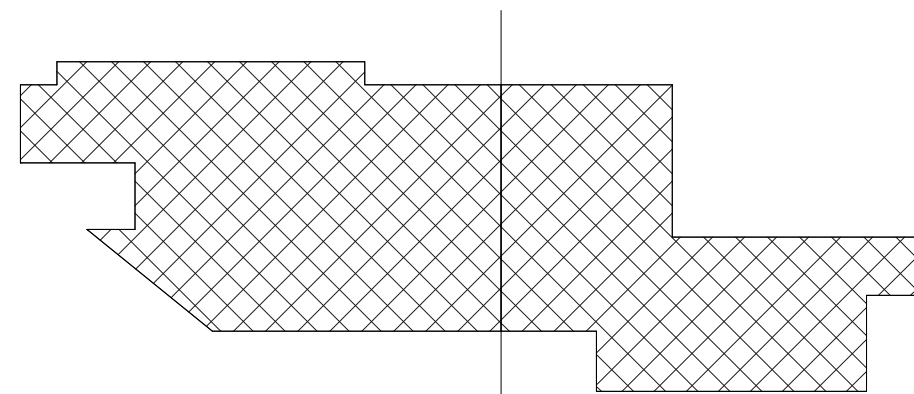
1 ROOF OVERALL PLAN
3/64" = 1'-0"

ROOF FRAMING PLAN NOTES:

- REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - RAISED GROUND FLOOR TOP OF SLAB ELEV. = $(0' - 0")$
 - FIRST FL. T.O. STEEL ELEV. = $(+1' - 2' 13' 16")$, TOP OF SLAB ELEV. = $(+1' - 3' - 7' 9' 16")$
 - SECOND FL. T.O. STEEL ELEV. = $(+2' - 2' 13' 16")$, TOP OF SLAB ELEV. = $(+2' - 7' 9' 16")$
 - ROOF T.O. STEEL ELEV. = $(+42' - 2' 1' 16")$, TOP OF DECK ELEV. = $(+42' - 3' 9' 16")$ELEVATIONS INDICATED (XX) ARE RELATIVE TO EXIST. FLOOR T.O. STEEL ELEVATION.
VERIFY DIMS. WITH ARCH'L DWGS.
- SEE ARCH'L DWGS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES, U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS, U.N.O.
- COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND SLEEVES WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- COORDINATE HOIST BEAM AND LIFE SAFETY BEAM REQUIREMENTS WITH ELEVATOR MANUFACTURER.
- EXIST. EDGE OF BENT PLATE = E.O.B.P.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

R-1 DENOTES SPAN OF 1 1/2" METAL ROOF DECK. SEE GENERAL NOTES FOR ADDITIONAL DETAILS.

COLUMN LOCATIONS	
GRIDLINE	DISTANCE FROM C.P. "ft"
a	123' - 9"
b	123' - 9"
c	117' - 10"
d	117' - 10"
e	117' - 10"
f	117' - 10"
g	117' - 10"



ROOF OVERALL PLAN

NO.	DESCRIPTION	DATE
1	DESIGN DEVELOPMENT	02/21/25
2	75% GMP	09/22/25
3	BID & PERMIT	10/03/25

CONSTRUCTION ISSUE DATE:	ISSUED DATE
DRAWN BY: EMS	
CHECKED BY: RPS	
SCALE: AS NOTED	

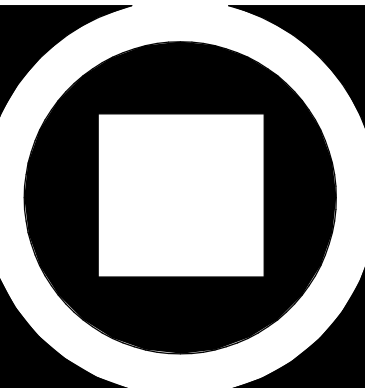
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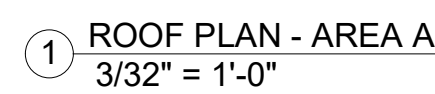


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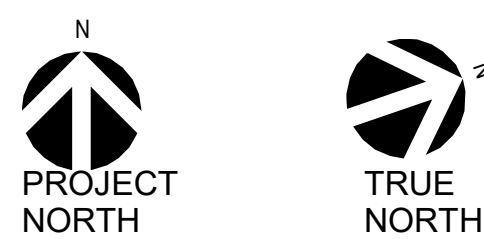


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1. REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
2. FLOOR ELEVATIONS (UNO J.F.I.):
 - A. BASED GROUND FLOOR TOP OF SLAB ELEVATION = $+0'-0"$
 - B. FIRST FLOOR TO STEEL ELEVATION = $+1'-3 \frac{1}{2}"$ TOP OF SLAB ELEVATION = $+1'-3 \frac{1}{2}" - 7 \frac{1}{8}"$
 - C. RASSED FLOOR TO STEEL ELEVATION = $+2'-7 \frac{1}{8}"$ TOP OF SLAB ELEVATION = $+2'-7 \frac{1}{8}" - 7 \frac{1}{8}"$
 - D. ROOF TO STEEL ELEVATION = $+14'-2 \frac{1}{8}"$ TOP OF DECK ELEVATION = $+14'-2 \frac{1}{8}" - 3 \frac{1}{8}"$
3. ELEVATIONS INDICATED "XY" ARE RELATIVE TO STATION FLOOR 1.0. STEEL ELEVATION.
4. VERIFY DIMS WITH ARCHIT. DWGS.
5. SEE ARCH. DWGS. FOR STEEL, THAT REQUIRES PIERPROOFING.
6. ALL COLUMNS SHALL BE CENTERED ON GRID LINES. UNO. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS.
7. COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND SLEEVES WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
8. CONSIDER ALL BEAMS TO BE FULLY SPANNED BEAM REQUIREMENTS WITH ELEVATOR MANUFACTURER.
9. EXIST. EDGE OF BENT PLATE = E.O.B.P.
10. ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

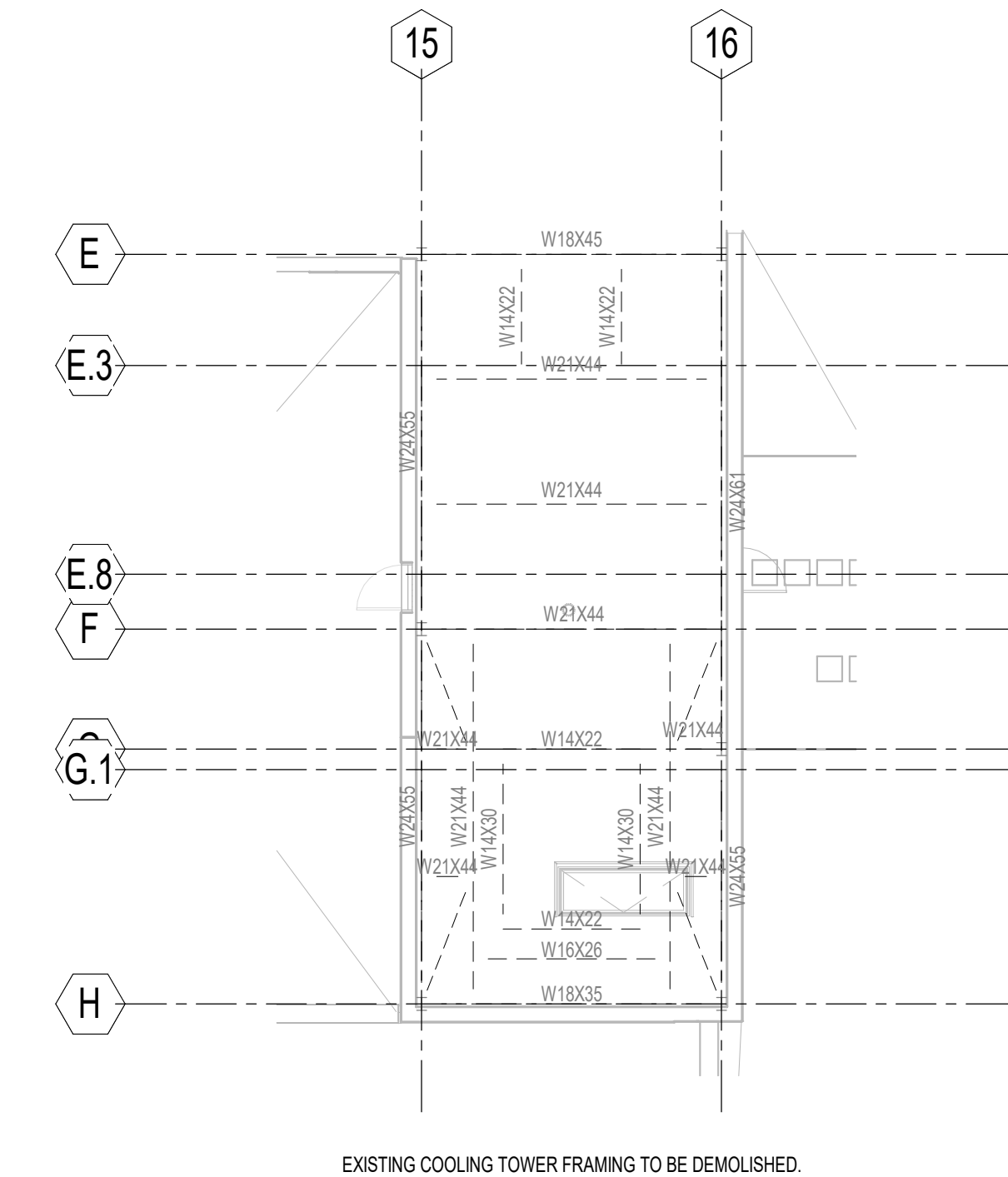


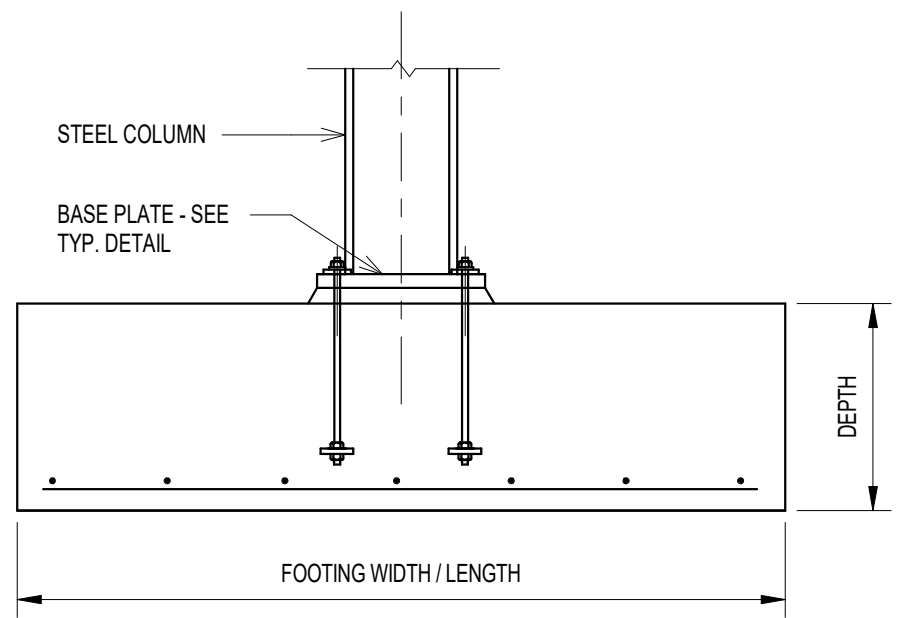
1 ROOF PLAN - AREA B
3/32" = 1'-0"

ROOF FRAMING PLAN NOTES:

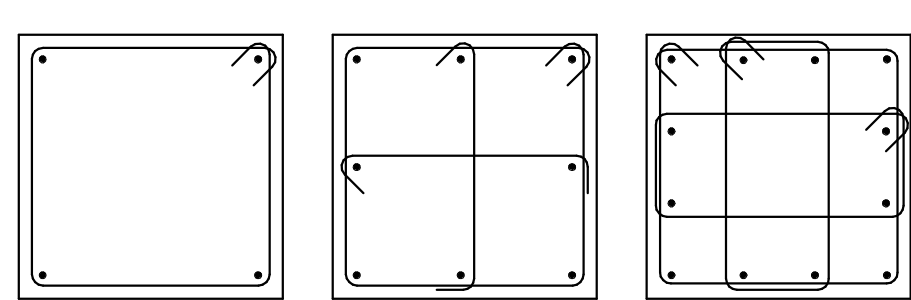
- REFER TO S0 SERIES FOR GENERAL NOTES AND S3 SERIES FOR TYPICAL DETAILS.
- FLOOR ELEVATIONS (U.N.O. V.I.F.):
 - RAISED GROUND FLOOR TOP OF SLAB ELEV. = (+7'-0")
 - FIRST FL. T.O. STEEL ELEV. = (+2'-2 13/16") TOP OF SLAB ELEV. = (+1'-3" - 7 9/16")
 - SECOND FL. T.O. STEEL ELEV. = (+2'-2 13/16") TOP OF SLAB ELEV. = (+2'-7 9/16")
 - ROOF T.O. STEEL ELEV. = (+42'-2 1/16") TOP OF DECK ELEV. = (+42'-9 9/16")
- ELEVATIONS INDICATED (ANY) ARE RELATIVE TO (+361) FLOOR T.O. STEEL ELEVATION.
- VERIFY DIMS. WITH ARCH. DWS.
- SEE ARCH. DWS. FOR STEEL THAT REQUIRES FIREPROOFING.
- ALL COLUMNS SHALL BE CENTERED ON GRID LINES. U.N.O. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMNS. U.N.O.
- COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND SLEEVES WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
- COORDINATE HOIST BEAM AND LIFE SAFETY BEAM REQUIREMENTS WITH ELEVATOR MANUFACTURER.
- EXIST. EDGE OF BENT PLATE - E.O.B.P.
- ALL REACTIONS SHOWN ON PLAN ARE SERVICE LOAD REACTIONS.

R-1 DENOTES SPAN OF 1 1/2" METAL ROOF DECK. SEE GENERAL NOTES FOR ADDITIONAL DETAILS.

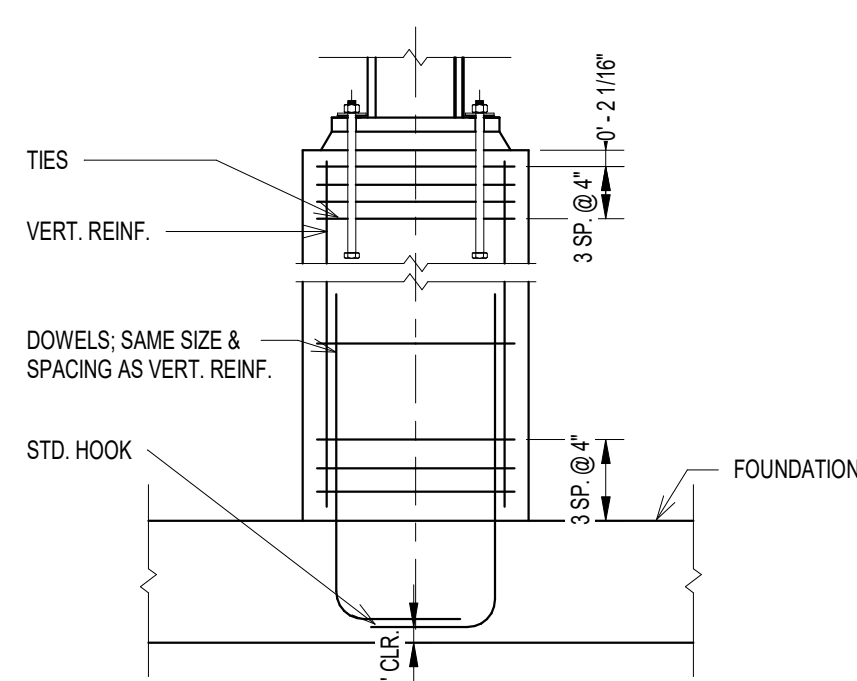




1 SPREAD FOOTING SUPPORTING STEEL COLUMN
1/2" = 1'-0"

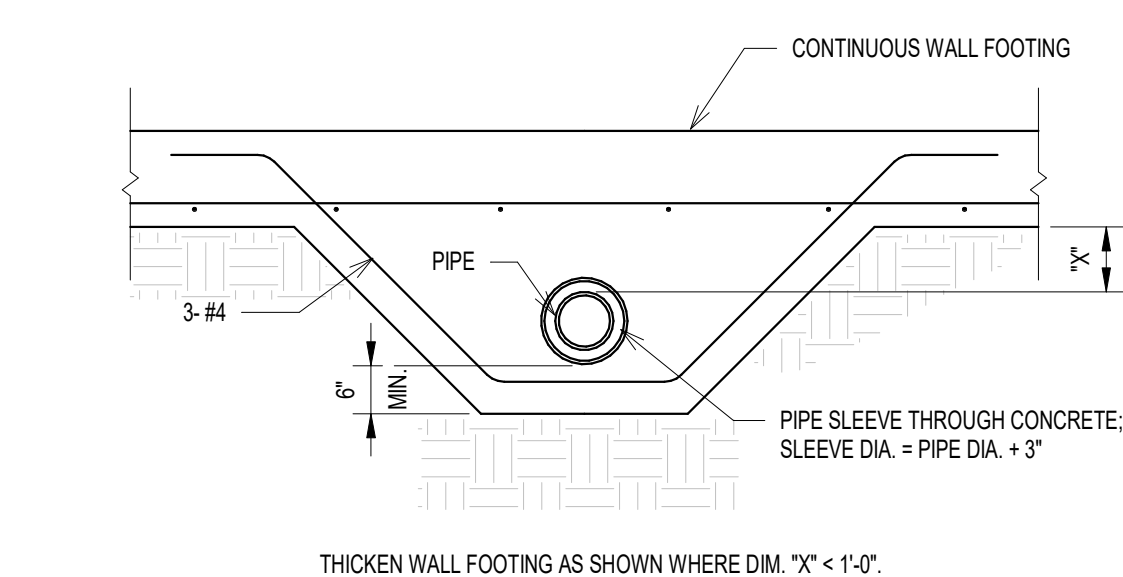
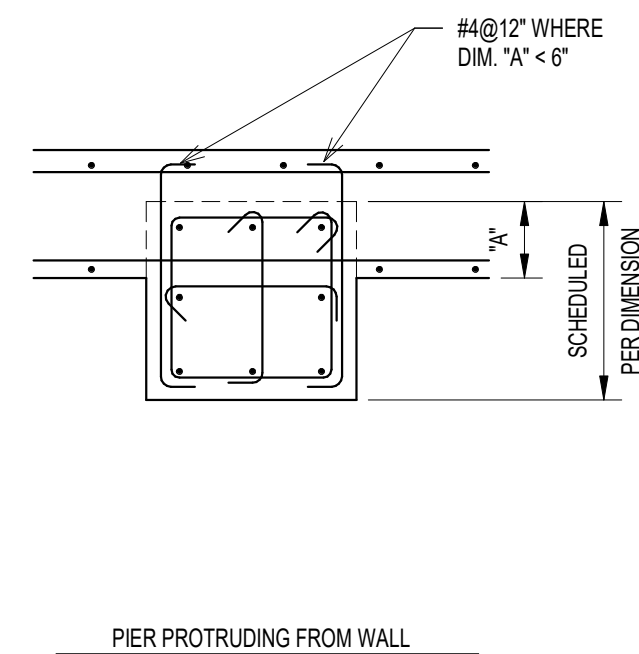


2 CONCRETE PIER VERTICAL REINF. STEEL AND TIE BAR ARRANGEMENTS
1/2" = 1'-0"

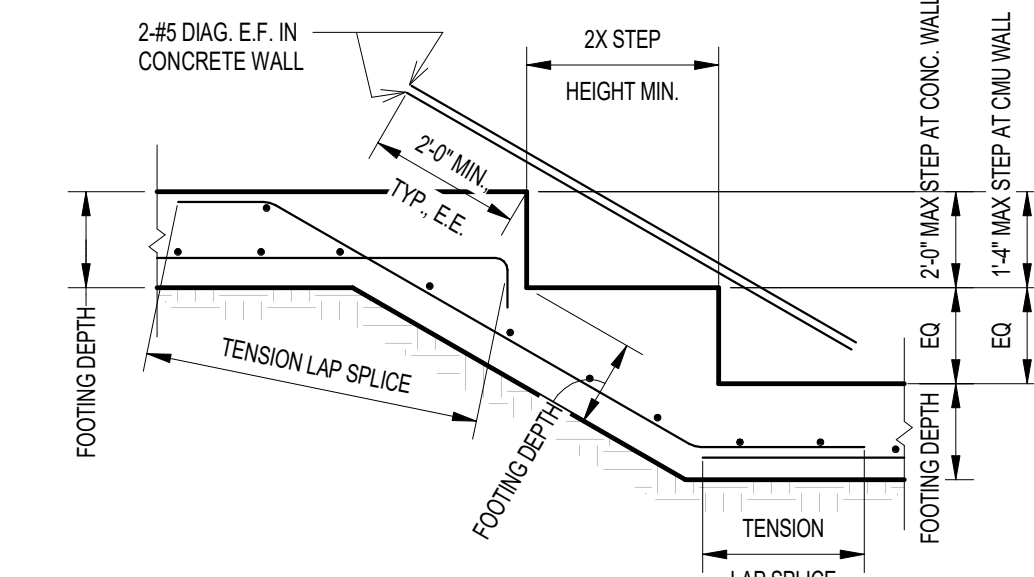


- NOTES:
- SEE SCHEDULE FOR PLASTER DIMENSIONS AND REINFORCING.
 - CONSTRUCT ADJOINING CONCRETE WALLS MONOLITHIC WITH PLASTER.
 - INSTALL HORIZONTAL WALL REINFORCING STEEL CONTINUOUS THROUGH INTERSECTING PLASTER OR SPlice HORIZONTAL REINFORCING WITH TENSION LAP SPICES IF REQUIRED TO PERMIT PROPER PLACEMENT OF PLASTER REINFORCING STEEL.

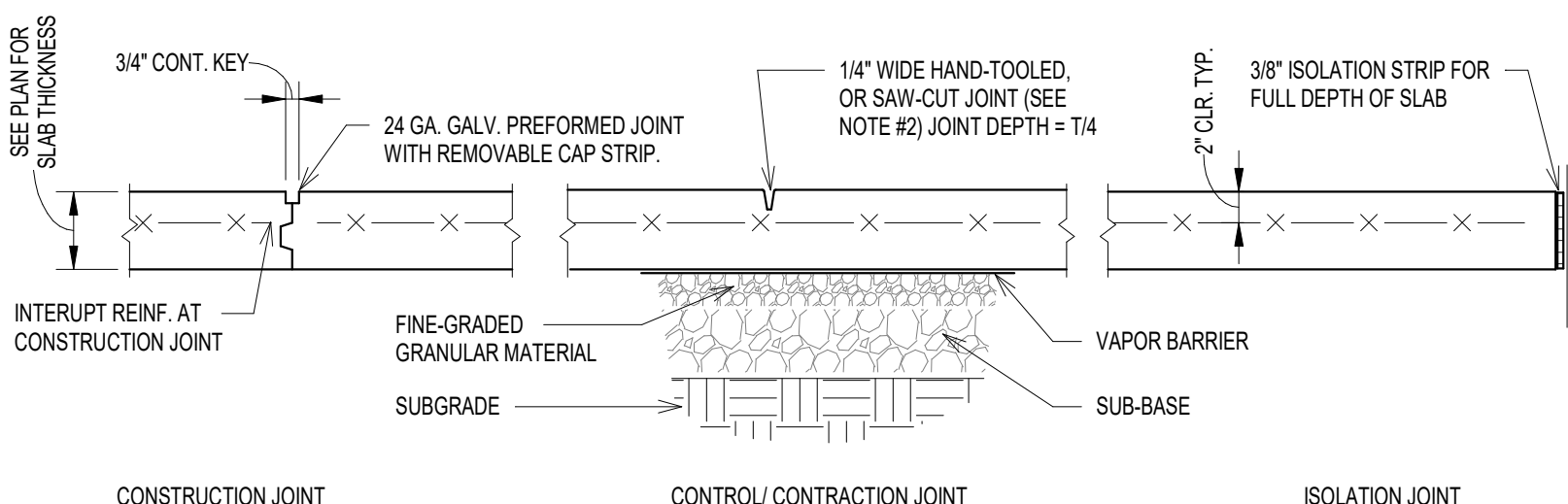
3 CONCRETE PIER & WALL INTERFACE
1/2" = 1'-0"



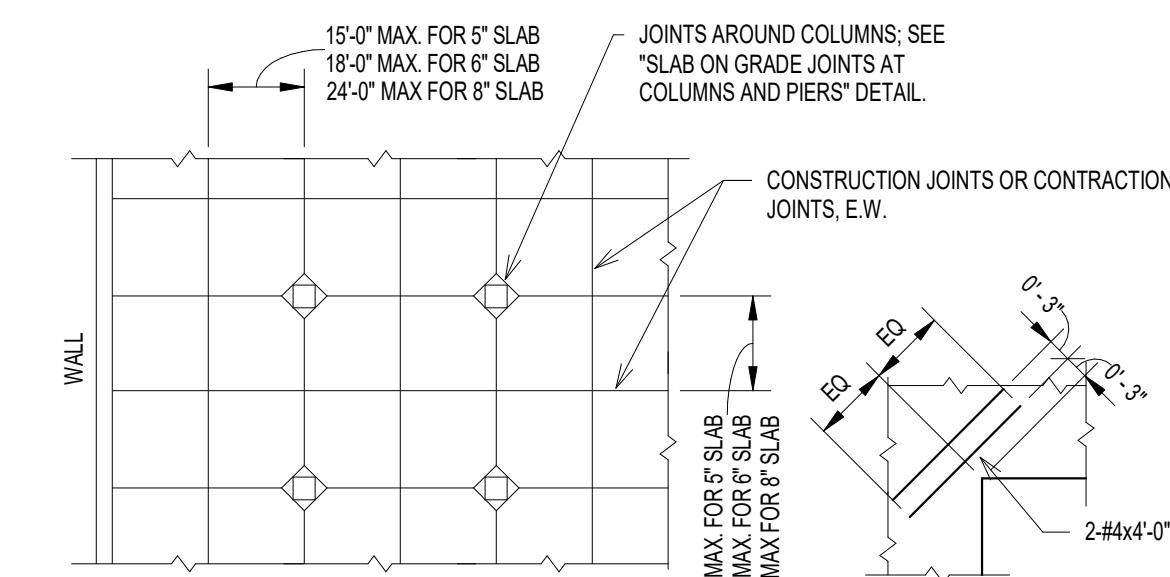
4 PIPE BELOW WALL FOOTING
1/2" = 1'-0"



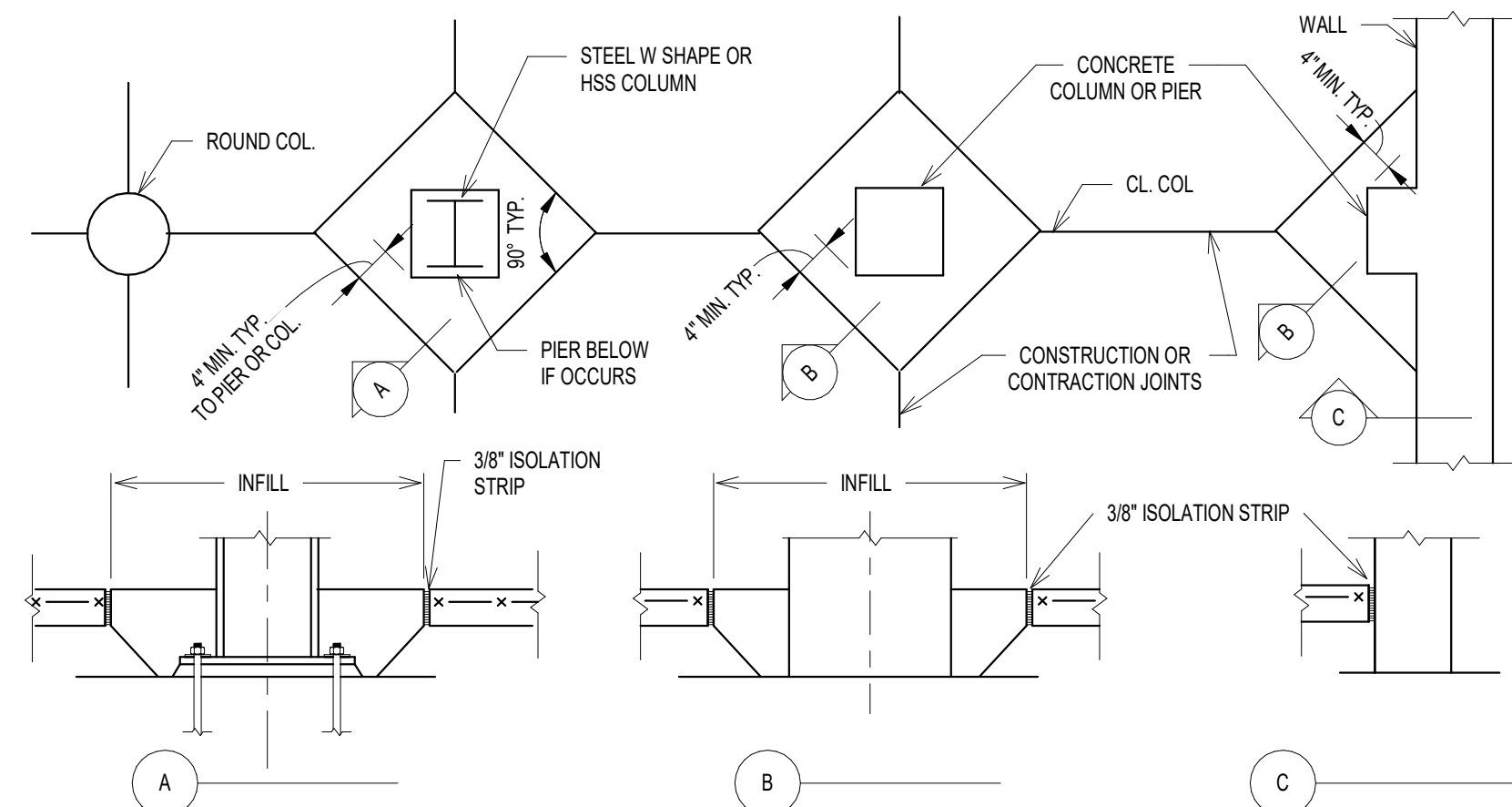
5 STEP IN WALL FOOTING
1/2" = 1'-0"



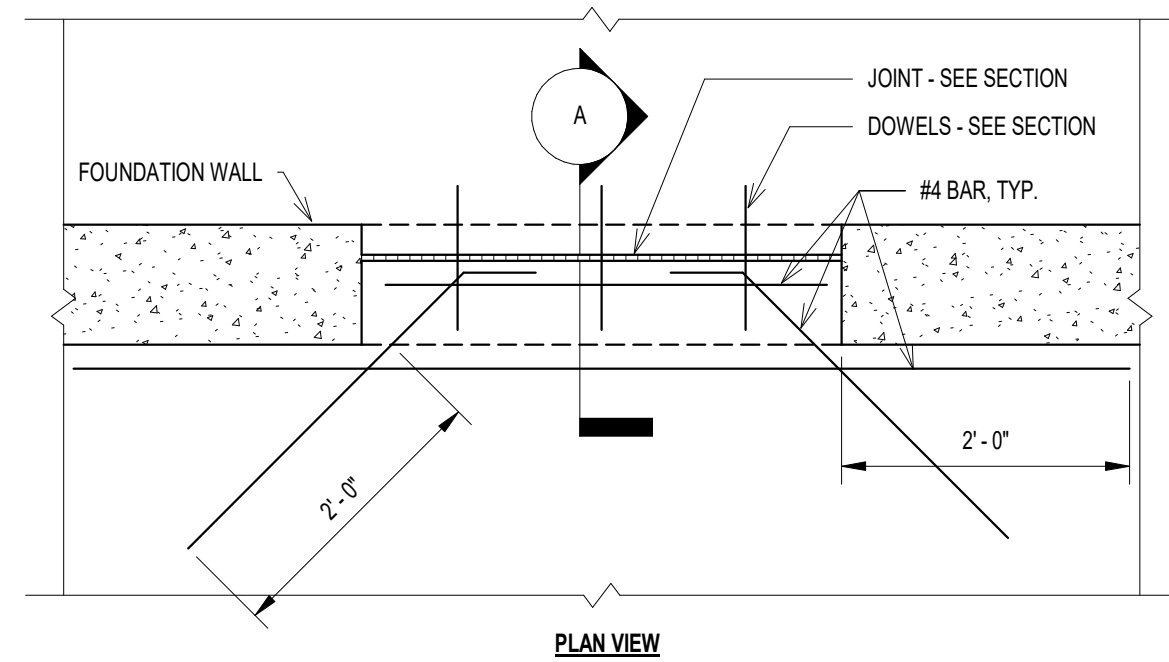
6 SLAB ON GRADE CONSTRUCTION
1" = 1'-0"



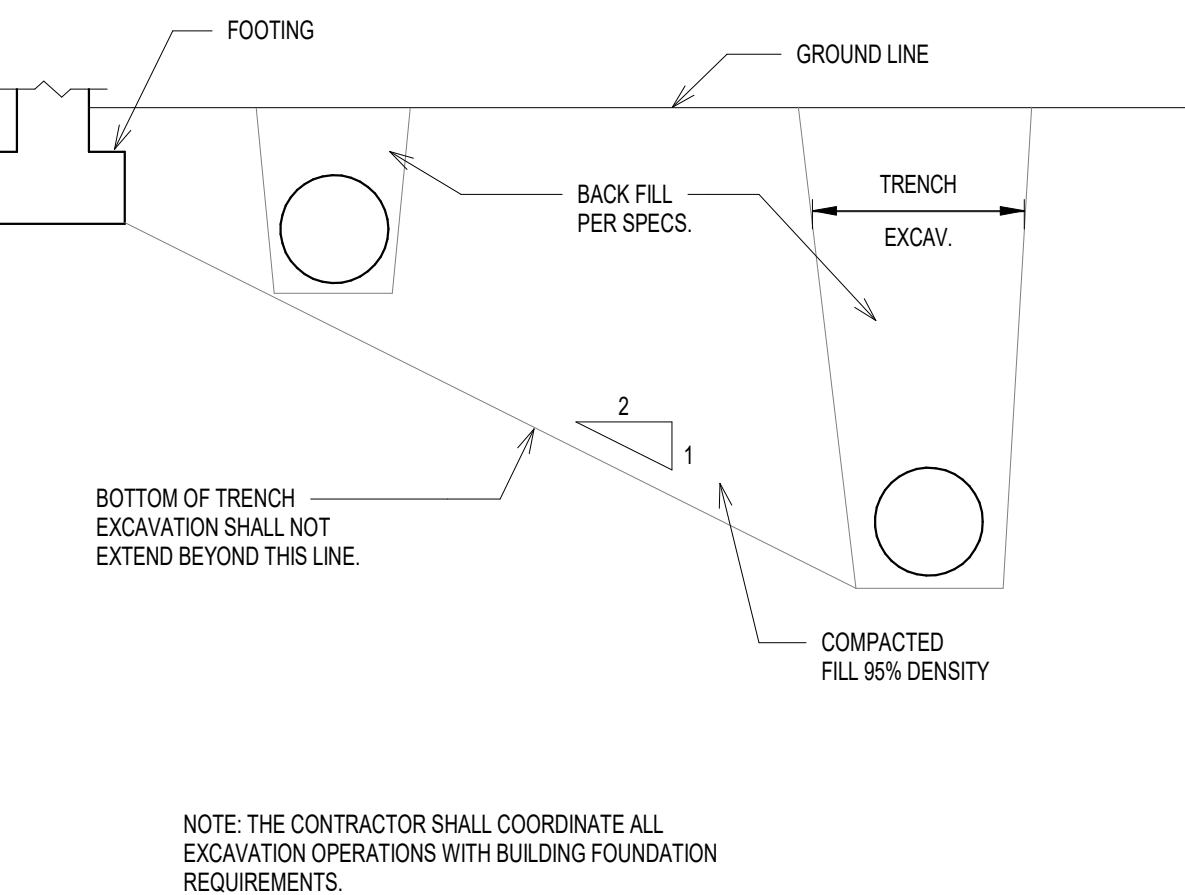
7 SLAB ON GRADE JOINT ARRANGEMENT
1/2" = 1'-0"



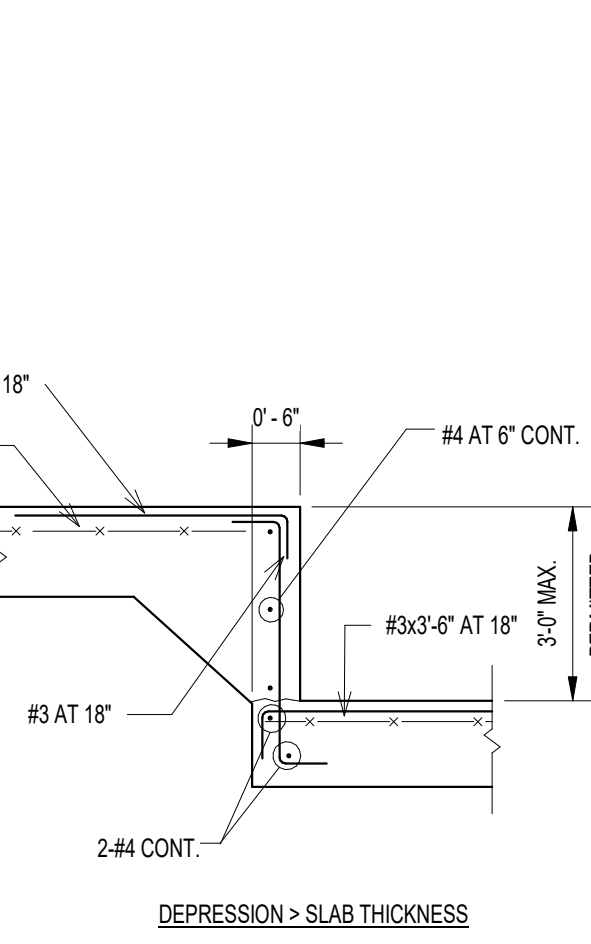
8 SLAB-ON-GRADE JOINTS AT COLUMNS, PIERS AND WALLS
1/2" = 1'-0"



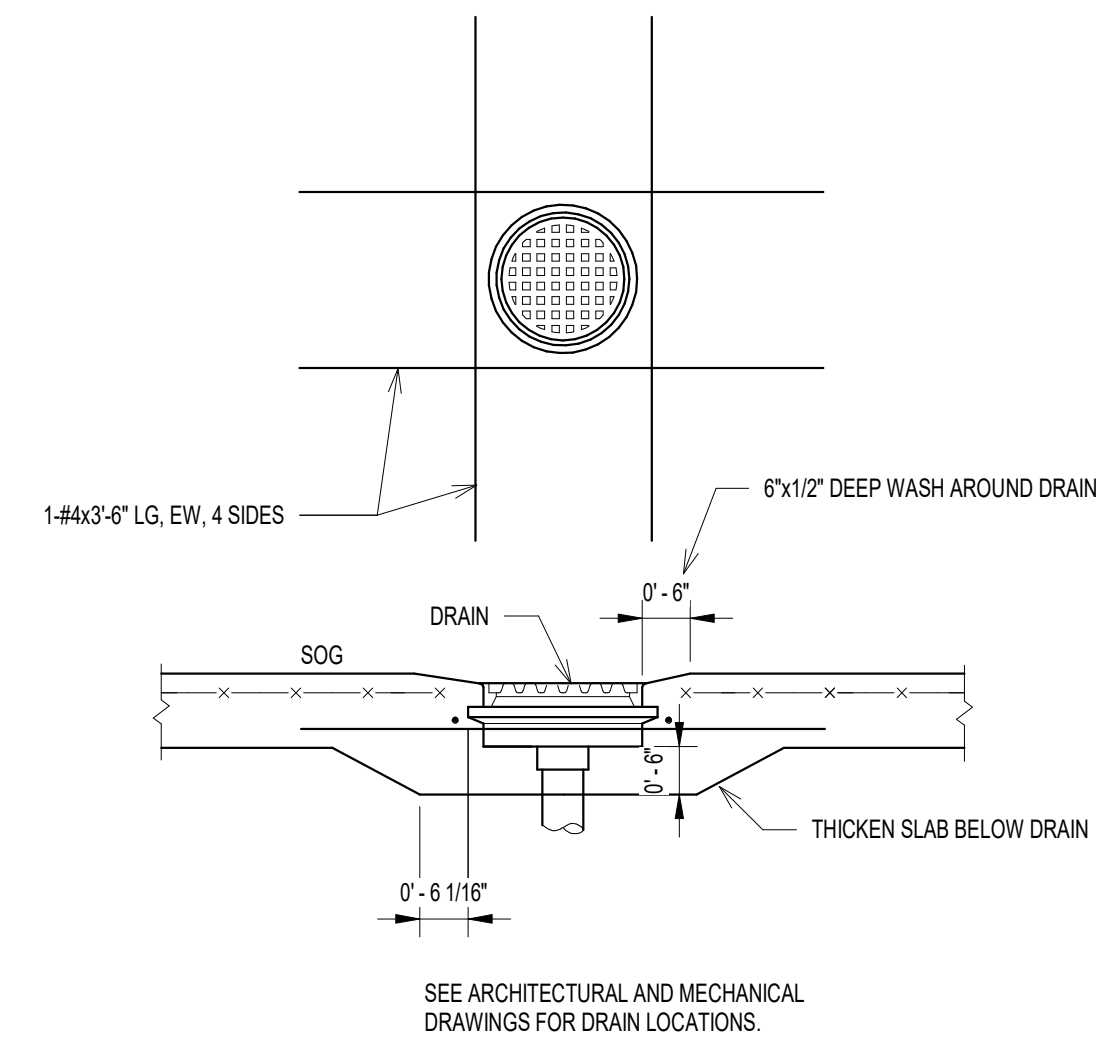
9 EXCAVATION PARALLEL TO FOOTINGS
1/2" = 1'-0"



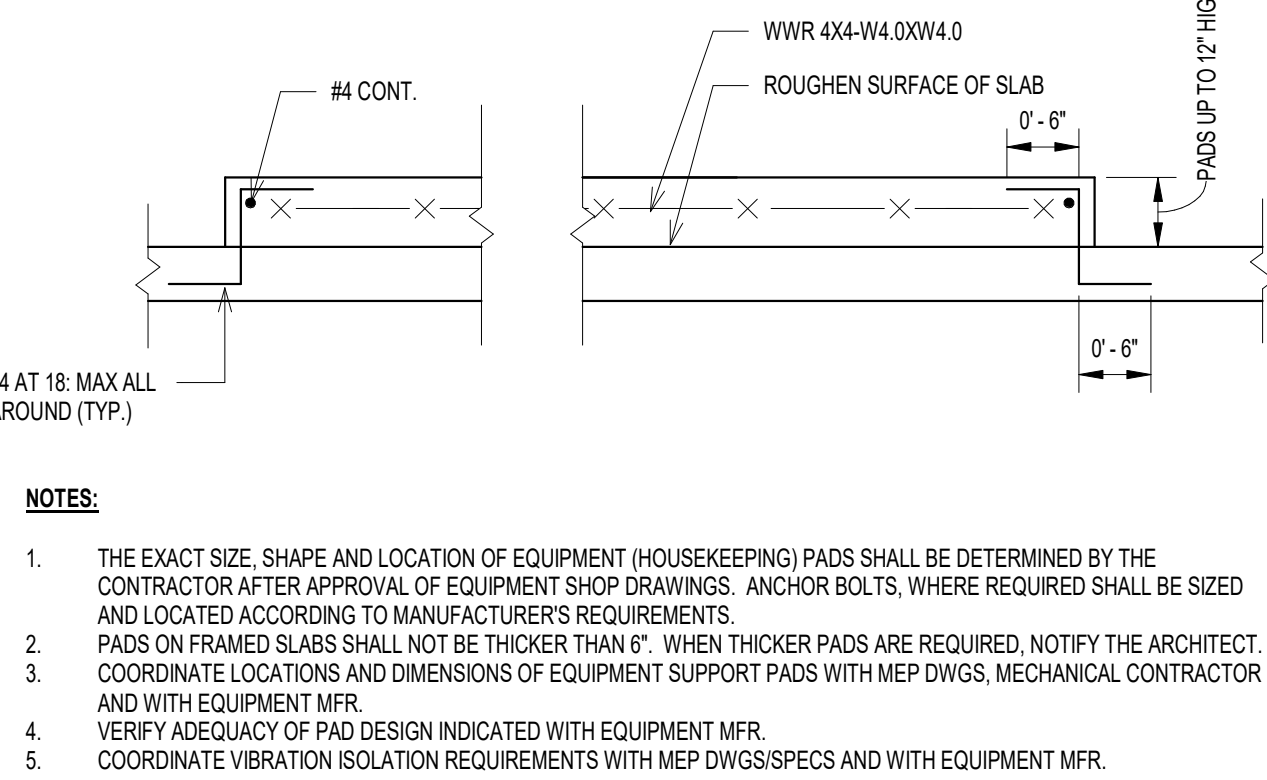
10 ADJACENT FOOTING TO EXCAVATION
3/4" = 1'-0"



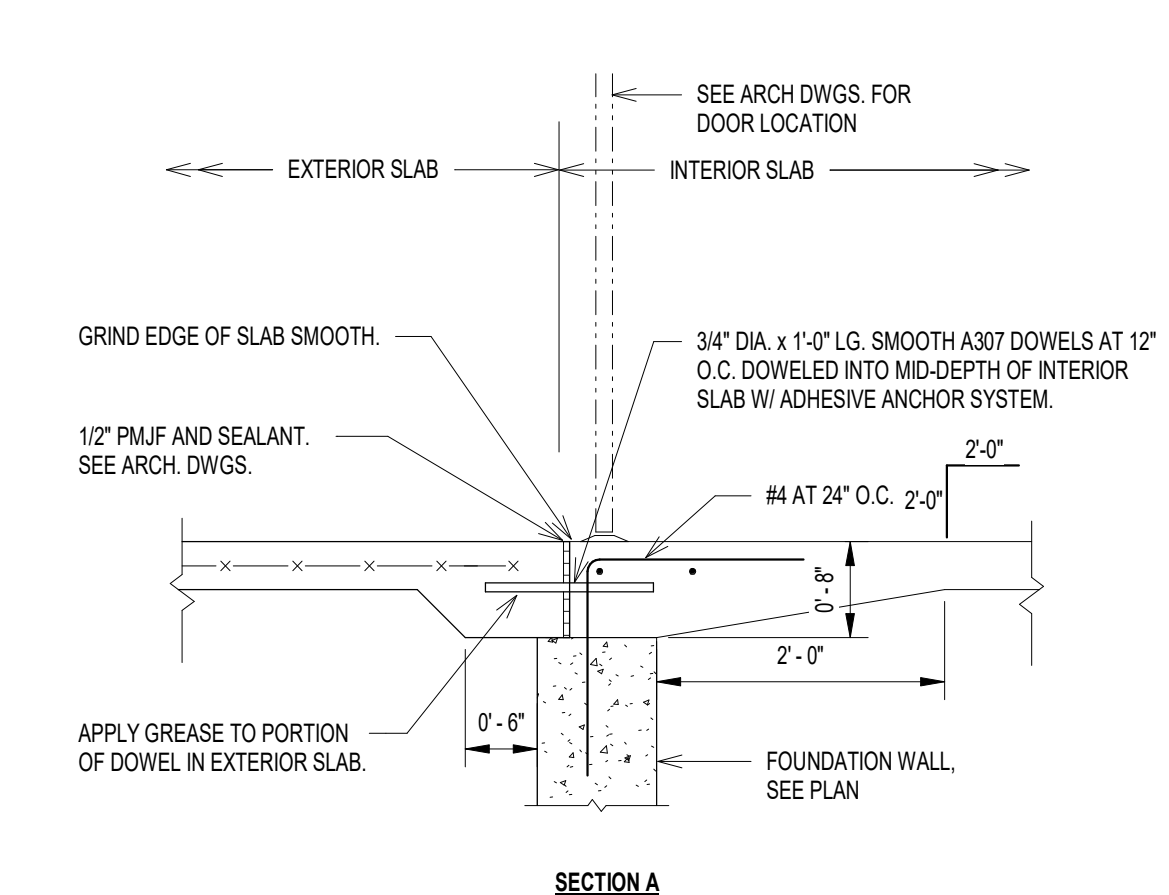
11 SLAB ON GRADE DEPRESSION AND STEP
1/2" = 1'-0"



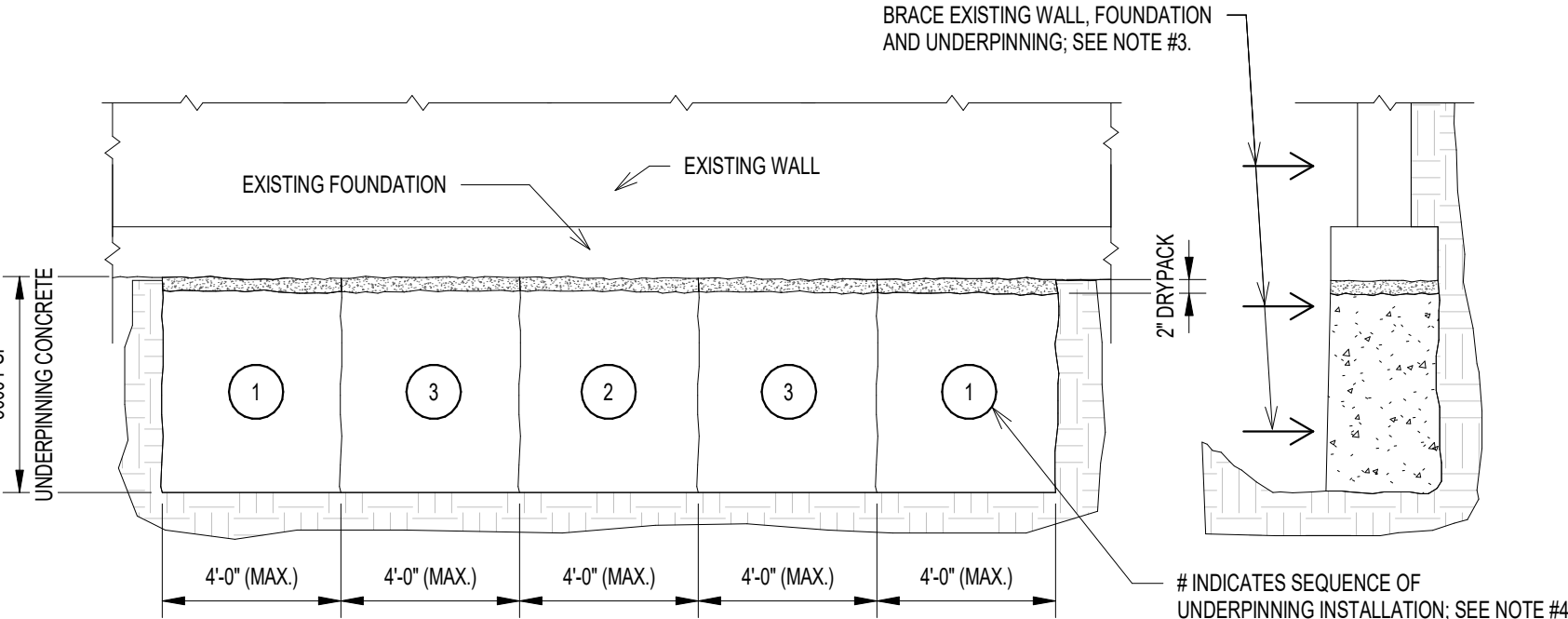
12 FLOOR DRAIN IN SLAB-ON-GRADE
1/2" = 1'-0"



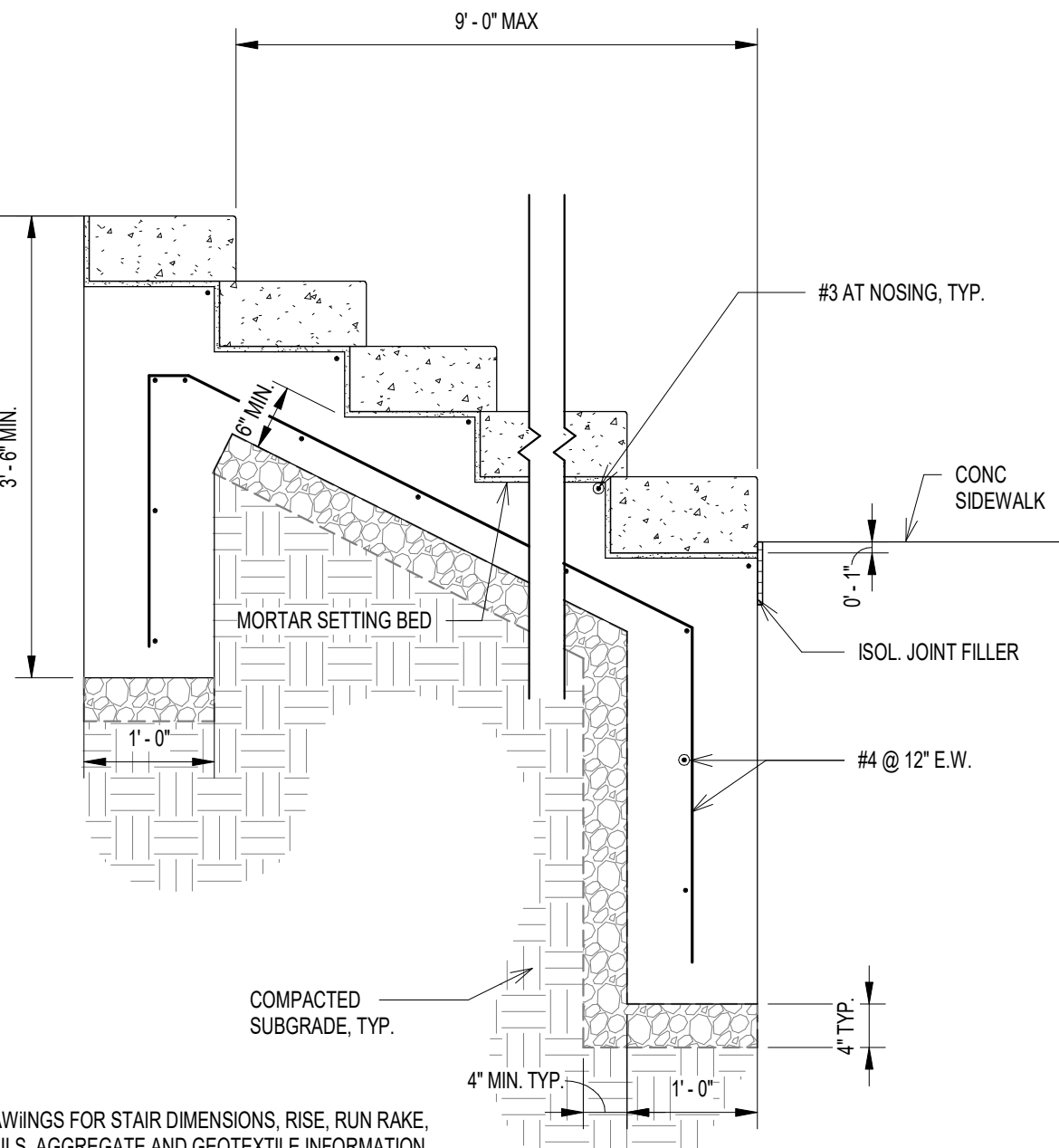
13 CONCRETE PAD FOR INTERIOR EQUIPMENT
3/4" = 1'-0"



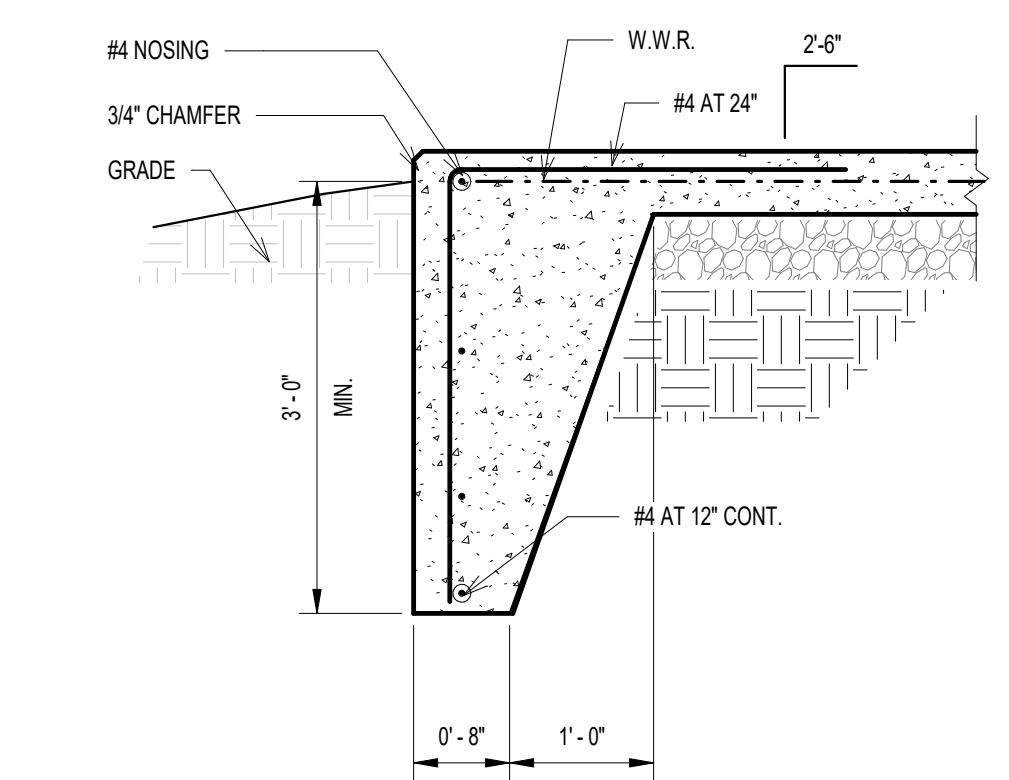
14 SOG AT EXTERIOR DOOR
3/4" = 1'-0"



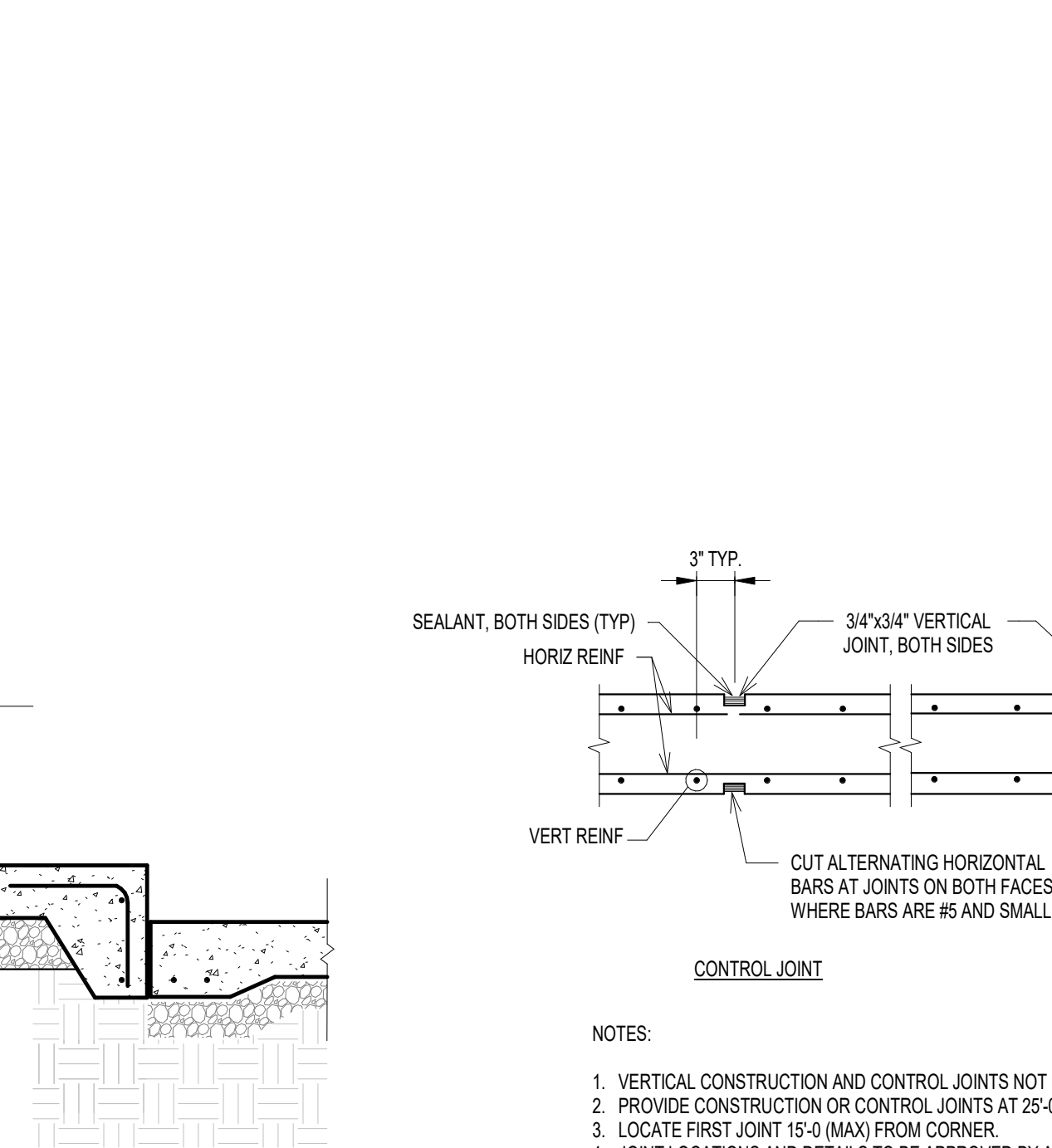
15 UNDERPINNING BELOW EXISTING WALL FOUNDATION
1/2" = 1'-0"



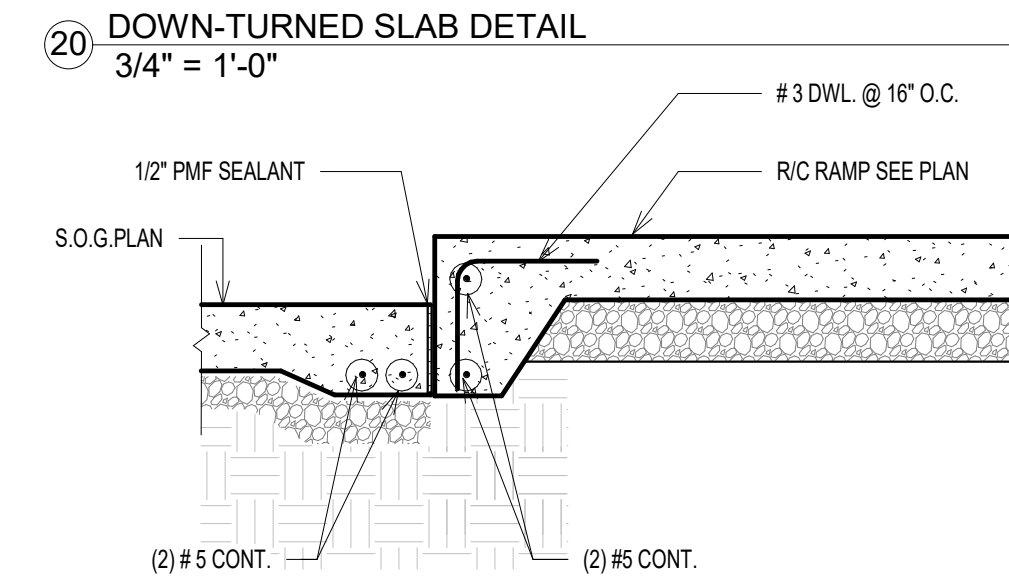
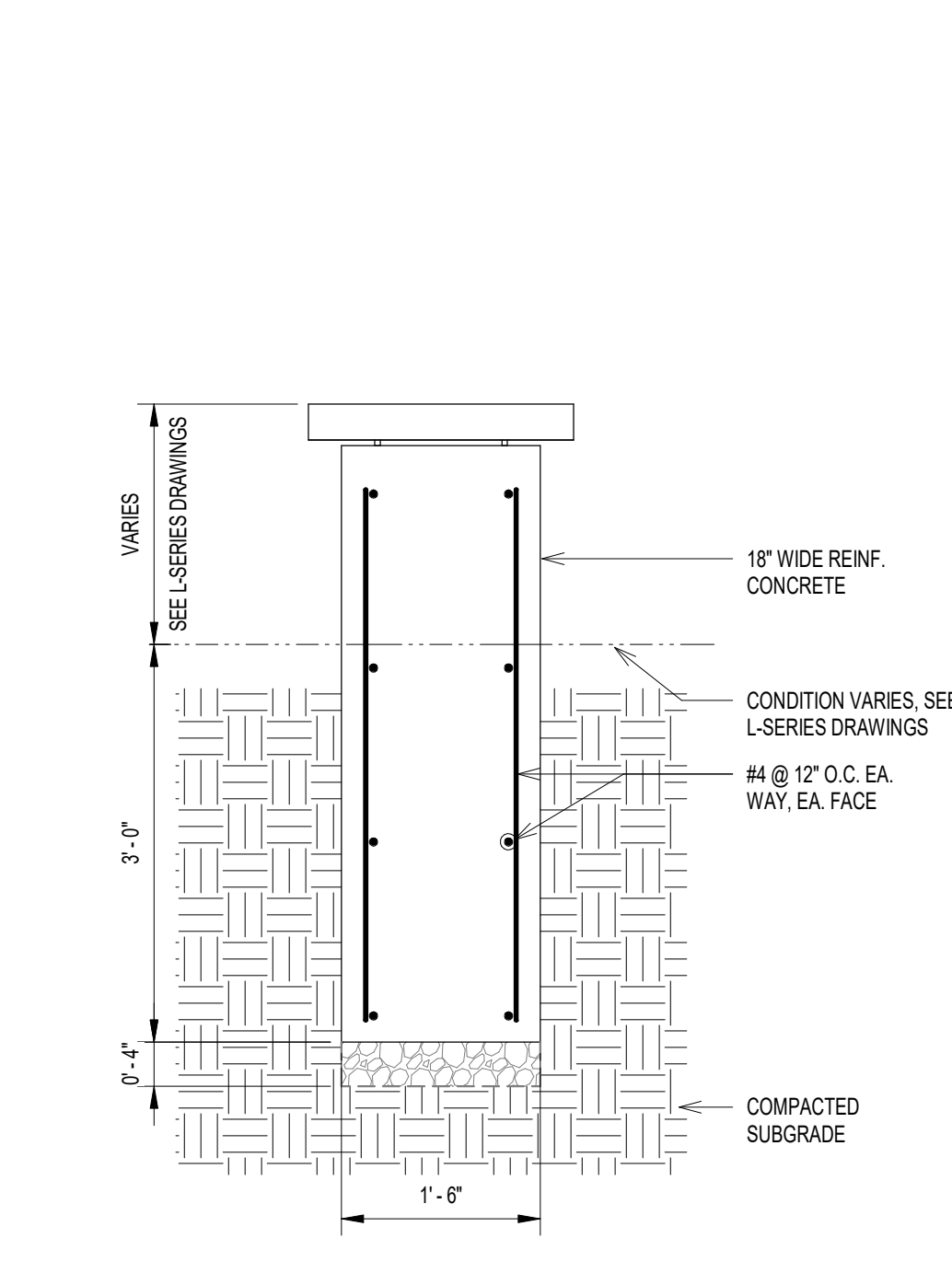
16 BLUESTONE STEPS
3/4" = 1'-0"



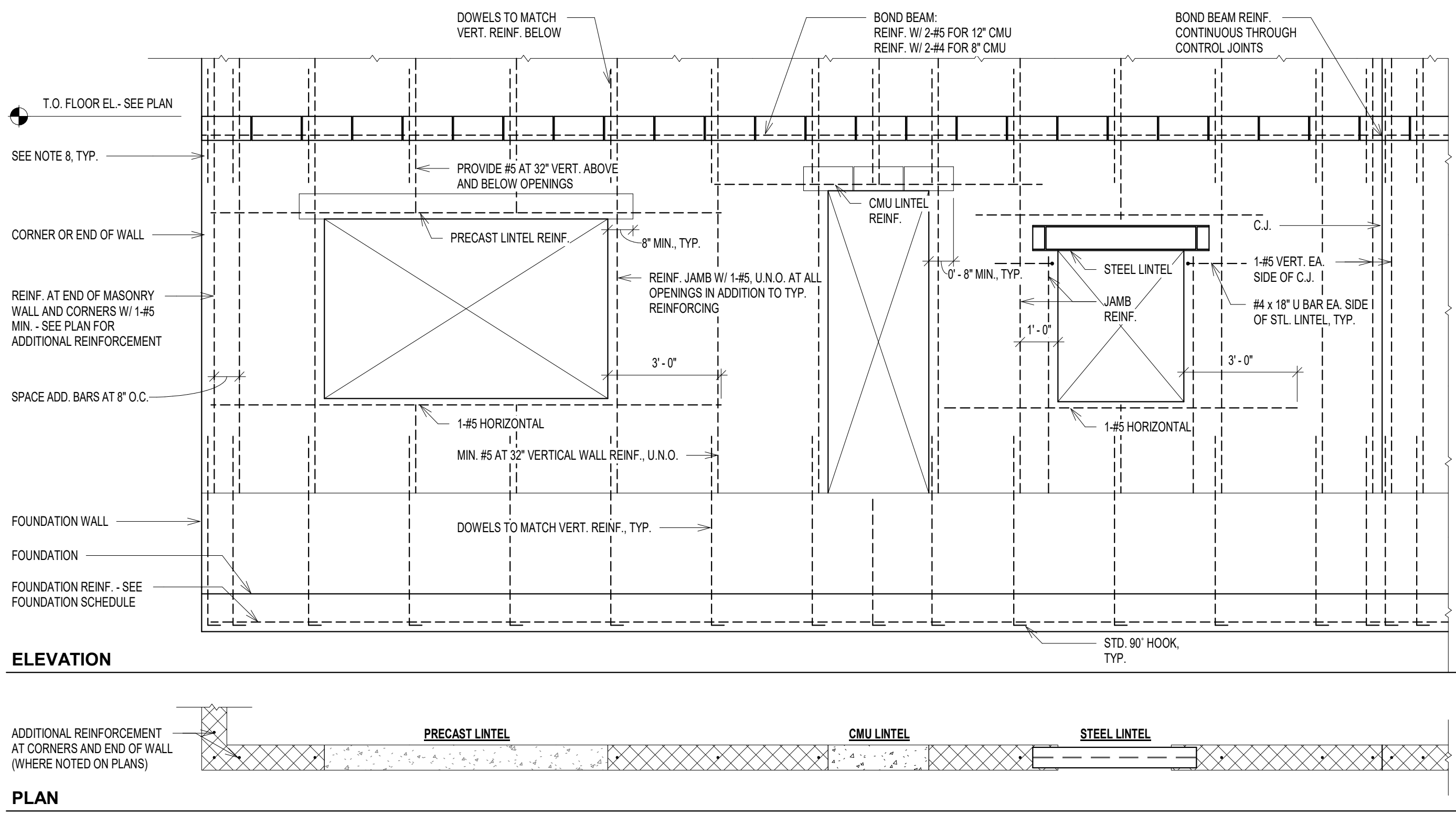
17 REINFORCED CONCRETE RAMP ON GRADE (INTERIOR)
1/2" = 1'-0"



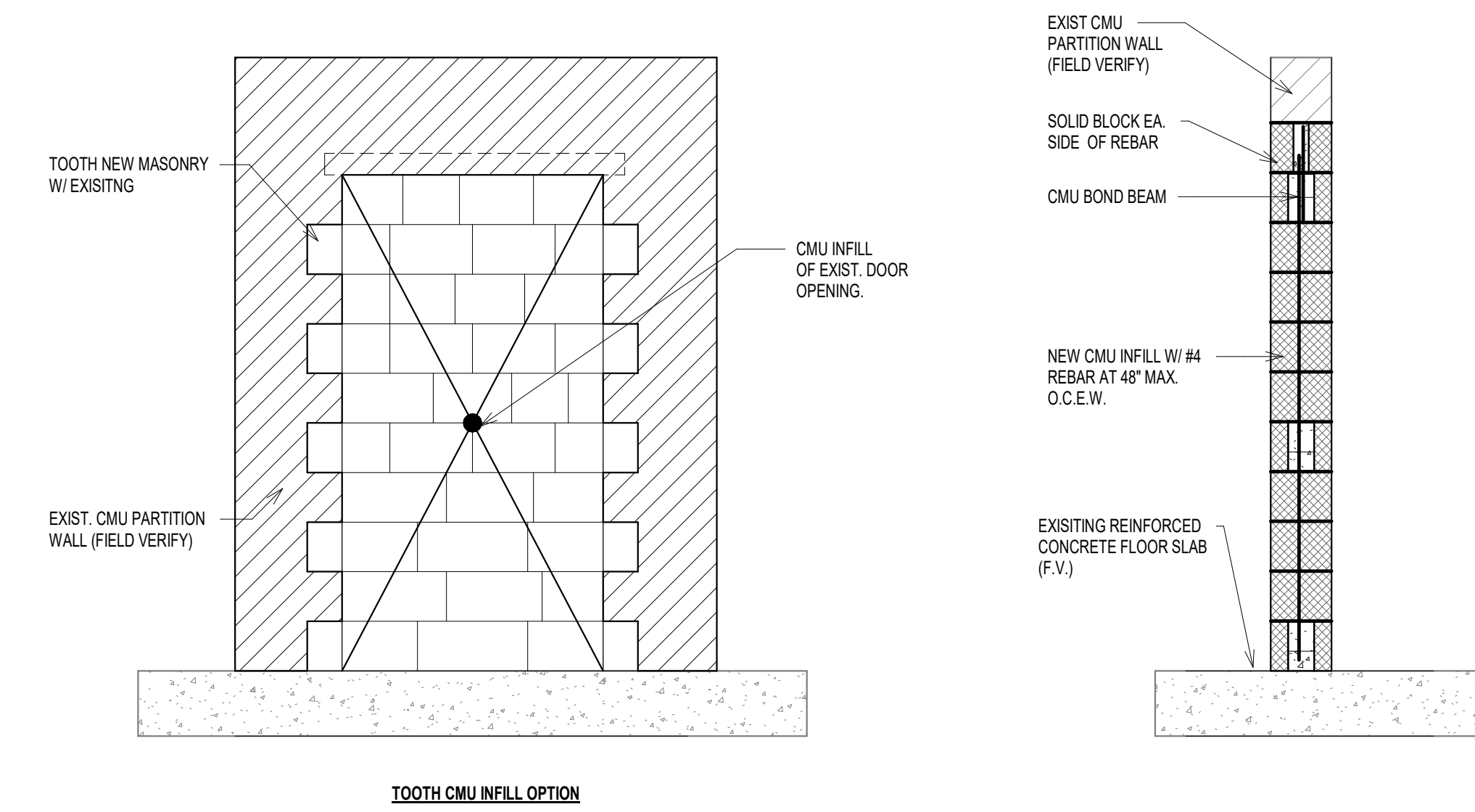
18 VERTICAL CONSTRUCTION AND CONTROL JOINTS IN CONCRETE WALL
1/2" = 1'-0"



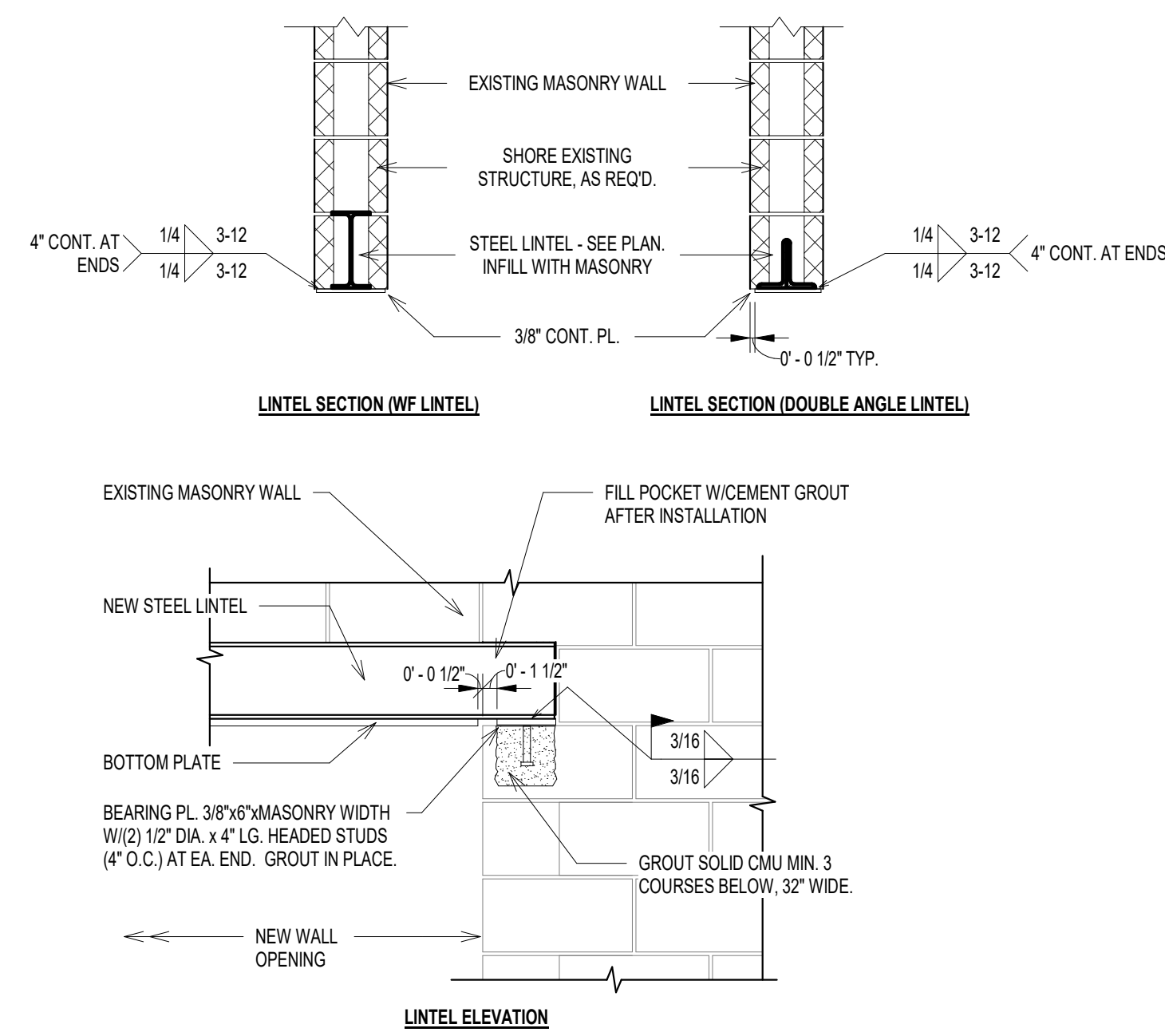
19 SEATWALL
3/4" = 1'-0"



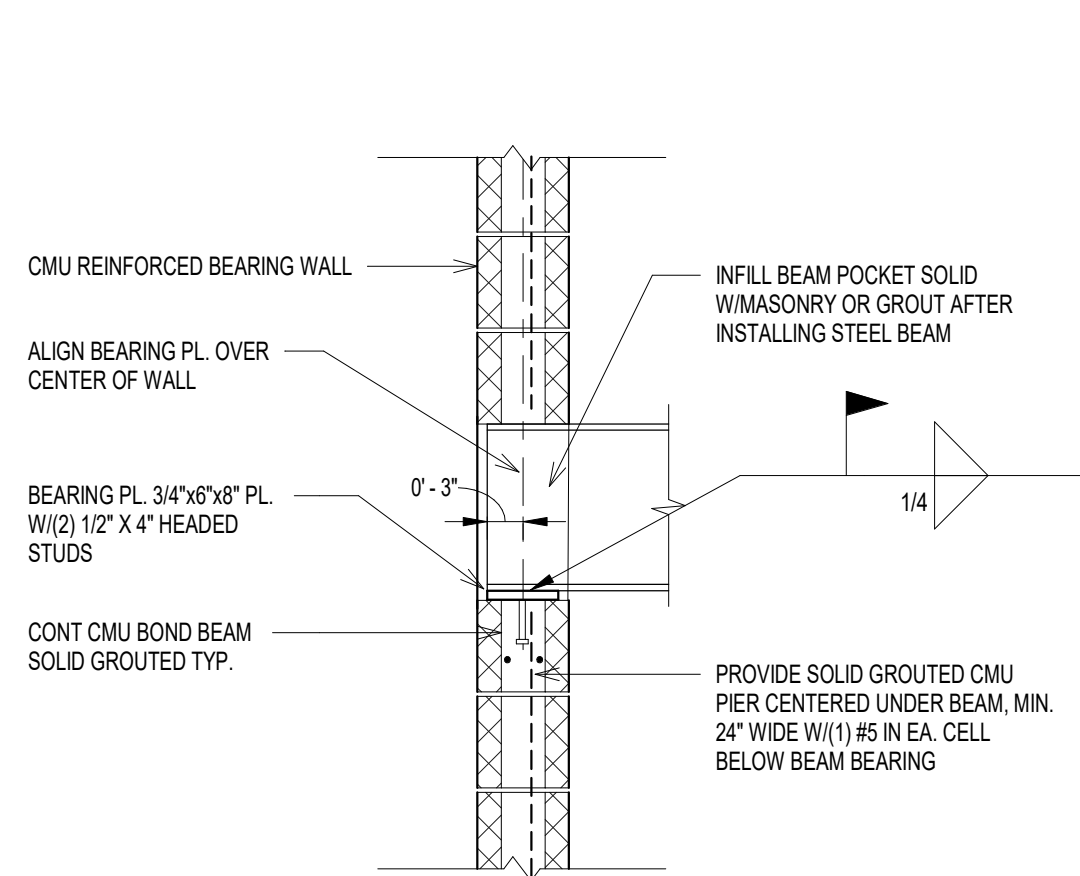
1 TYPICAL CMU WALL REINFORCING DETAILS
3/8" = 1'-0"



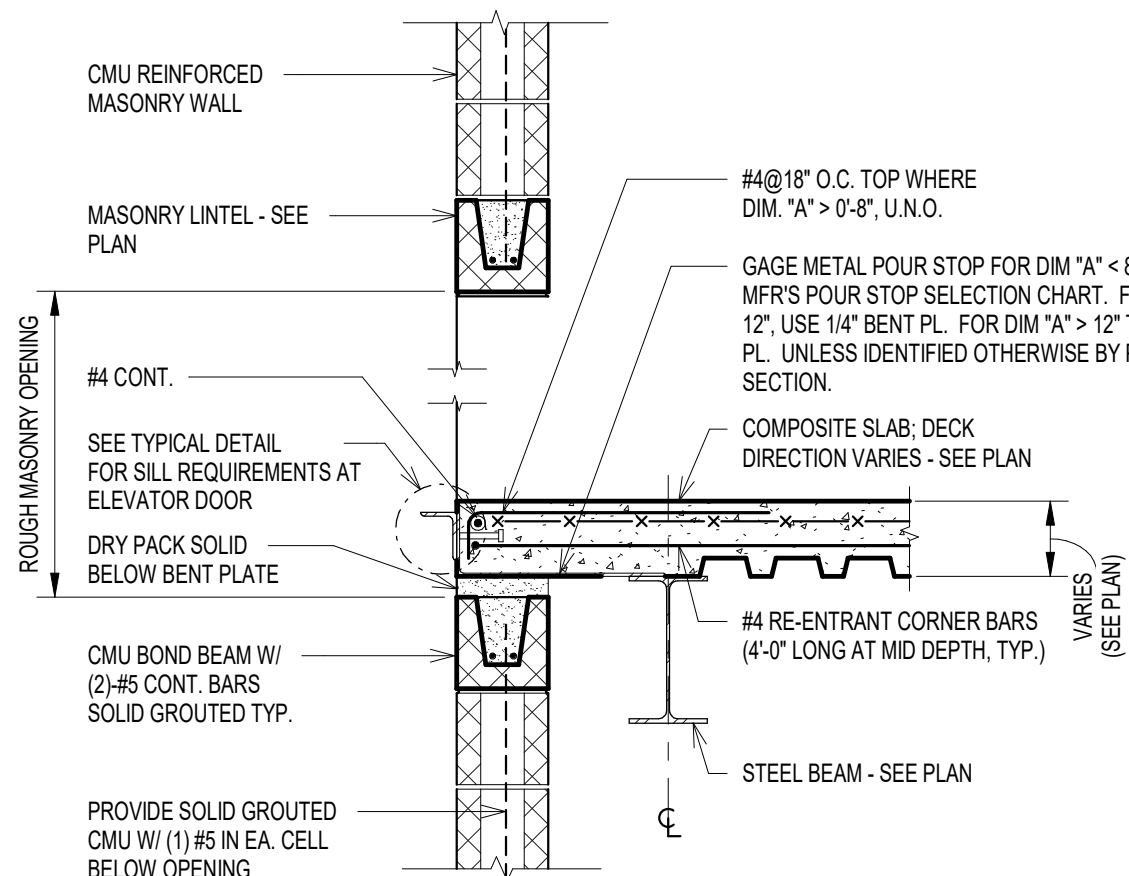
2 TYPICAL MASONRY OPENING INFILL DETAILS
3/8" = 1'-0"



3 STEEL LINTEL SECTION AT EXISTING CMU
3/4" = 1'-0"

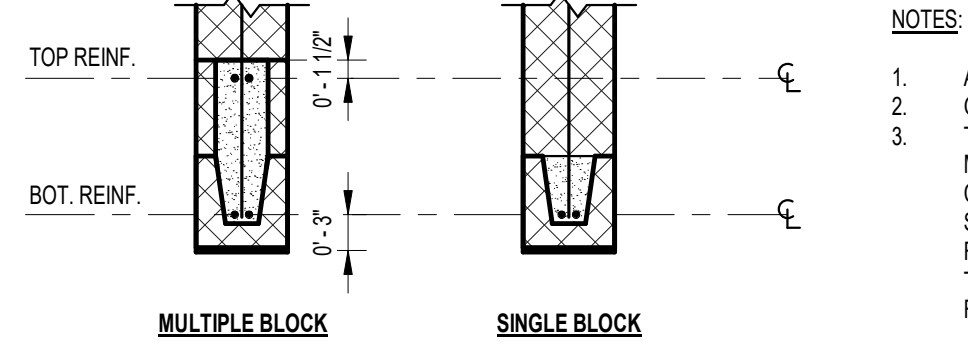


4 BEAM BEARING PERPENDICULAR TO MASONRY WALL
3/4" = 1'-0"

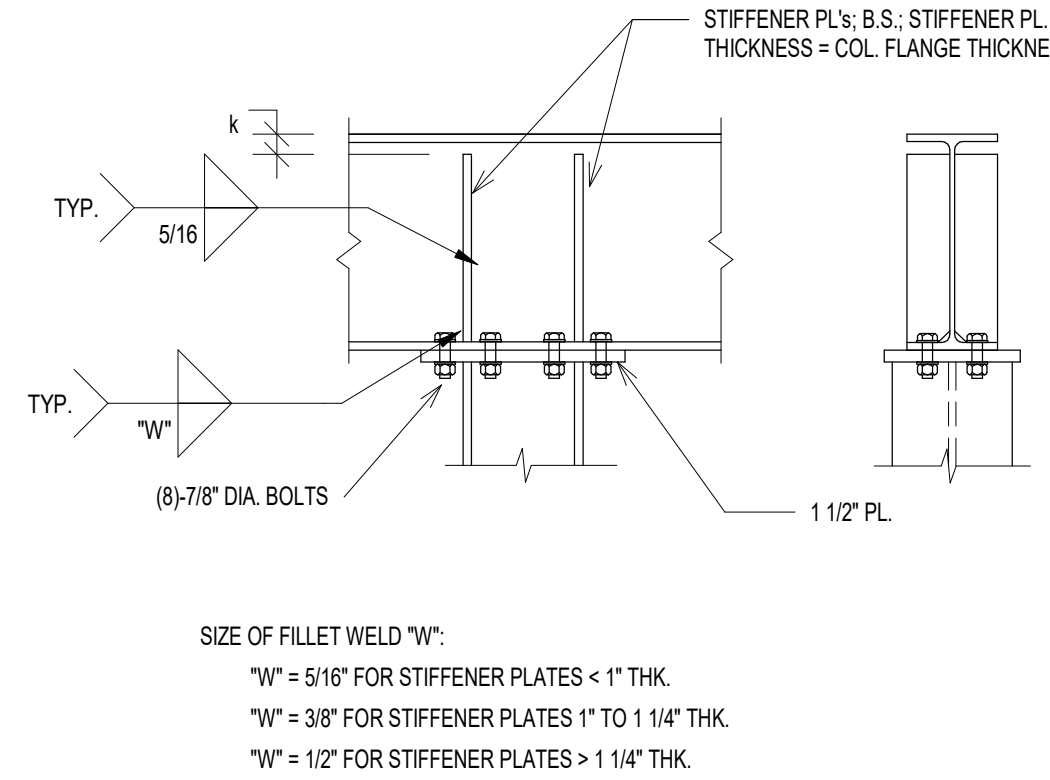


5 SLAB EDGE PARALLEL TO MASONRY ELEVATOR OPENINGS
3/4" = 1'-0"

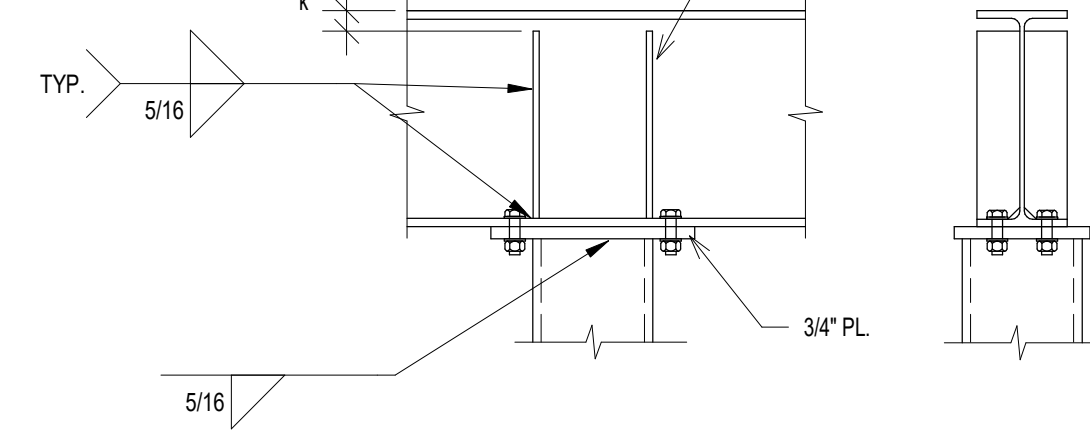
MASONRY LINTEL SCHEDULE					
MARK	WALL WIDTH	LINTEL DEPTH	REINFORCING STEEL	JAMB WIDTH	JAMB REINF.
ML1	8"	8"	1-#5 BOT.	8"	2-#5 VERT.
ML2	8"	16"	2-#5 TOP & BOT.	8"	2-#5 VERT.
-	-	-	-	-	-



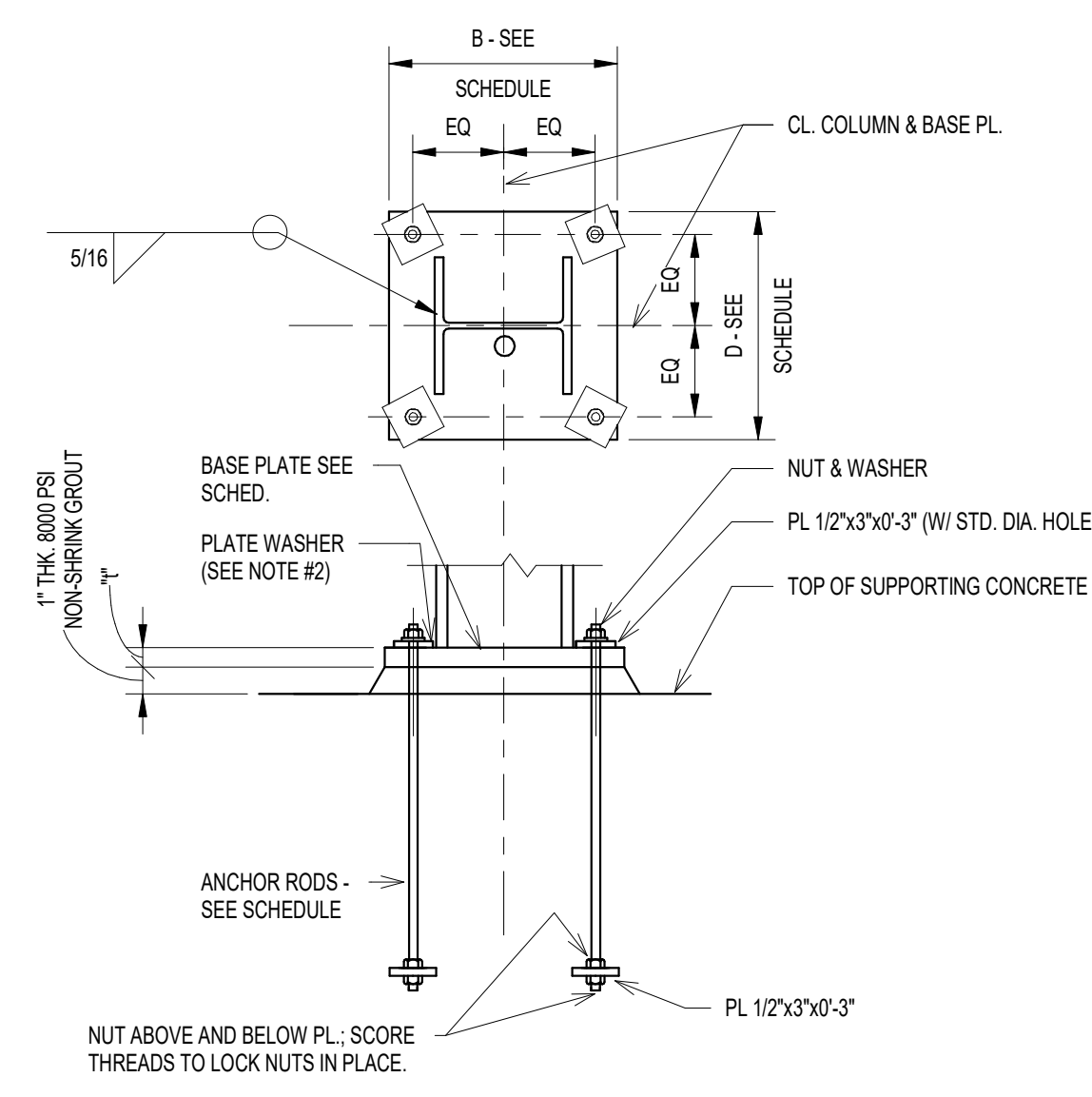
6 MASONRY LINTEL SCHEDULE
3/4" = 1'-0"



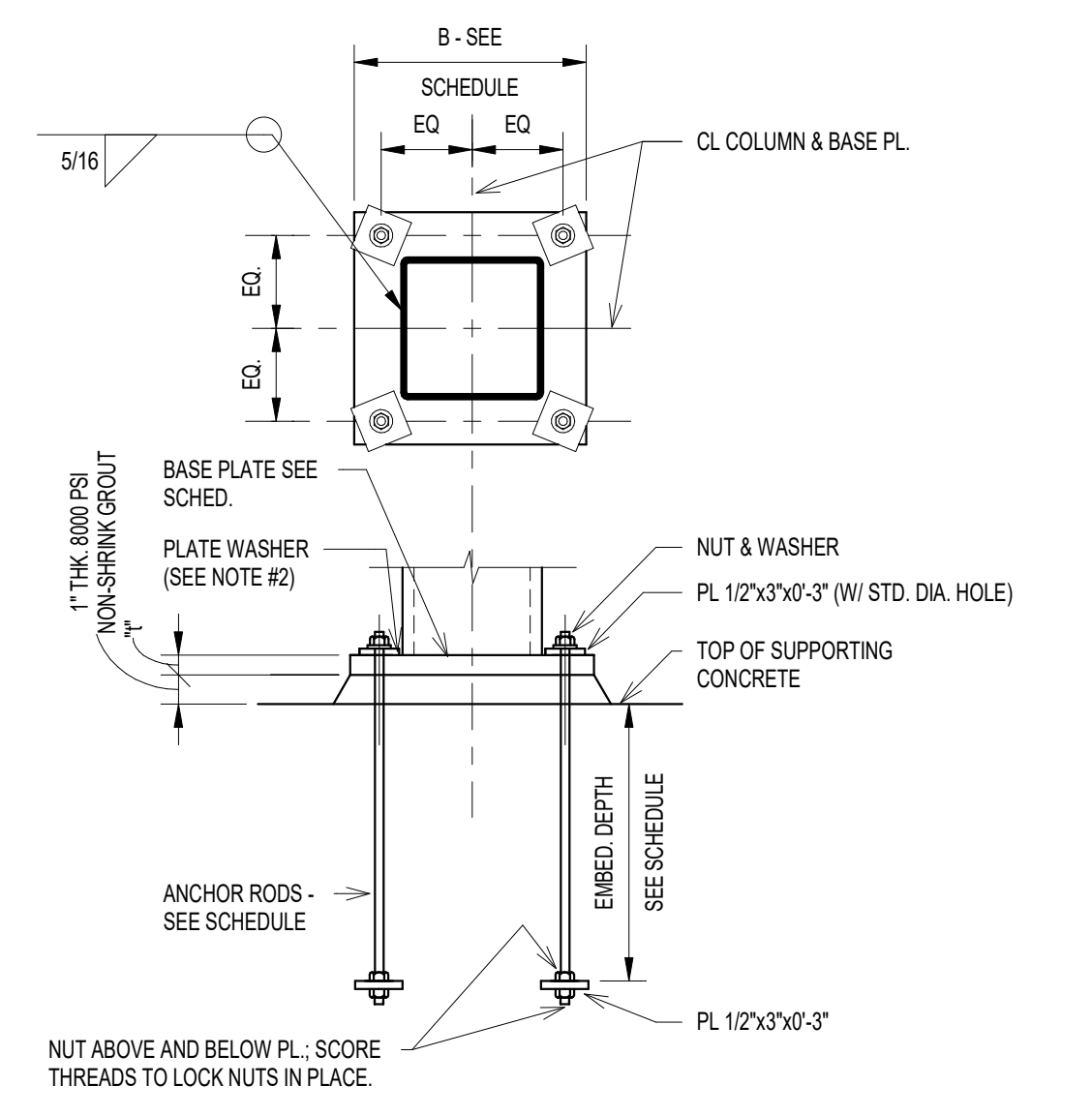
7 BEAM OVER COLUMN CONNECTION
3/4" = 1'-0"



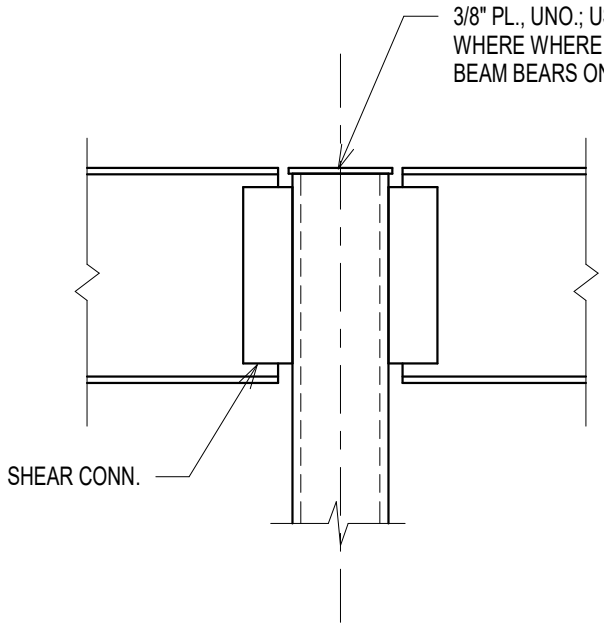
8 BEAM OVER HSS COLUMN CONNECTION
3/4" = 1'-0"



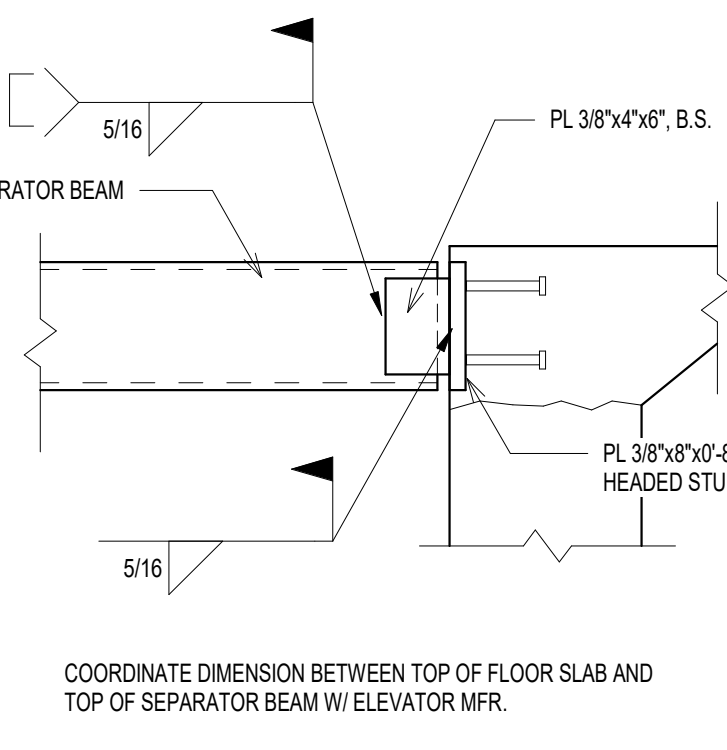
9 COLUMN BASE PLATE WITH HEADED ANCHOR RODS
3/4" = 1'-0"



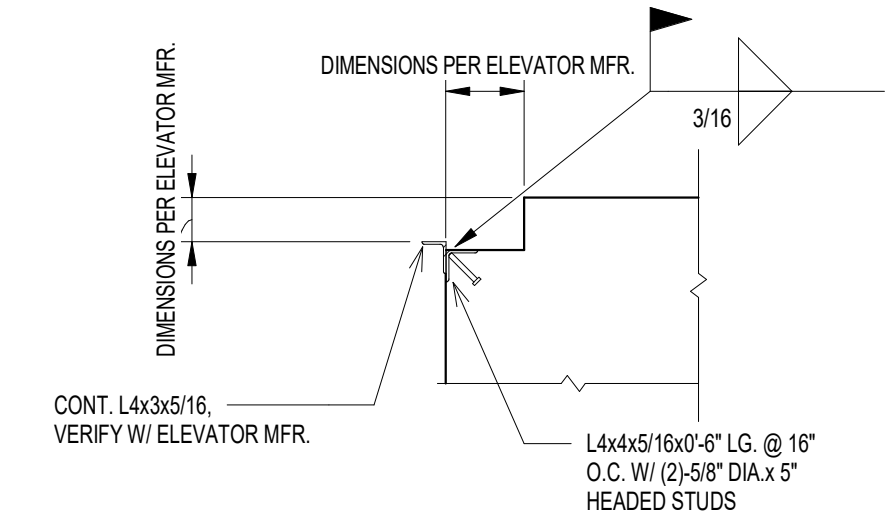
10 HSS COLUMN PLATE AND ANCHOR RODS
3/4" = 1'-0"



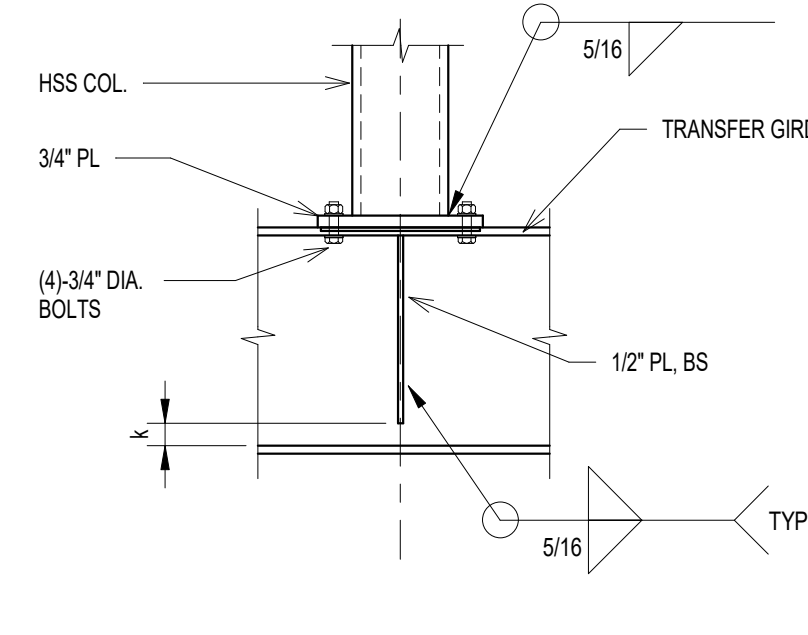
11 BEAM CONNECTION TO HSS OR PIPE COLUMN
3/4" = 1'-0"



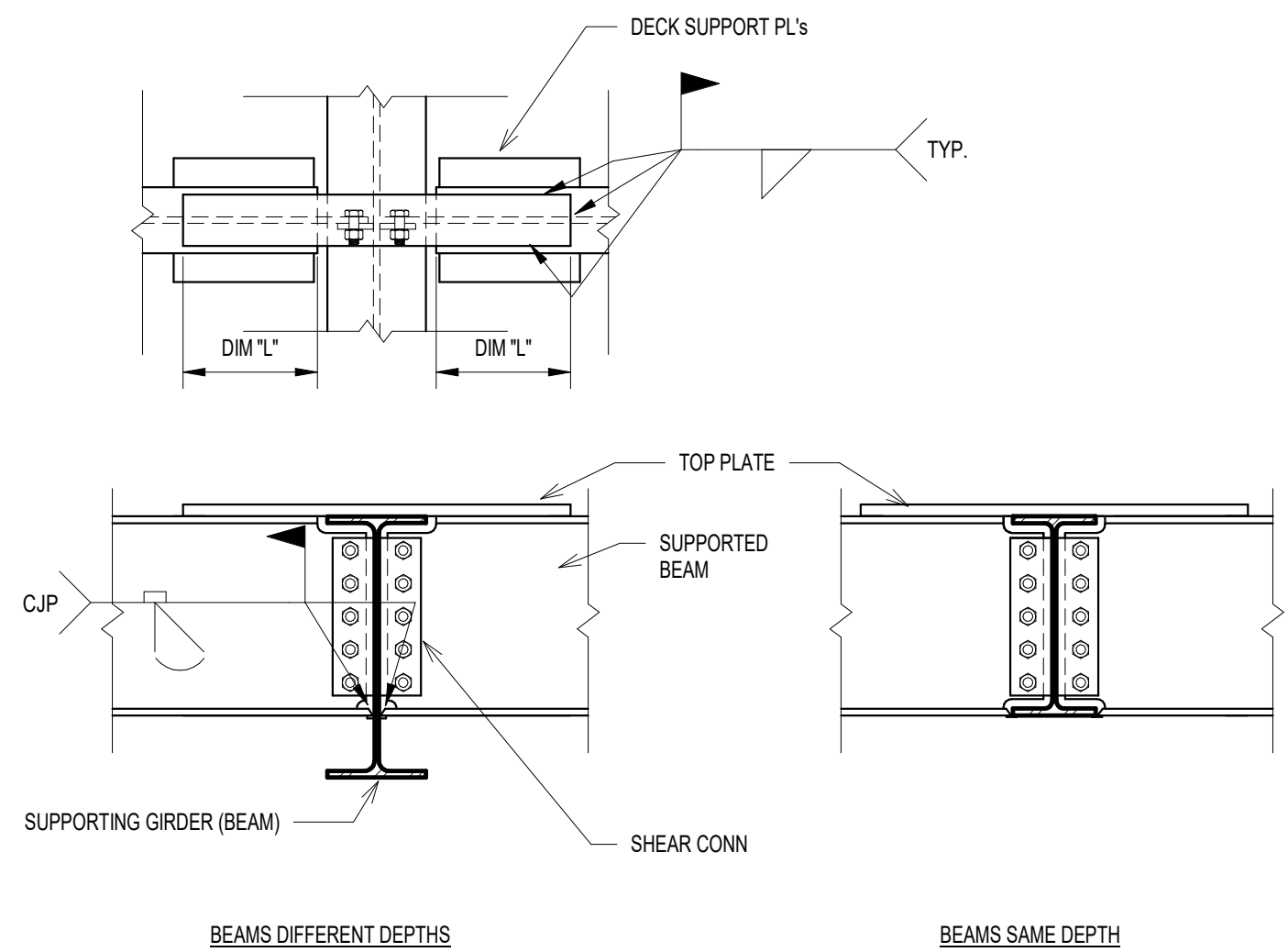
12 ELEVATOR SEPARATOR BEAM CONNECTION TO CONC. WALL
1" = 1'-0"



13 SILL AT ELEVATOR DOOR
1/2" = 1'-0"

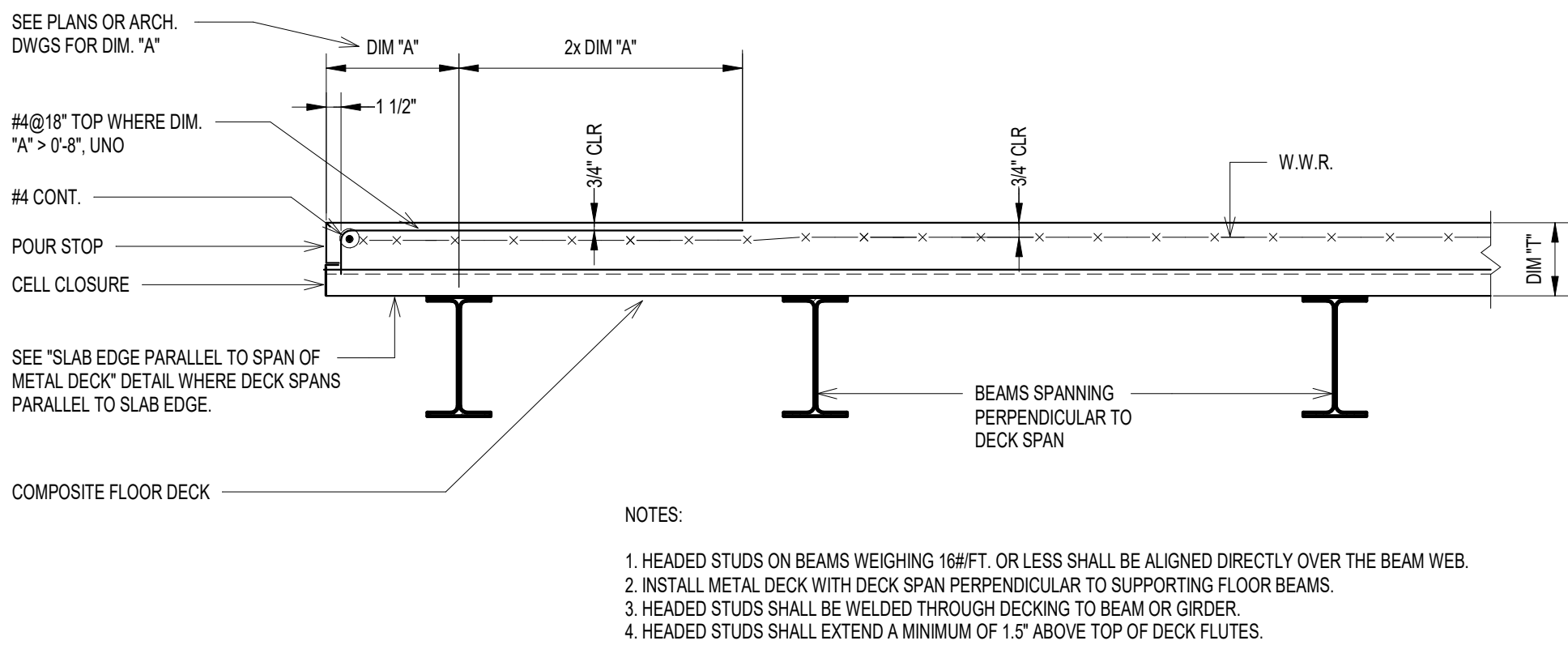


14 HSS COLUMN CONN. TO TRANSFER GIRDER
3/4" = 1'-0"



SEE SCHEDULE FOR SIZES AND QUANTITIES.

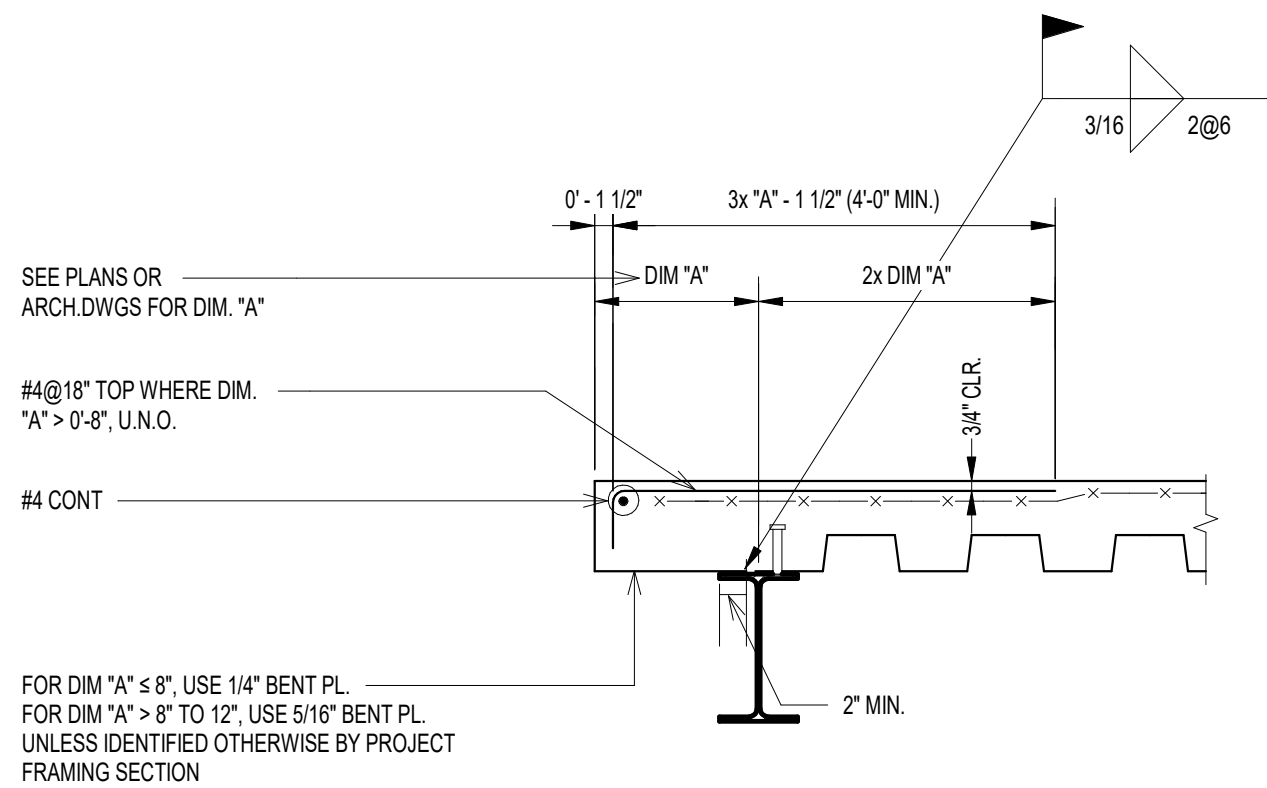
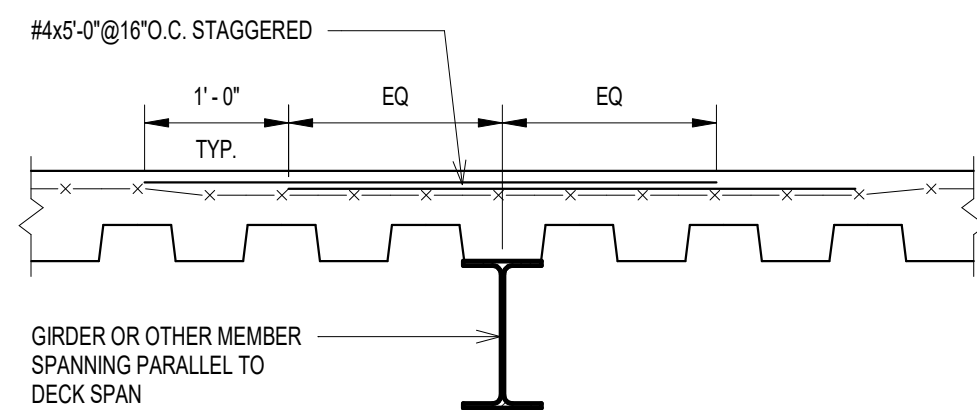
1 BEAM TO GIRDER MOMENT CONNECTION FIELD WELDED
3/4" = 1'-0"



NOTES:

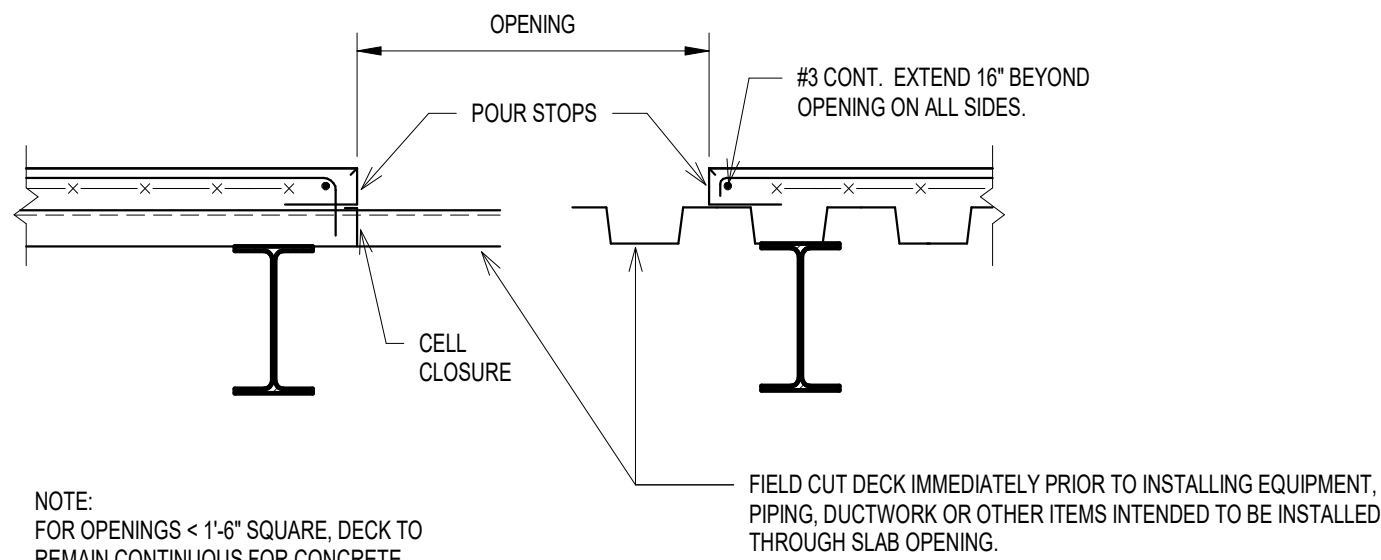
1. HEADED STUDS ON BEAMS WEIGHING 16#/FT. OR LESS SHALL BE ALIGNED DIRECTLY OVER THE BEAM WEB.
2. INSTALL METAL DECK WITH DECK SPAN PERPENDICULAR TO SUPPORTING FLOOR BEAMS.
3. HEADED STUDS SHALL BE WELDED THROUGH DECKING TO BEAM OR GIRDER.
4. HEADED STUDS SHALL EXTEND A MINIMUM OF 1.5" ABOVE TOP OF DECK FLUTES.

2 CONCRETE SLAB ON METAL DECK W/ COMPOSITE BEAM FRAMING
3/4" = 1'-0"

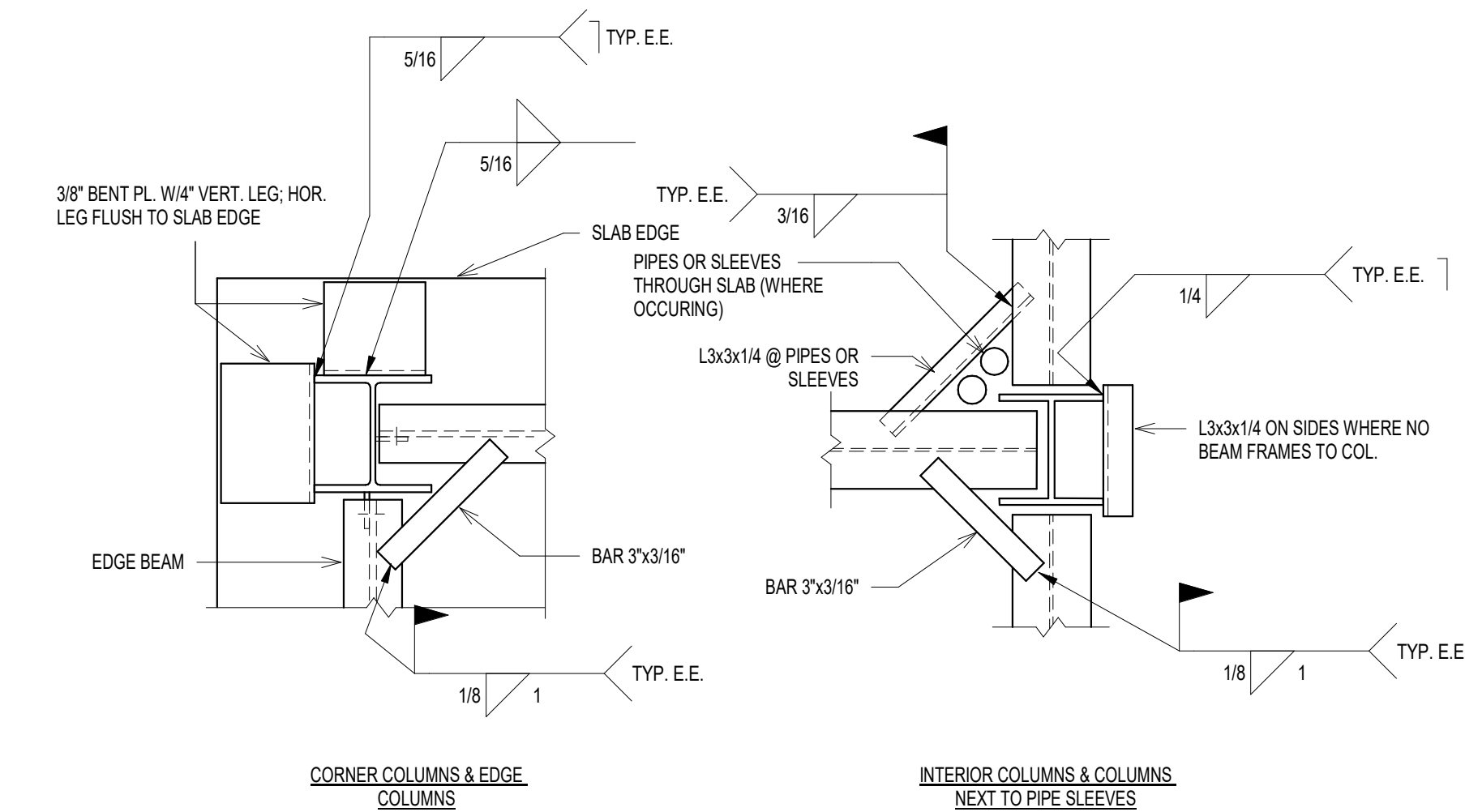


SEE "OPENING IN SLAB ON METAL DECK" DETAIL FOR SLAB EDGE DETAIL AT INTERIOR OPENINGS THAT ARE TEMPORARILY CLOSED OFF DURING CONSTRUCTION.

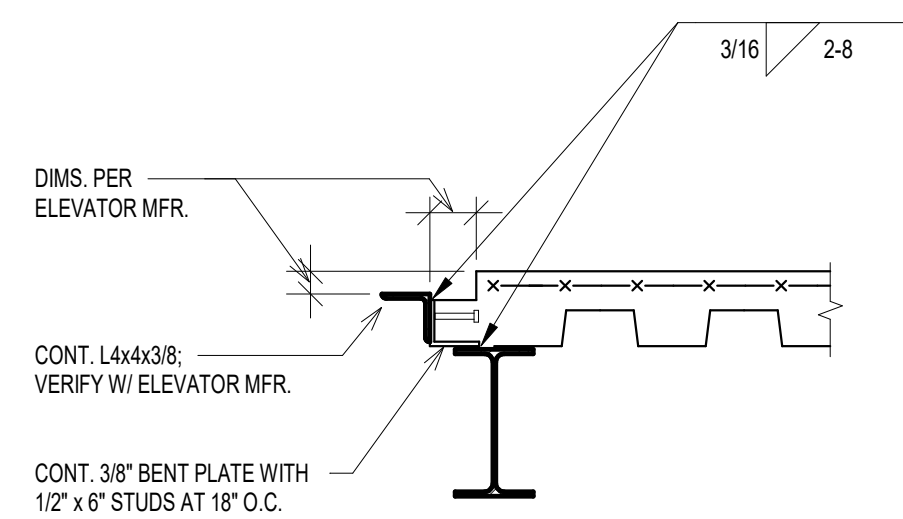
3 SLAB EDGE PARALLEL TO SPAN OF METAL DECK
3/4" = 1'-0"



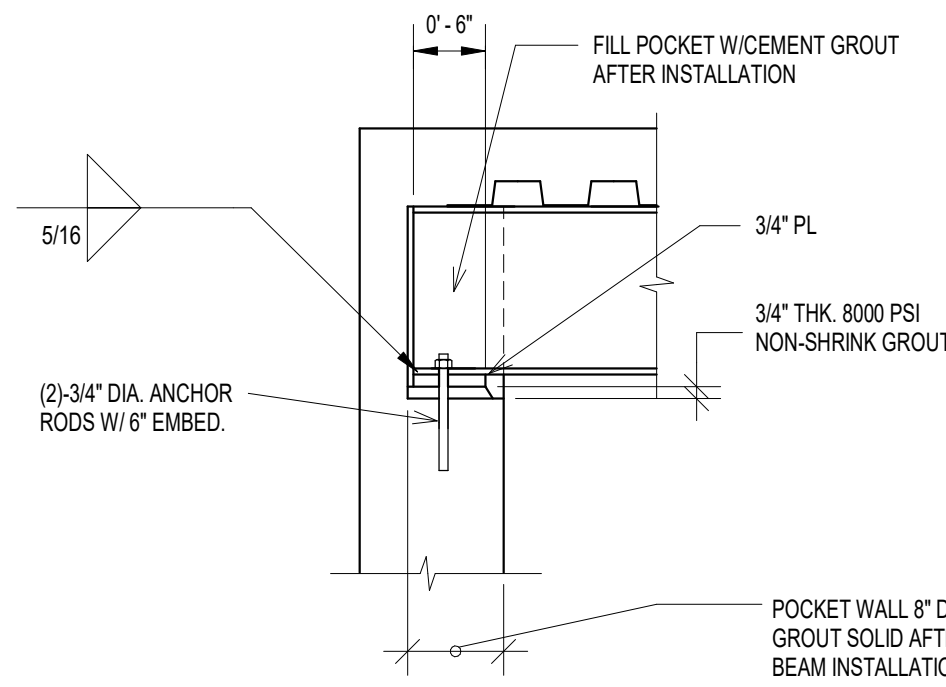
4 OPENING IN SLAB ON METAL DECK
3/4" = 1'-0"



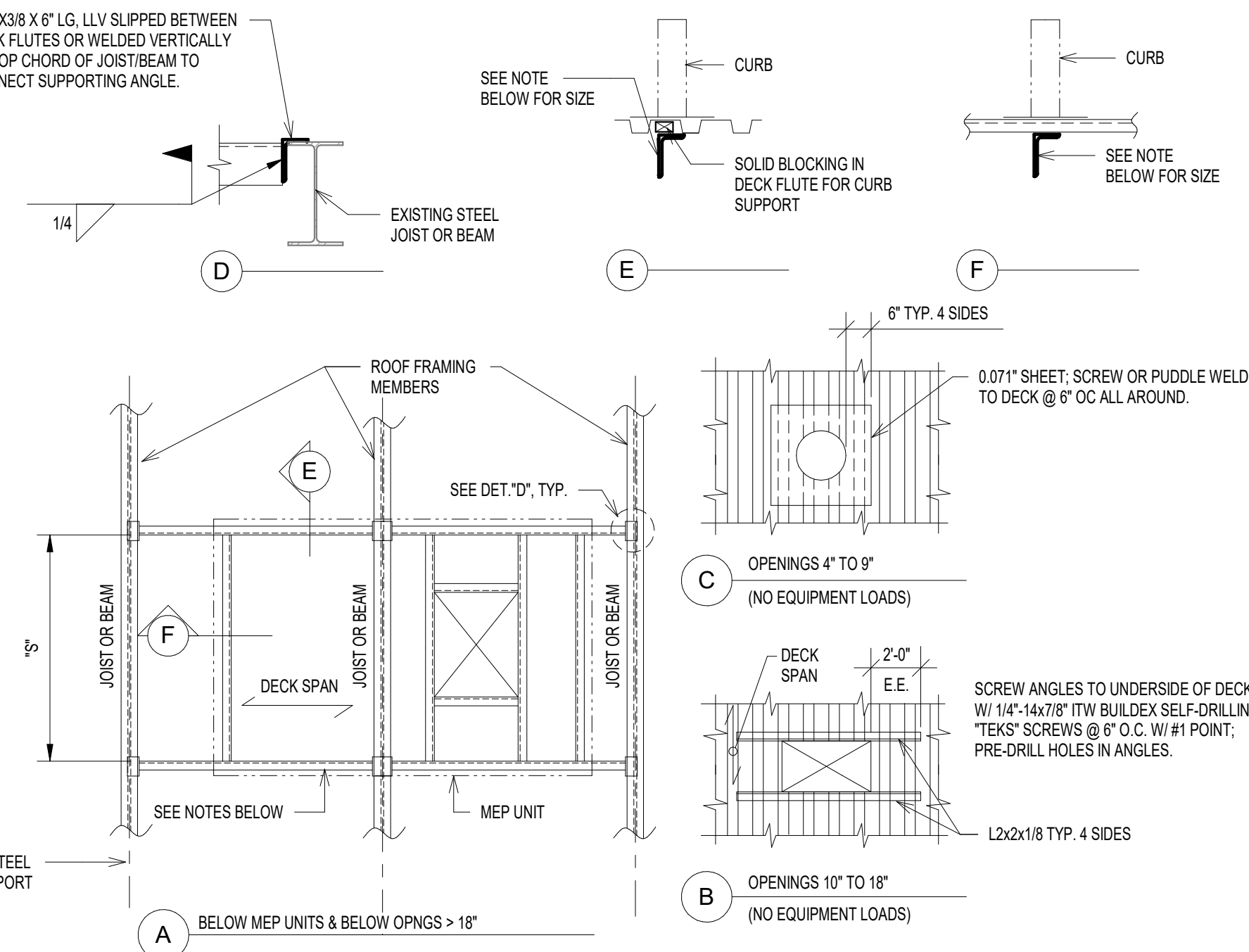
5 DECK SUPPORT AT COLUMNS
3/4" = 1'-0"



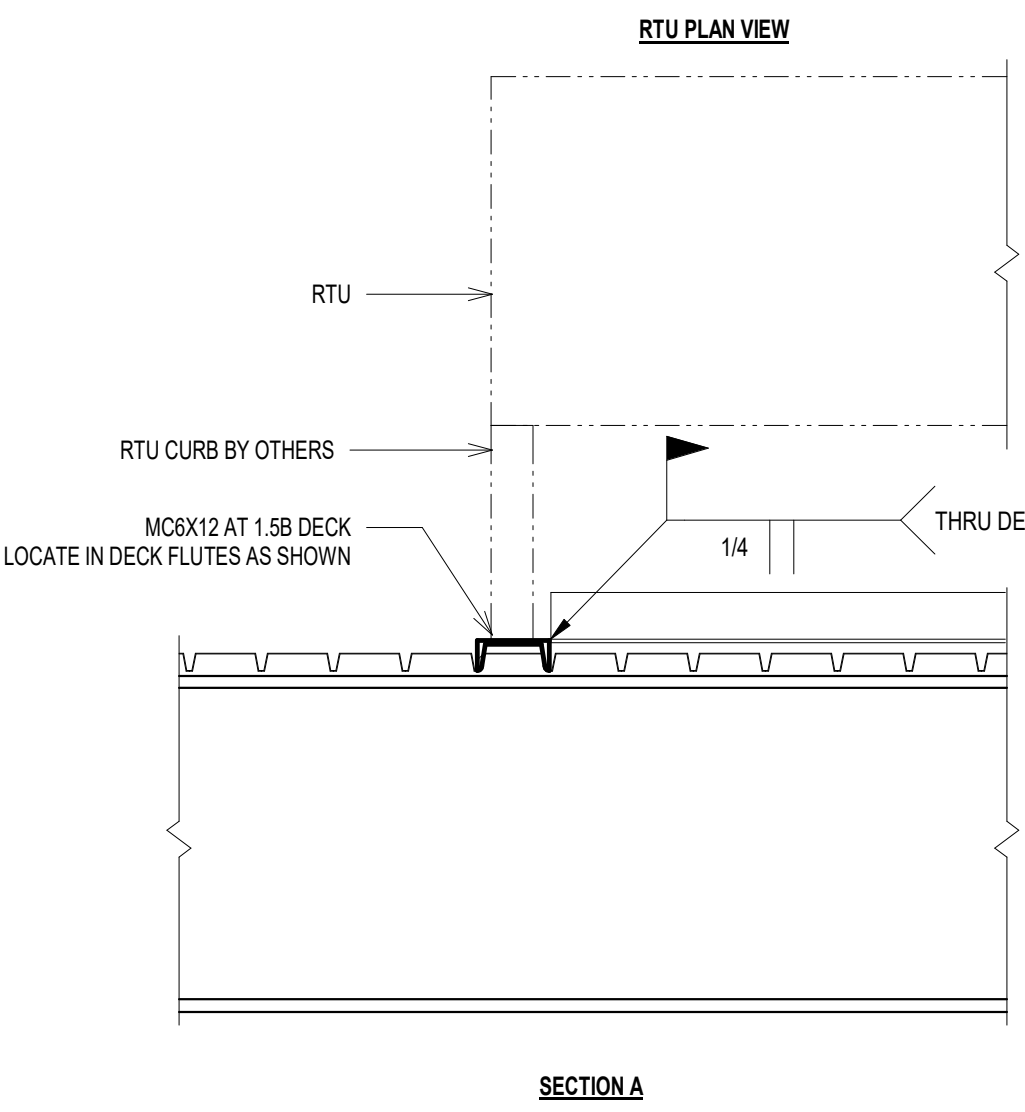
6 SILL AT ELEVATOR DOOR
3/4" = 1'-0"



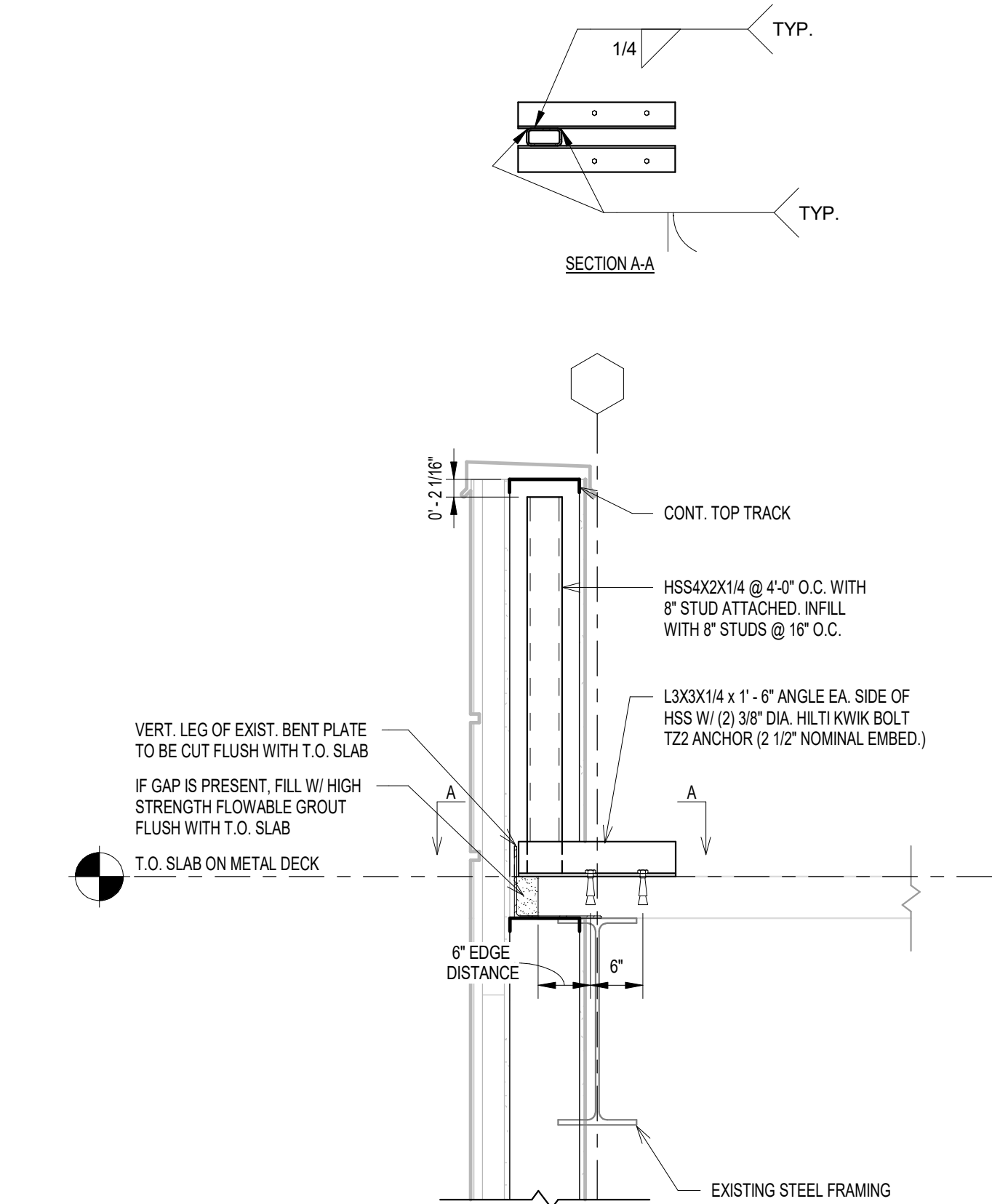
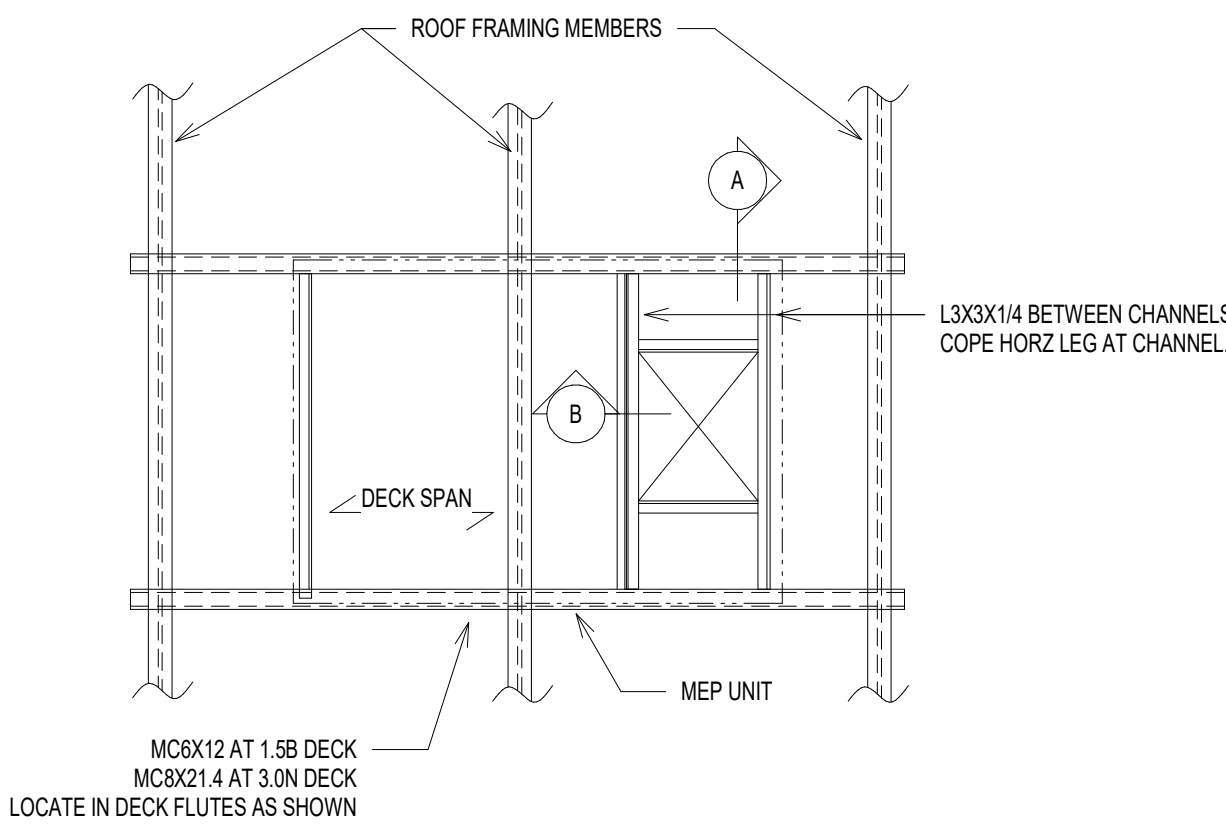
7 BEAM BEARING ON CONCRETE WALL
3/4" = 1'-0"



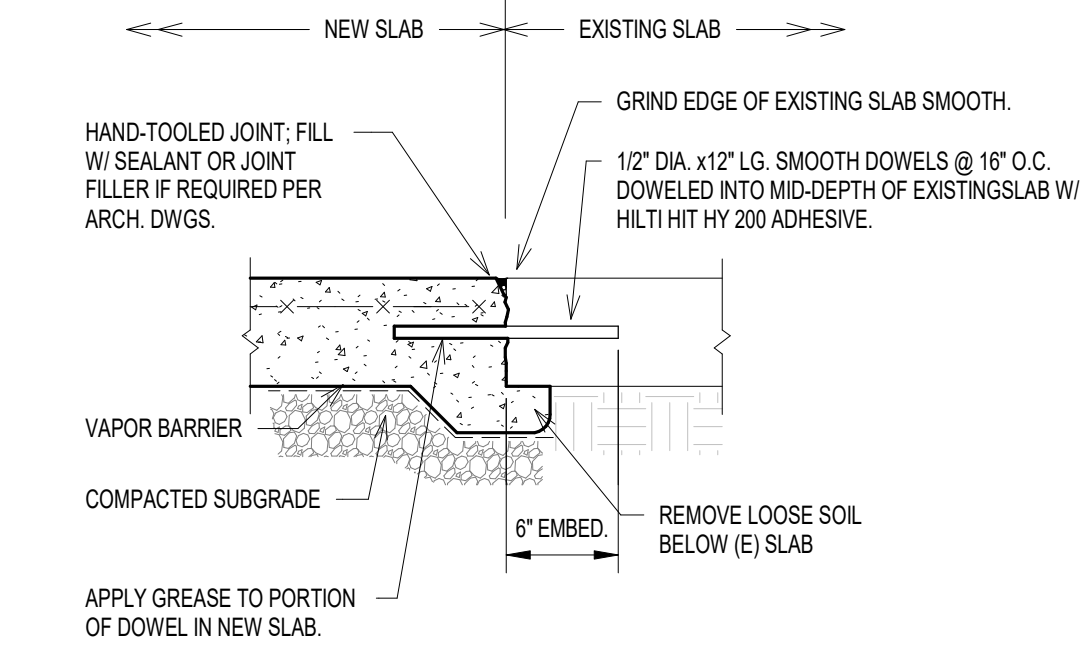
8 FRAMING BELOW EXISTING ROOF DECK OPENINGS AND BELOW ROOFTOP MEP UNITS (<1500 POUNDS)
3/4" = 1'-0"



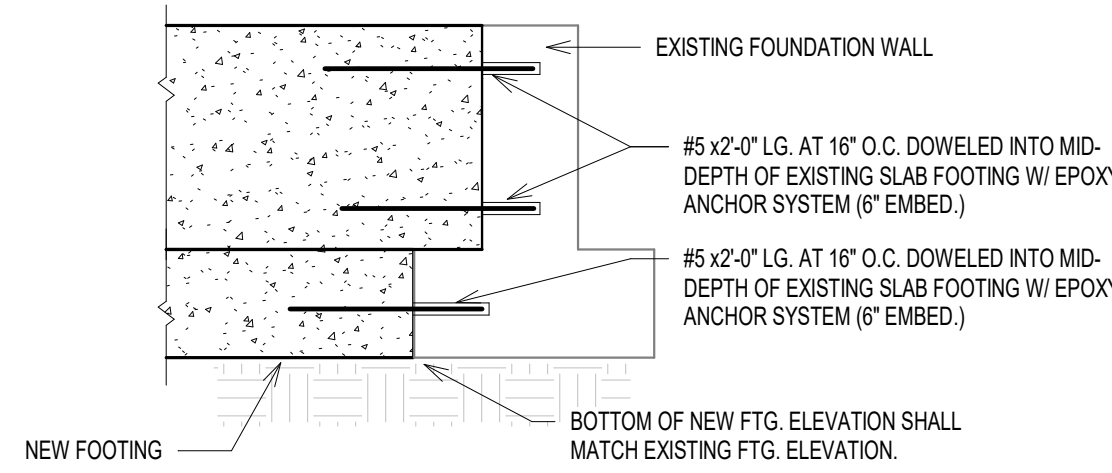
9 MECHANICAL UNIT SUPPORT DETAIL
3/4" = 1'-0"



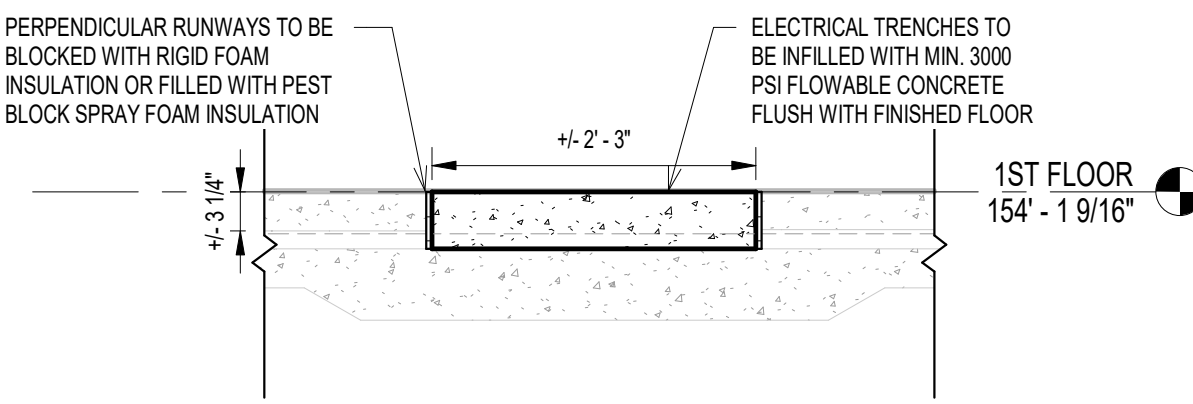
10 TYPICAL PARAPET SUPPORT DETAIL
3/4" = 1'-0"



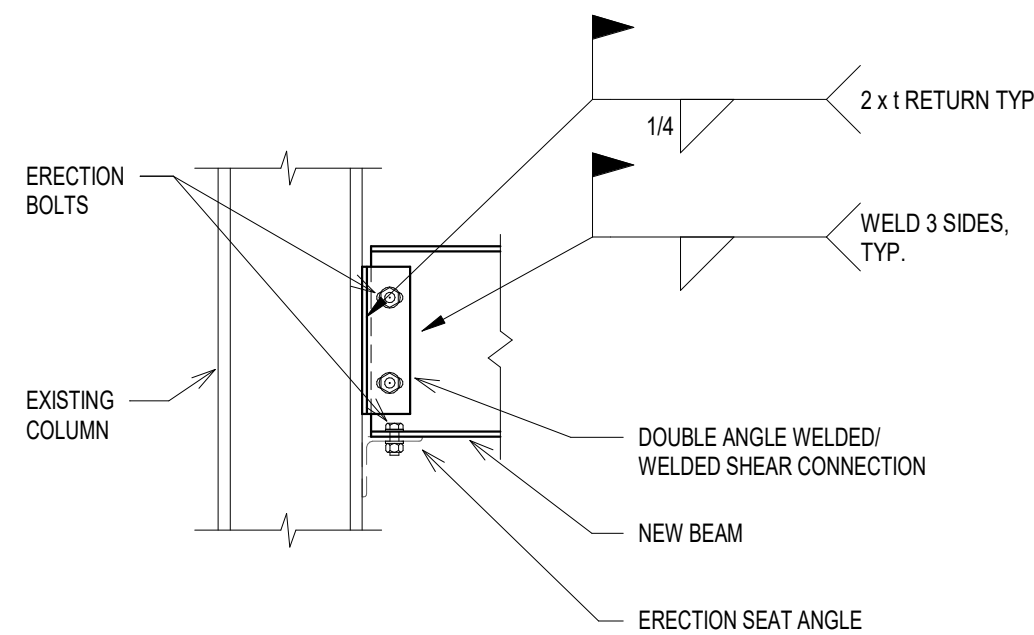
1 NEW SLAB-ON-GRADE CONNECTION TO EXISTING SLAB-ON-GRADE
1/2" = 1'-0"



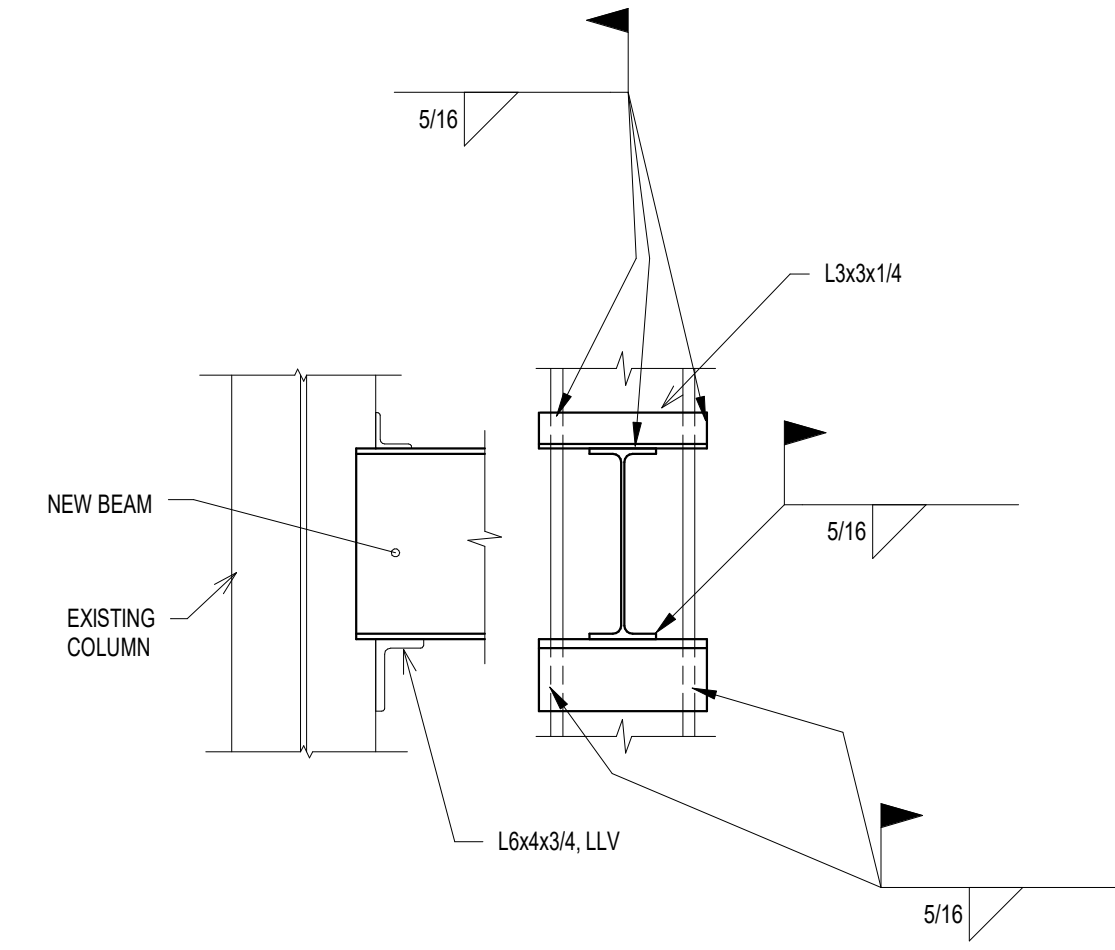
2 NEW FOUNDATION CONNECTION TO EXISTING FOUNDATION
1/2" = 1'-0"



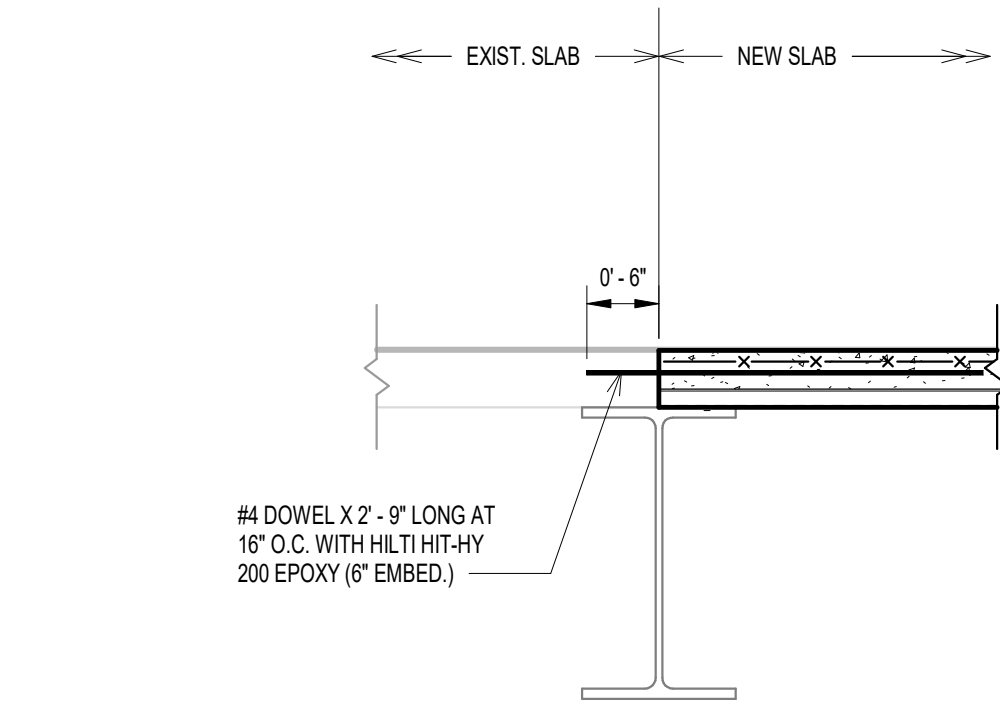
3 EXISTING ELECTRICAL TRENCH INFILL DETAIL
3/4" = 1'-0"



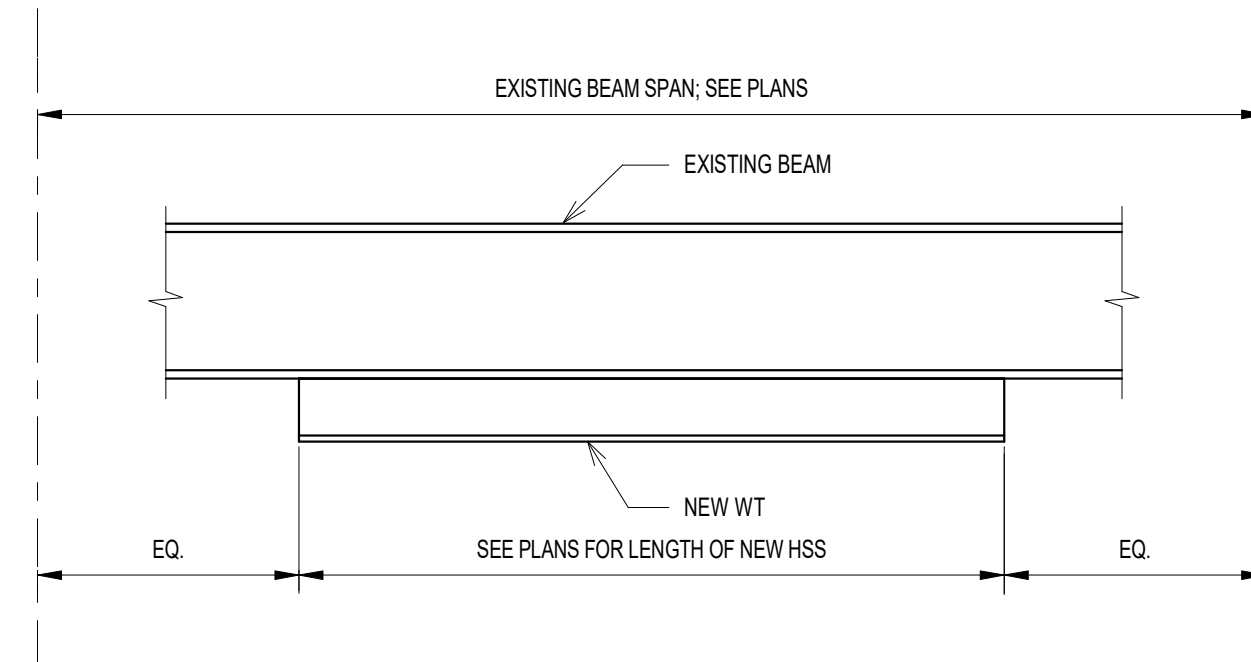
4 NEW BEAM TO EXISTING COLUMN DOUBLE ANGLE SHEAR CONNECTION
3/4" = 1'-0"



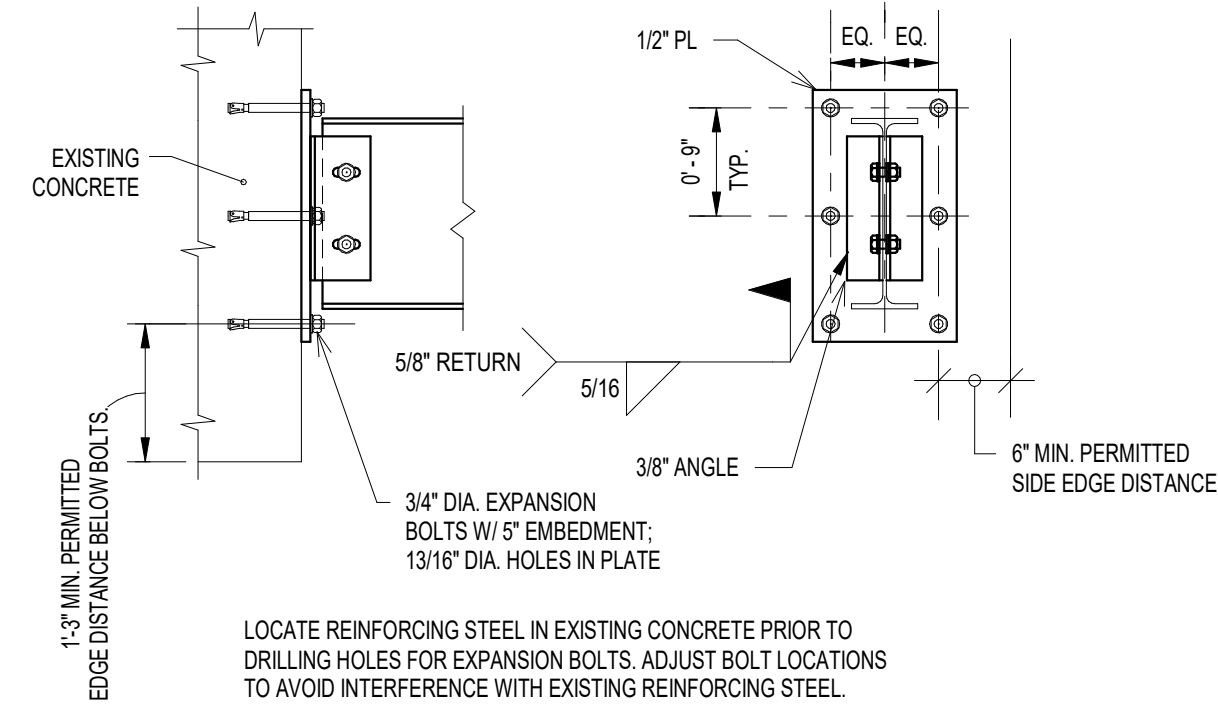
5 NEW BEAM CONNECTION TO EXISTING COLUMN PERPENDICULAR TO WEB
3/4" = 1'-0"



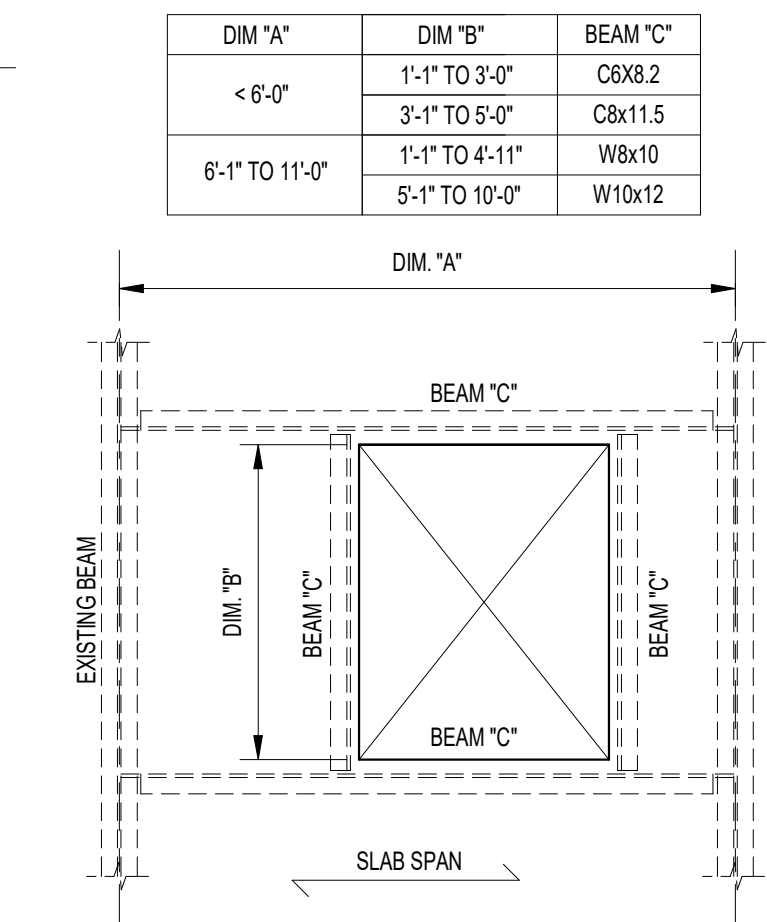
6 NEW SLAB-ON-METAL DECK CONNECT. TO EXIST. SLAB-ON-METAL DECK
3/4" = 1'-0"



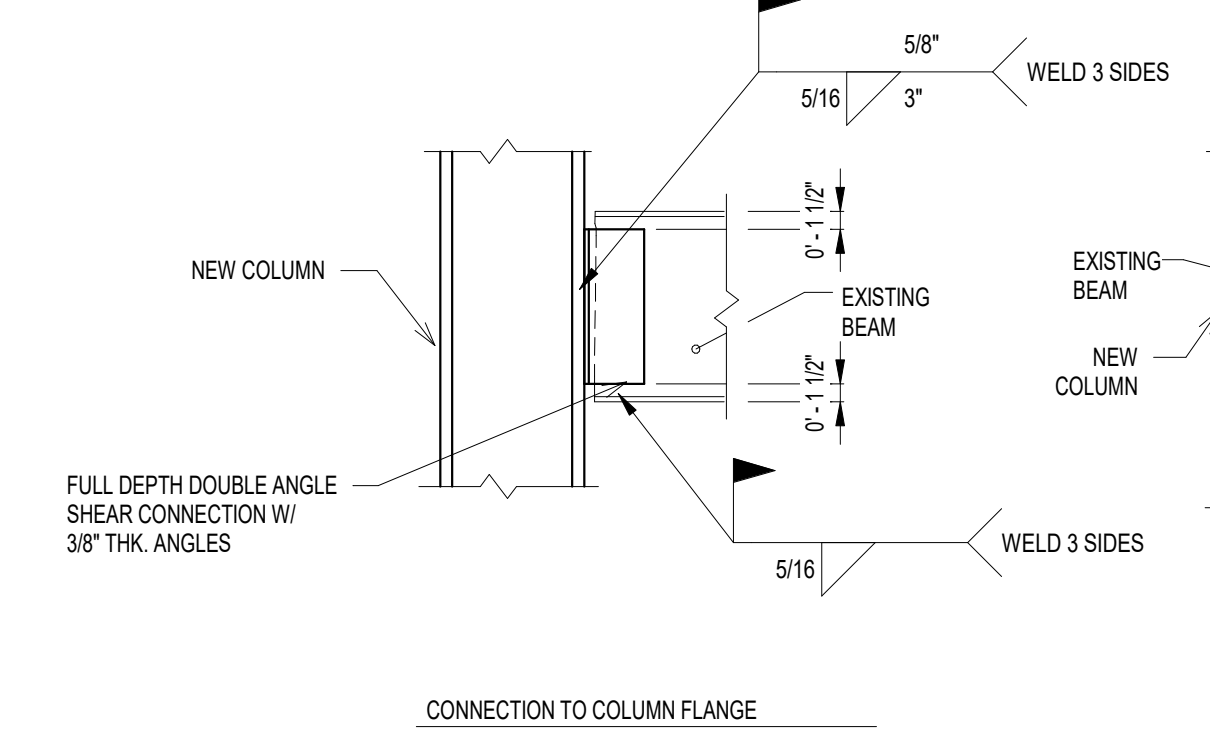
7 NEW BEAM CONNECTION TO EXISTING CONCRETE
3/4" = 1'-0"



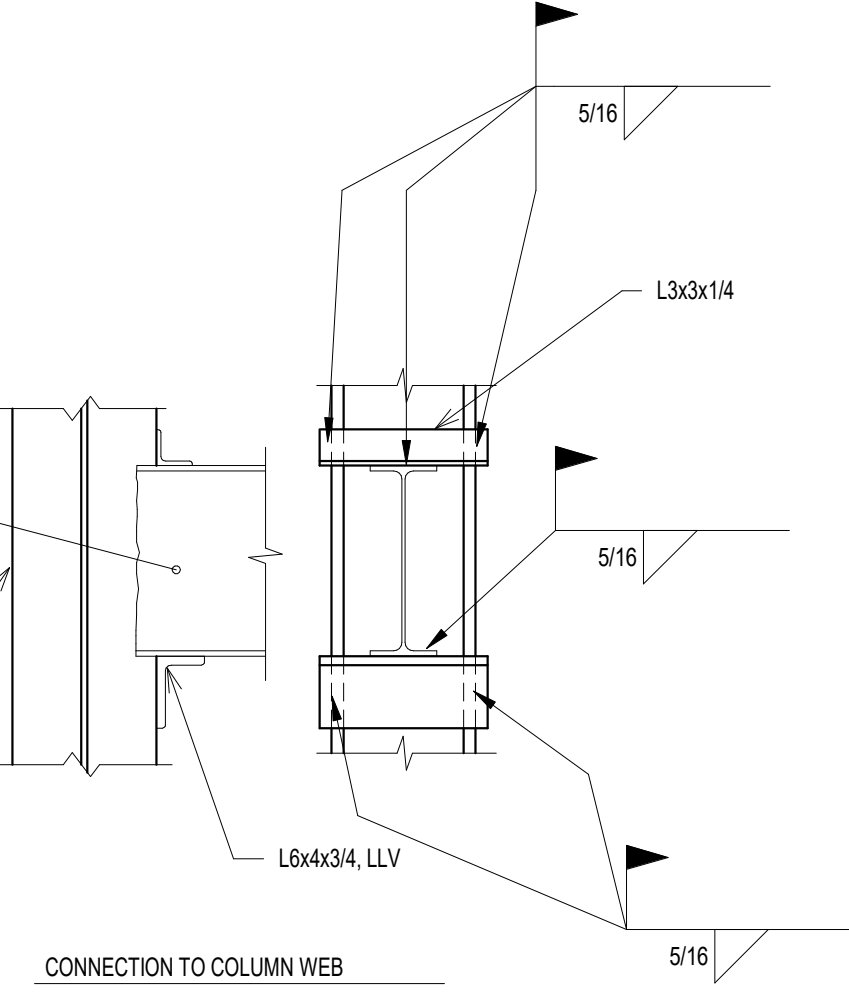
8 EXISTING BEAM REINFORCEMENT
3/4" = 1'-0"



9 EXISTING BEAM CONNECTION TO NEW SUPPORTING BEAM
3/4" = 1'-0"

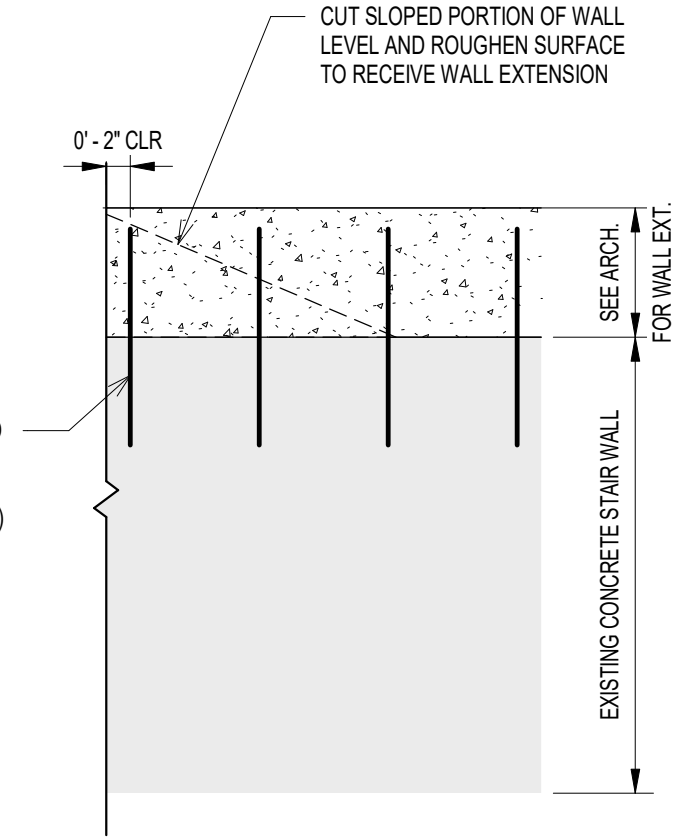


10 EXISTING BEAM CONNECTION TO NEW COLUMN
3/4" = 1'-0"

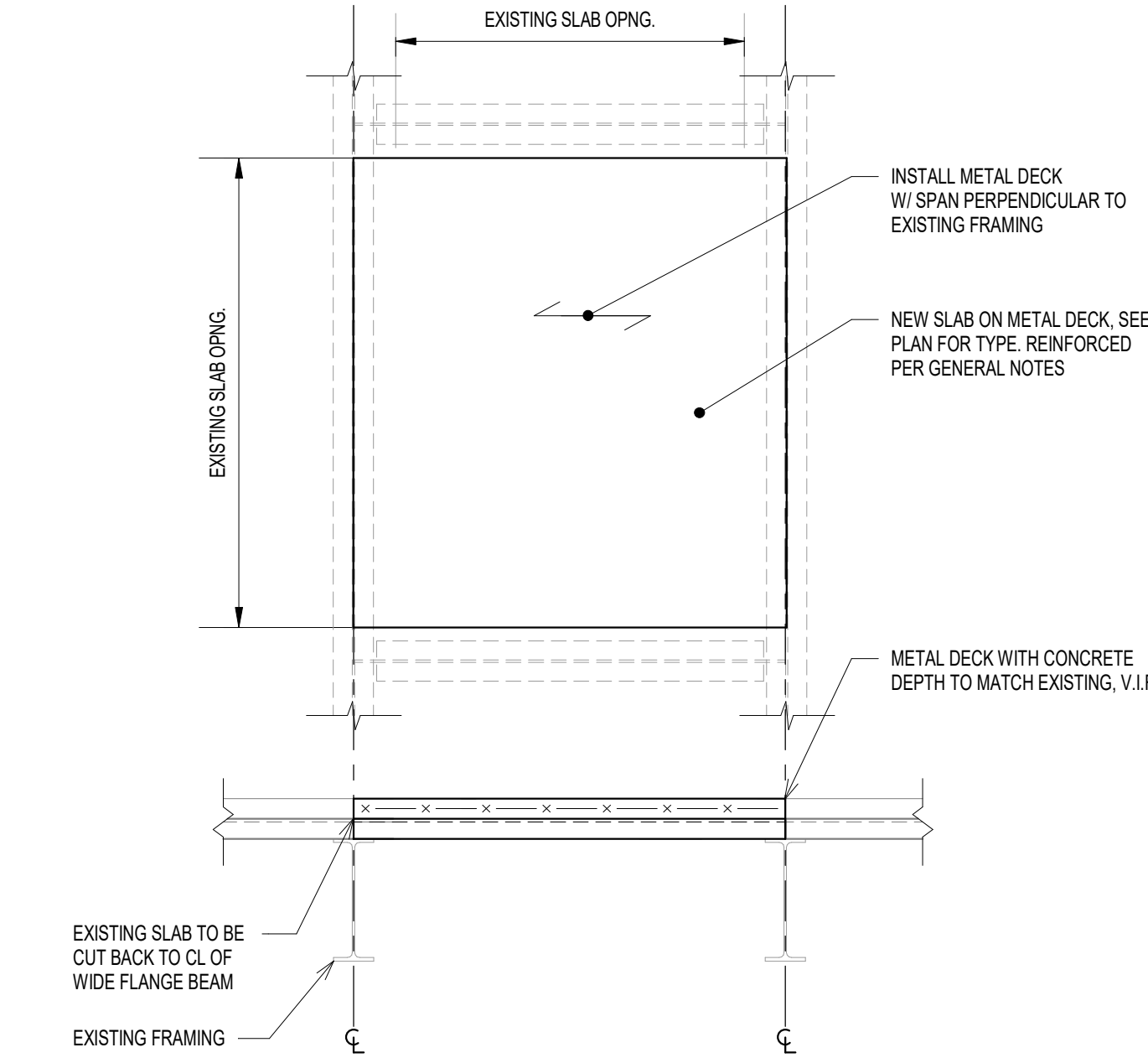


11 NEW OPENINGS THROUGH EXISTING CONCRETE WALL
3/4" = 1'-0"

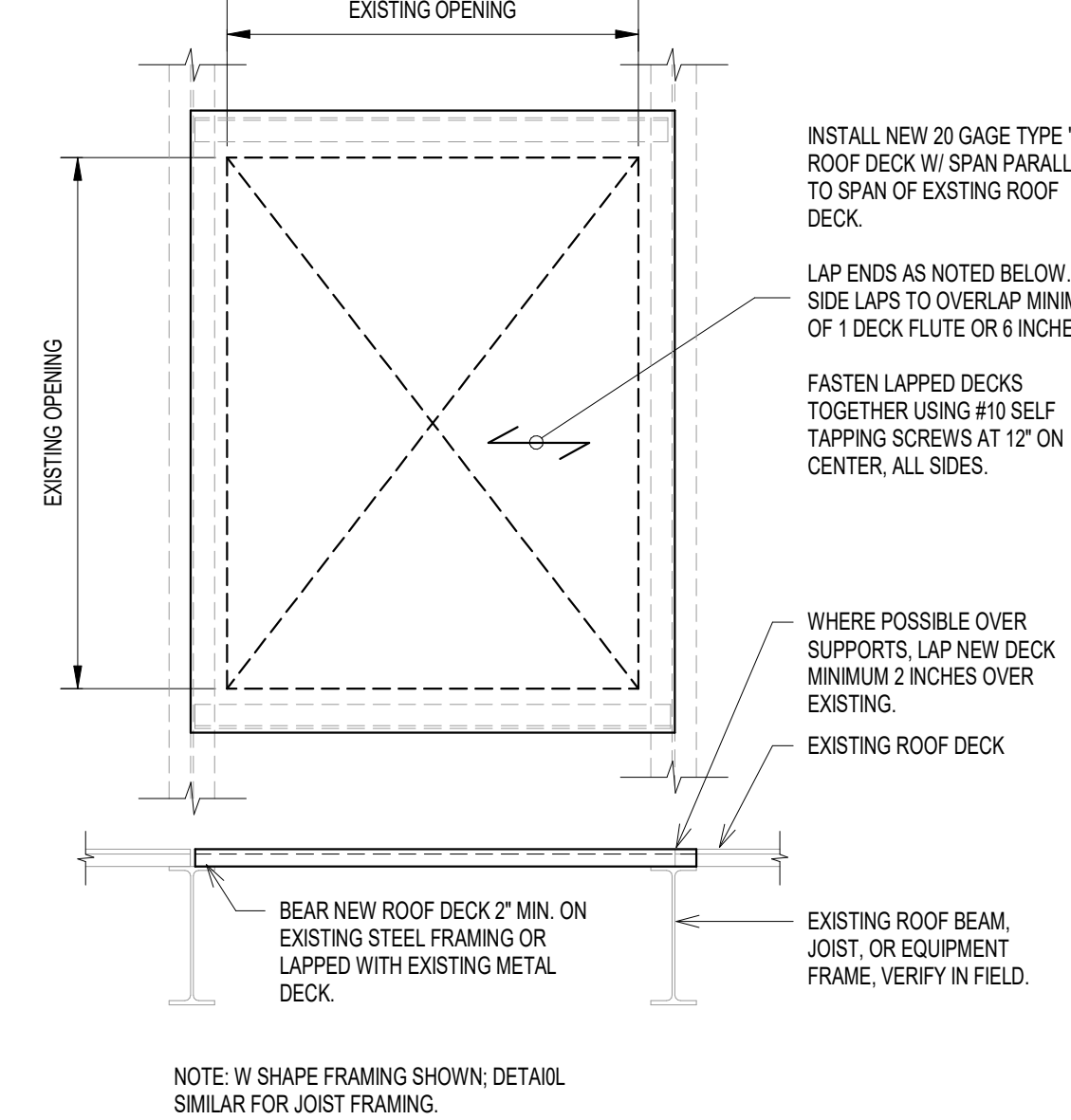
12 CONCRETE WALL EXTENSION
3/4" = 1'-0"



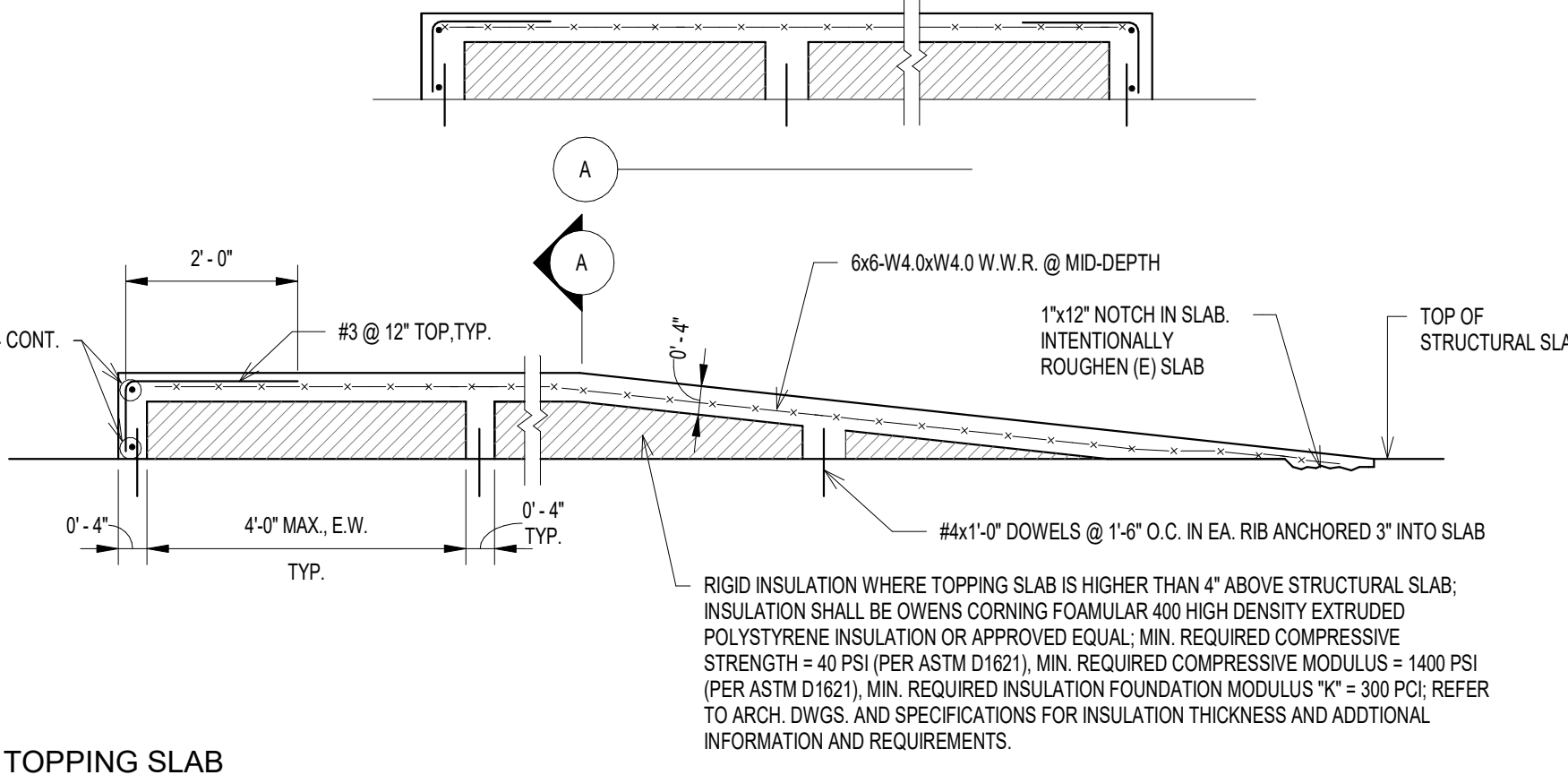
13 NEW OPENING IN EXISTING SLAB
3/4" = 1'-0"



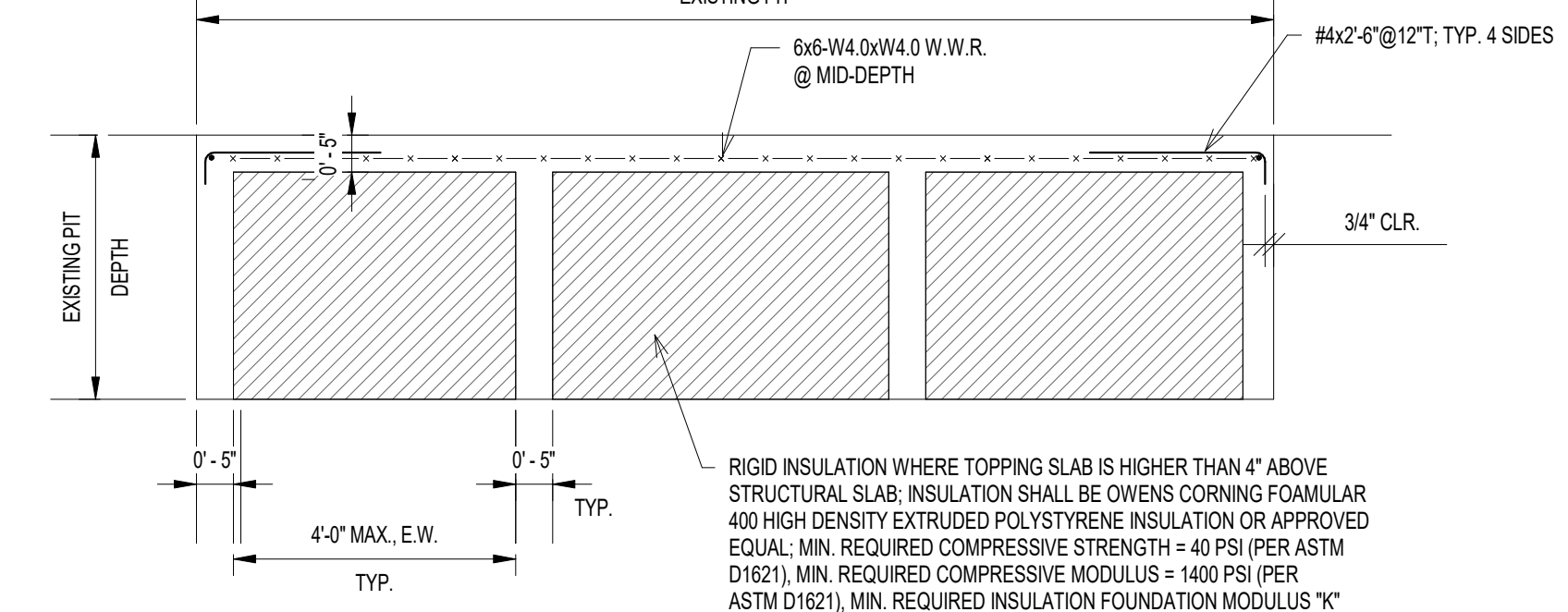
14 NEW SLAB INFILL AT OPENING IN EXISTING SLAB
3/4" = 1'-0"



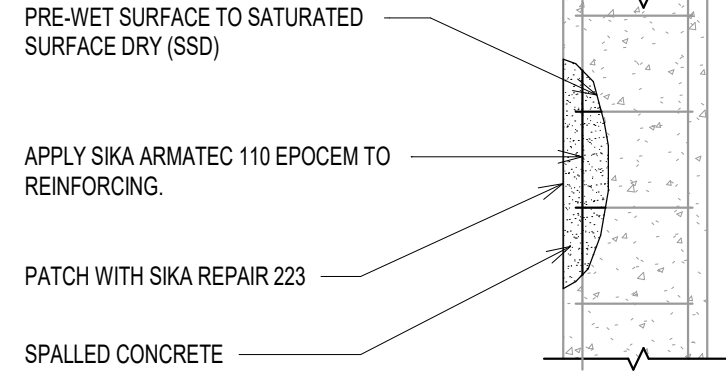
15 NEW ROOF DECK OVER OPENING IN EXISTING ROOF
3/4" = 1'-0"



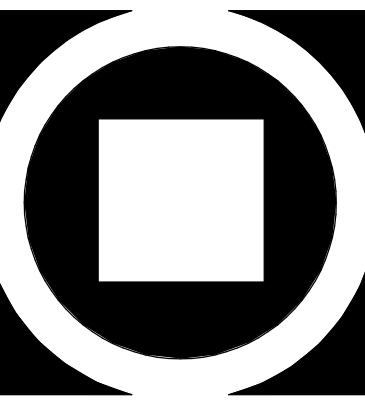
16 TOPPING SLAB
1/2" = 1'-0"



17 EXISTING PIT INFILL
1/2" = 1'-0"



18 CONCRETE REPAIR
3/4" = 1'-0"



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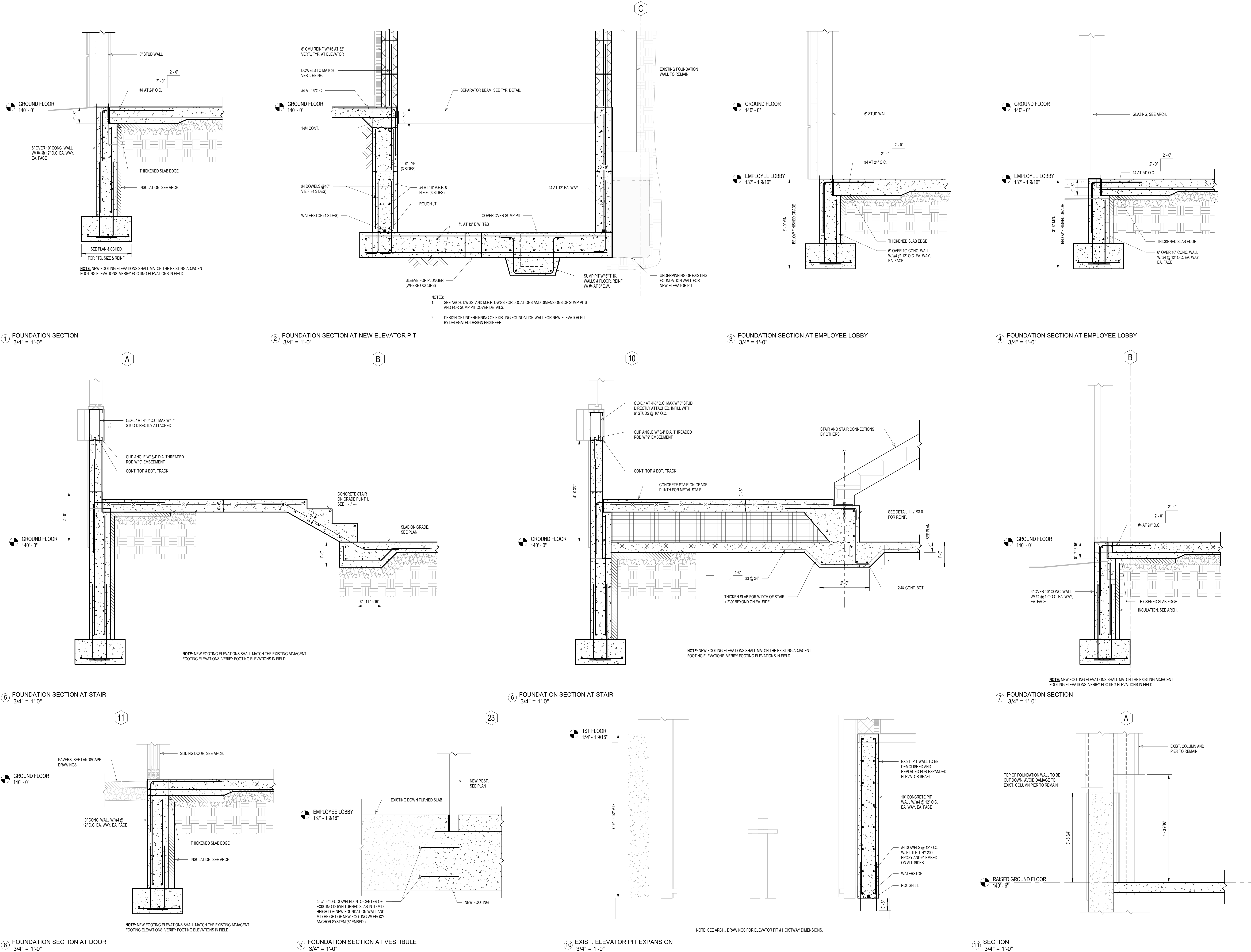


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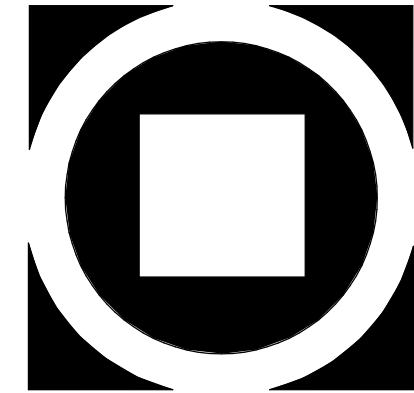
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ISSUE DATE: 02/21/25
DRAWN BY: EMS
CHECKED BY: RPS
SCALE: AS NOTED

TYPICAL FRAMING DETAILS	
NO.	DESCRIPTION
1	DESIGN DEVELOPMENT
2	75% GMP
3	BID & PERMIT
4	
5	
6	
7	
8	
9	
10	

S3.3



10/15/2025 3:48:37 PM



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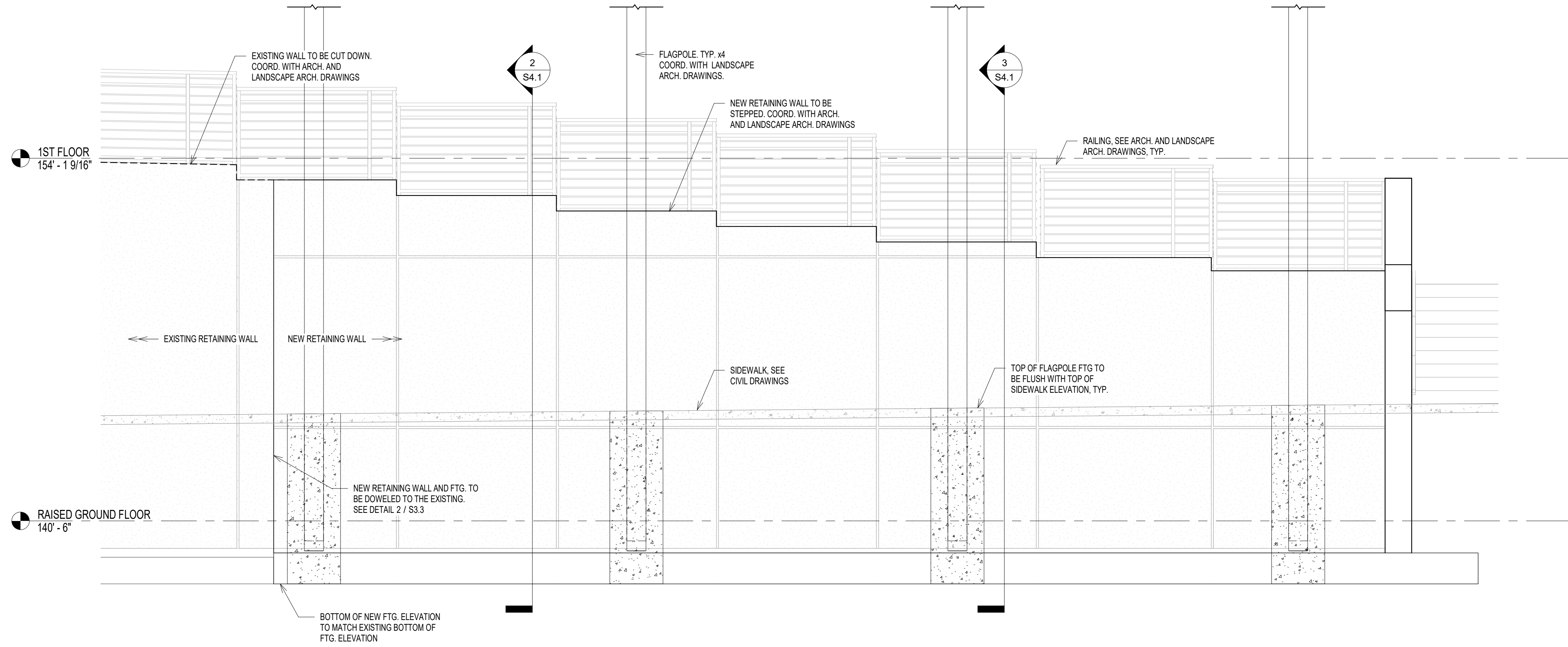


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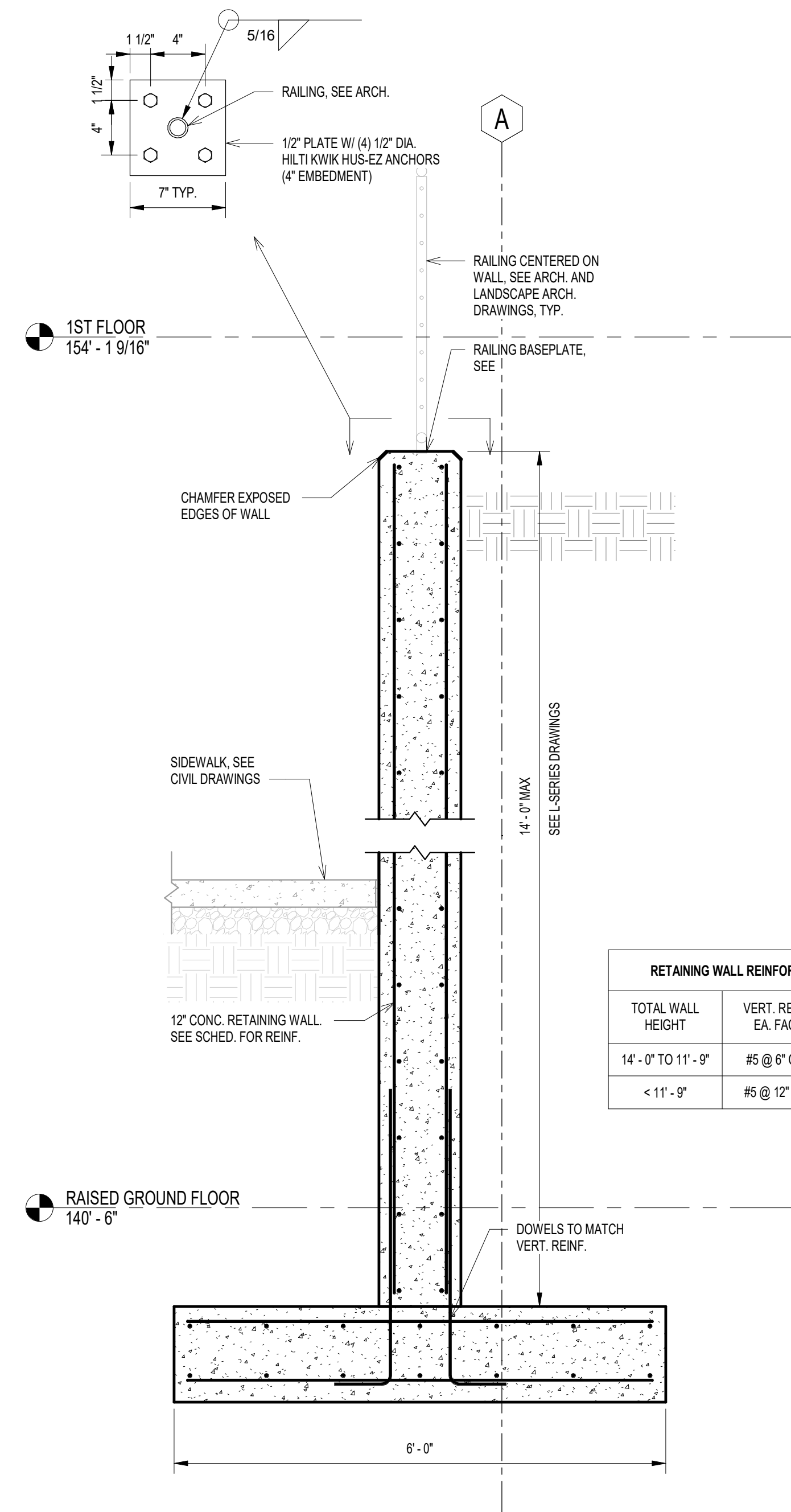
FOUNDATION SECTIONS		DATE
NO.	DESCRIPTION	
75% GMP	BID & PERMIT	08/22/25
		10/03/25

S4.0



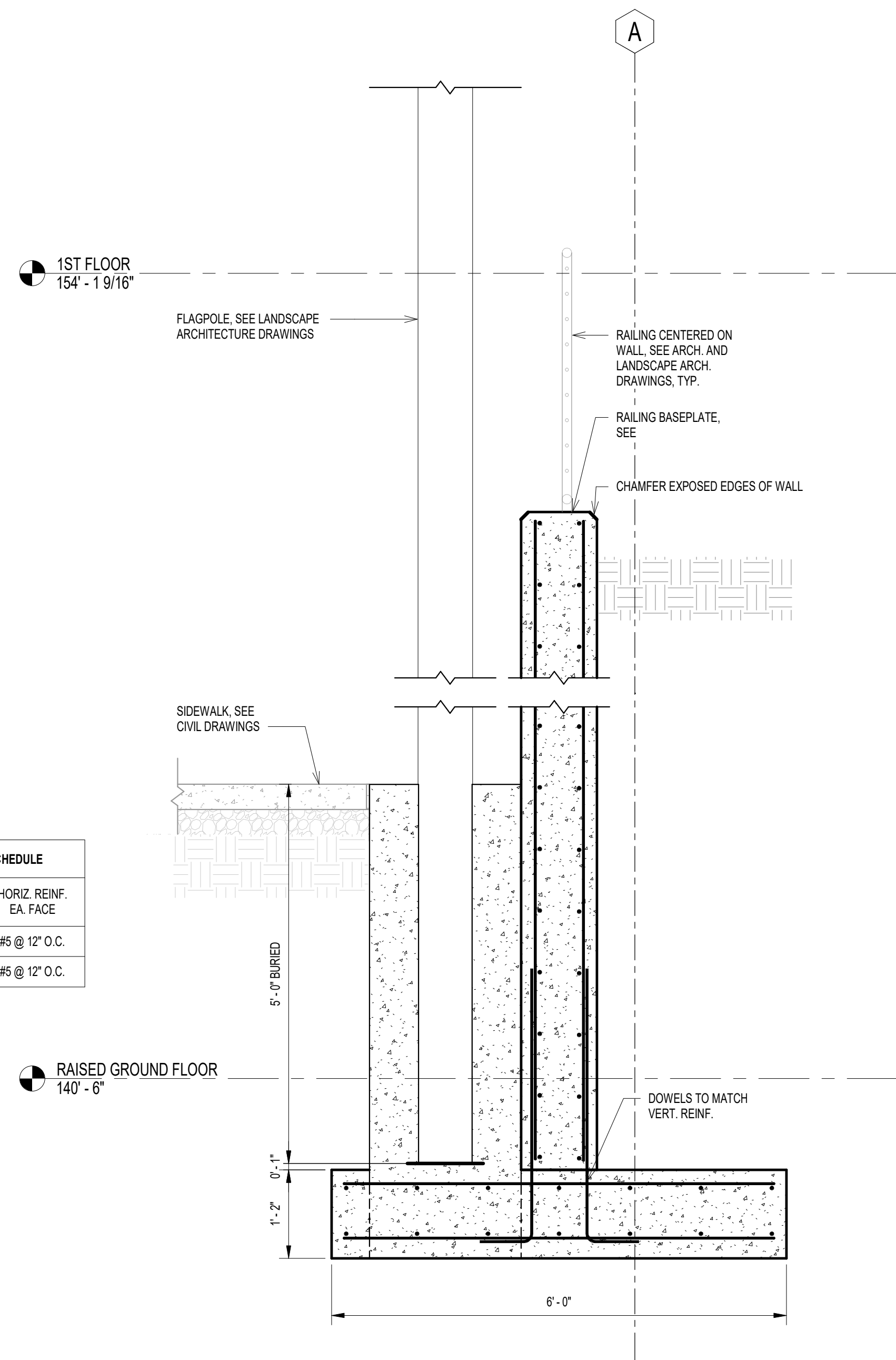
NOTE: SEE ARCH. FOR WALL FINISHES

1 WALL ELEVATION
3/8" = 1'-0"

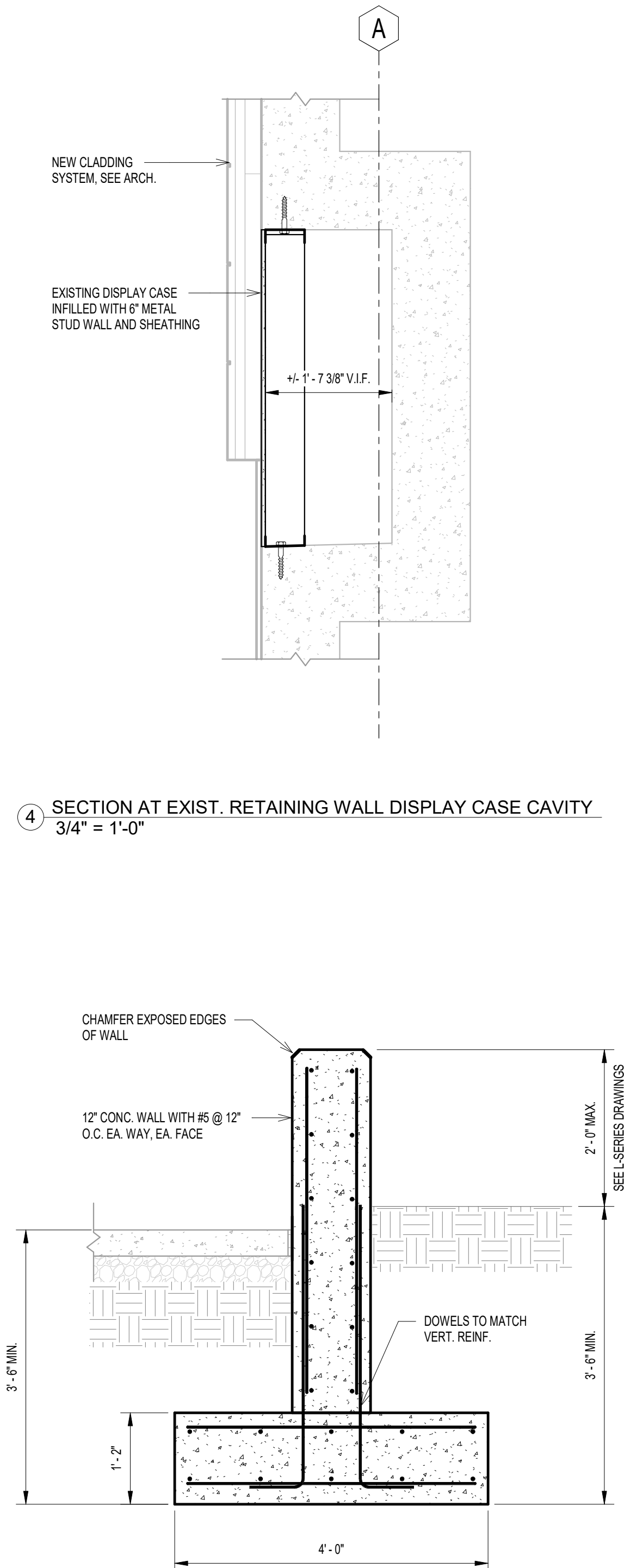


RETAINING WALL REINFORCEMENT SCHEDULE		
TOTAL WALL HEIGHT	VERT. REINF. EA. FACE	HORIZ. REINF. EA. FACE
14'-0" TO 11'-9"	#5 @ 8' O.C.	#5 @ 12' O.C.
< 11'-9"	#5 @ 12' O.C.	#5 @ 12' O.C.

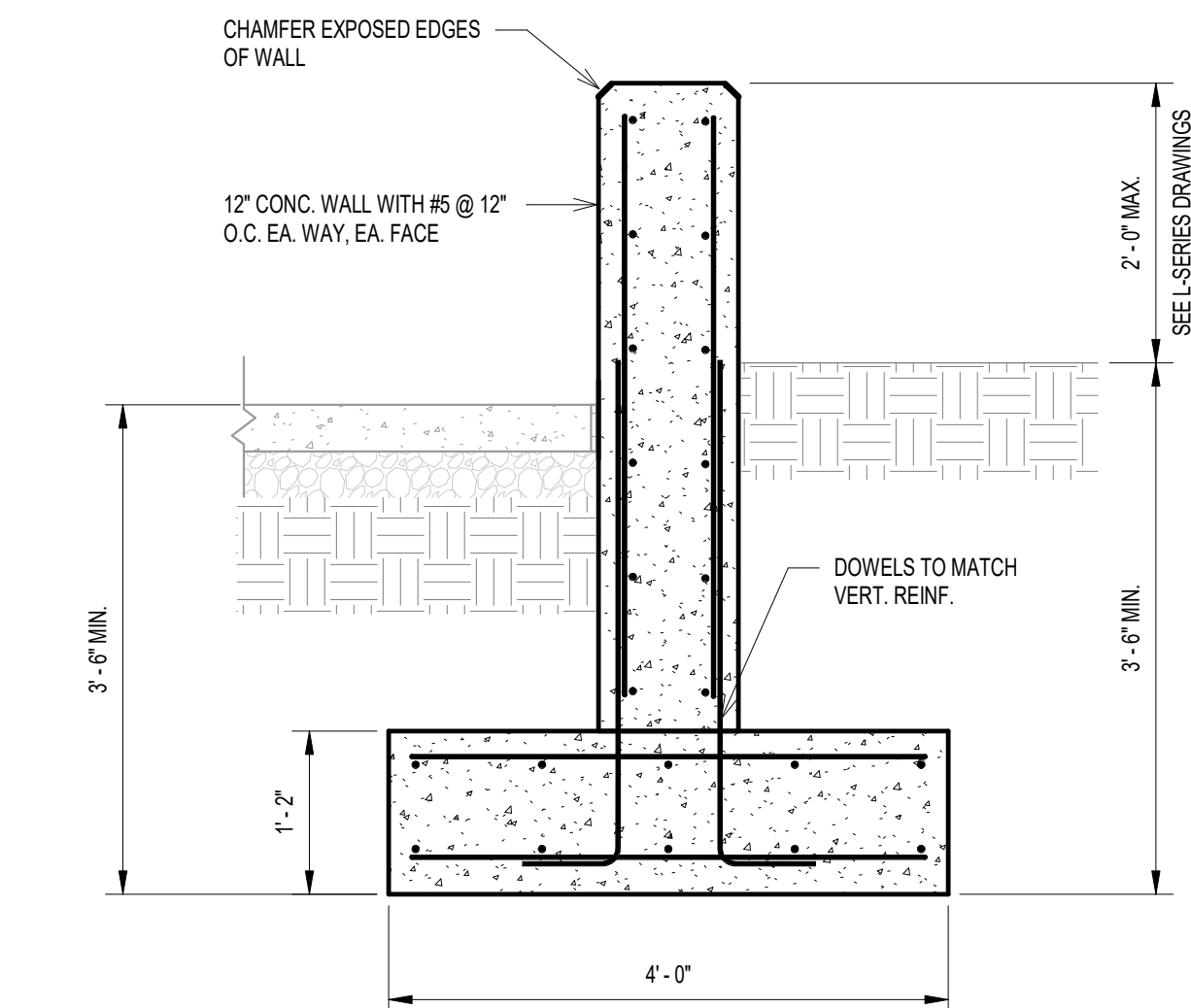
2 SITE RETAINING WALL SECTION
3/4" = 1'-0"



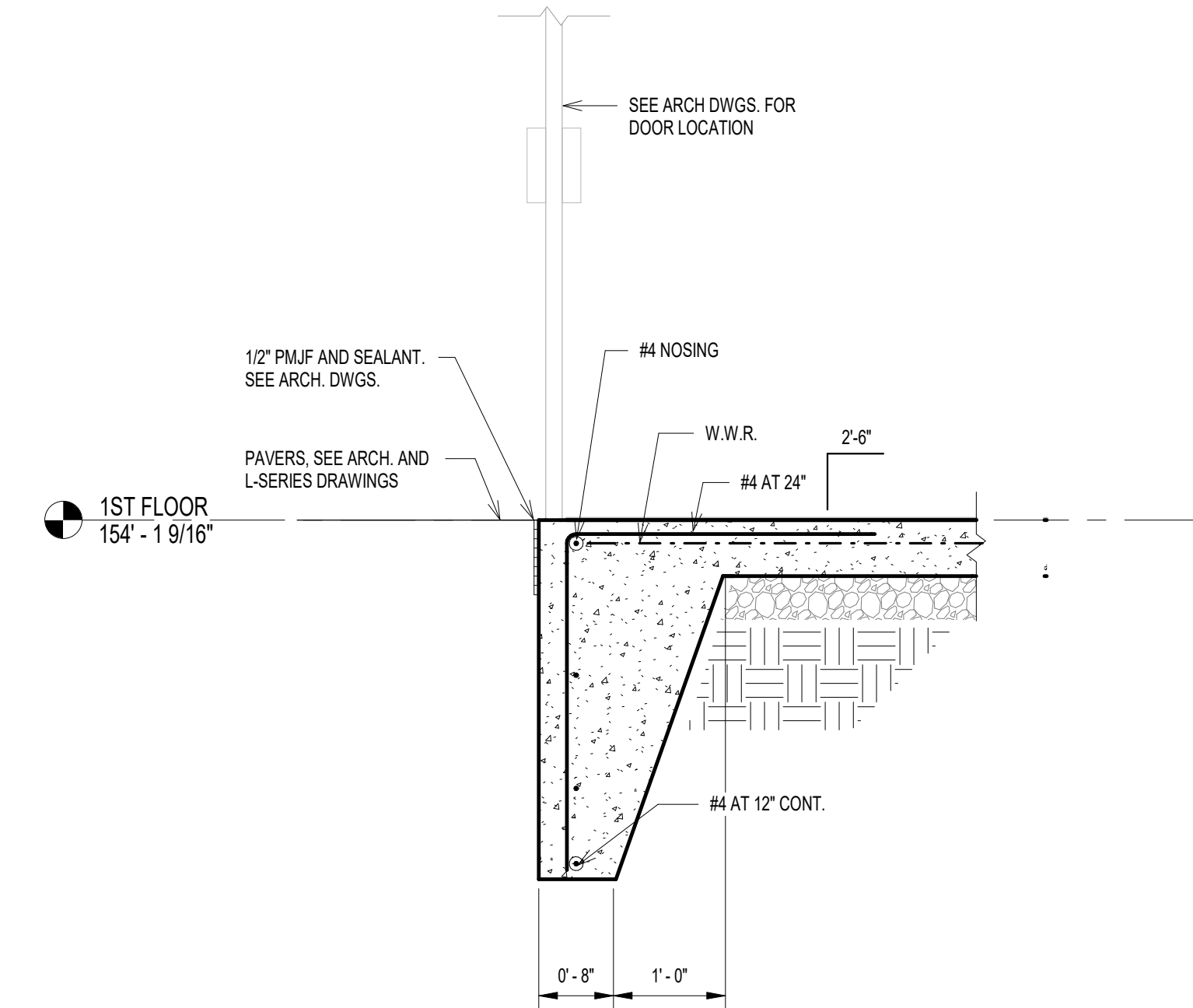
3 SITE RETAINING WALL SECTION
3/4" = 1'-0"



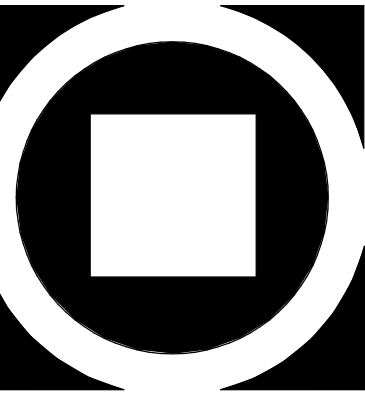
4 SECTION AT EXIST. RETAINING WALL DISPLAY CASE CAVITY
3/4" = 1'-0"



5 PLANTER WALL SECTION
3/4" = 1'-0"



6 SECTION AT DOOR
3/4" = 1'-0"



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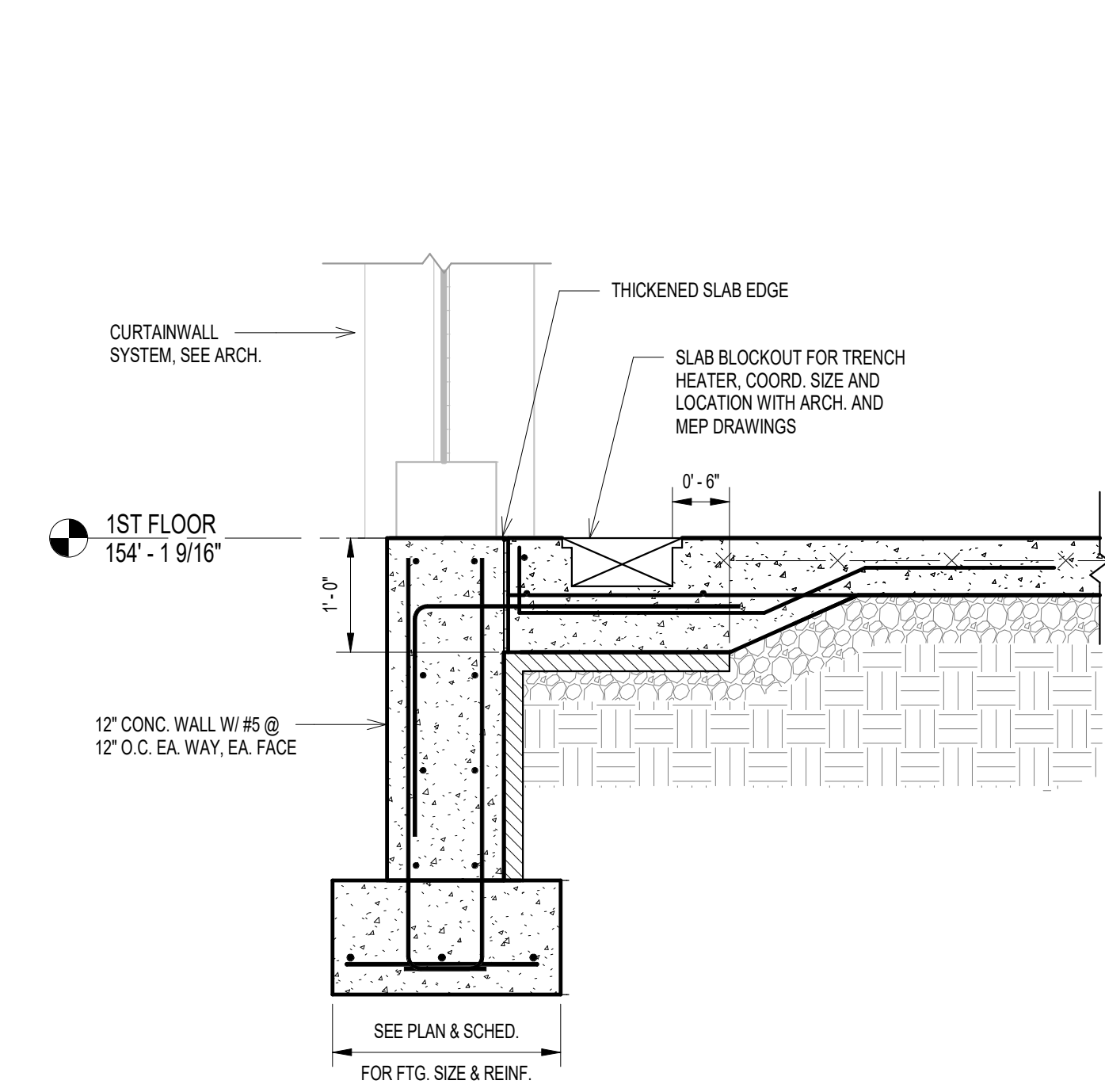


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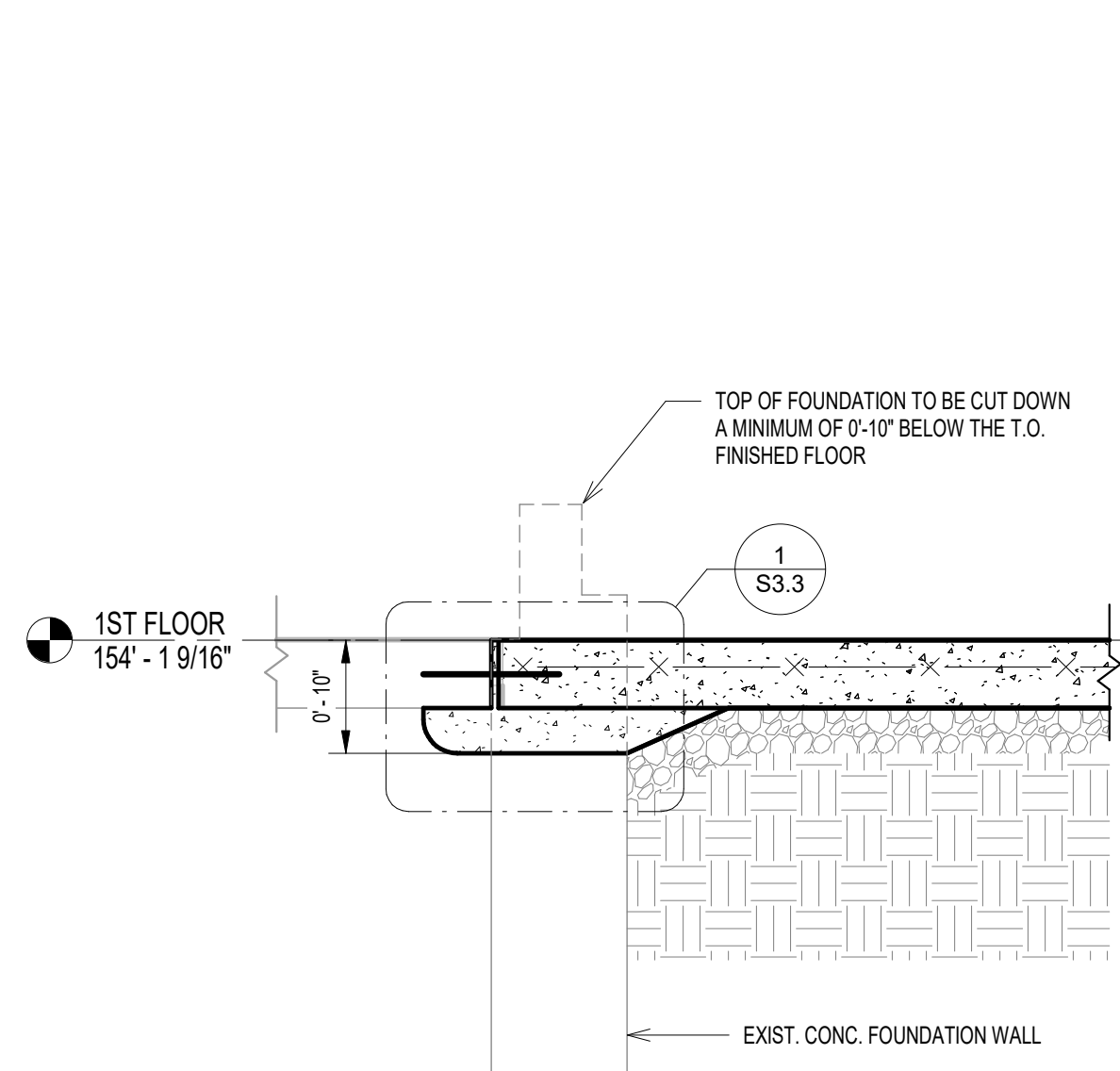
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SECTIONS	NO.	DESCRIPTION	DATE
	75% GMP	BID & PERMIT	08/22/25
			10/03/25

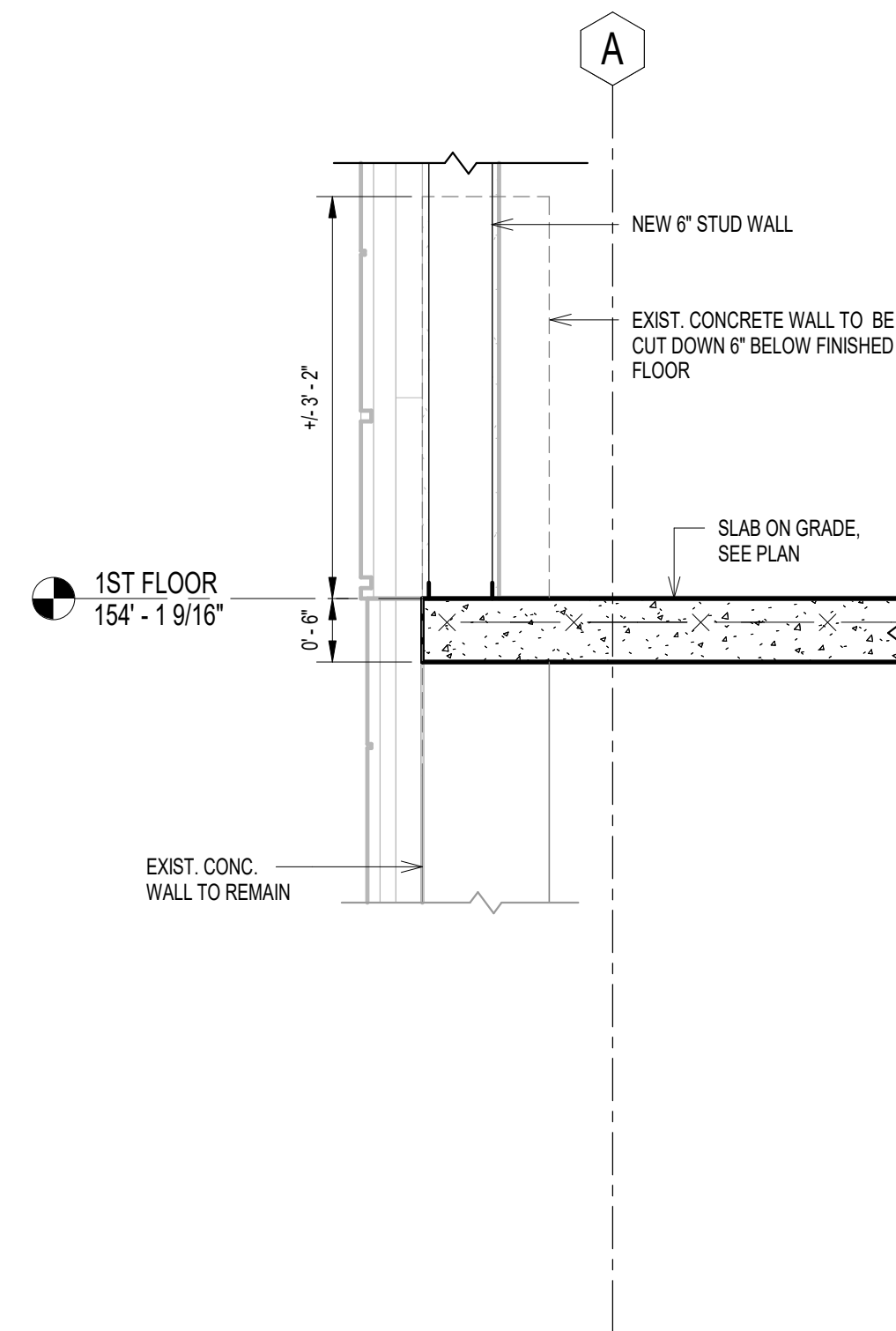
S4.1



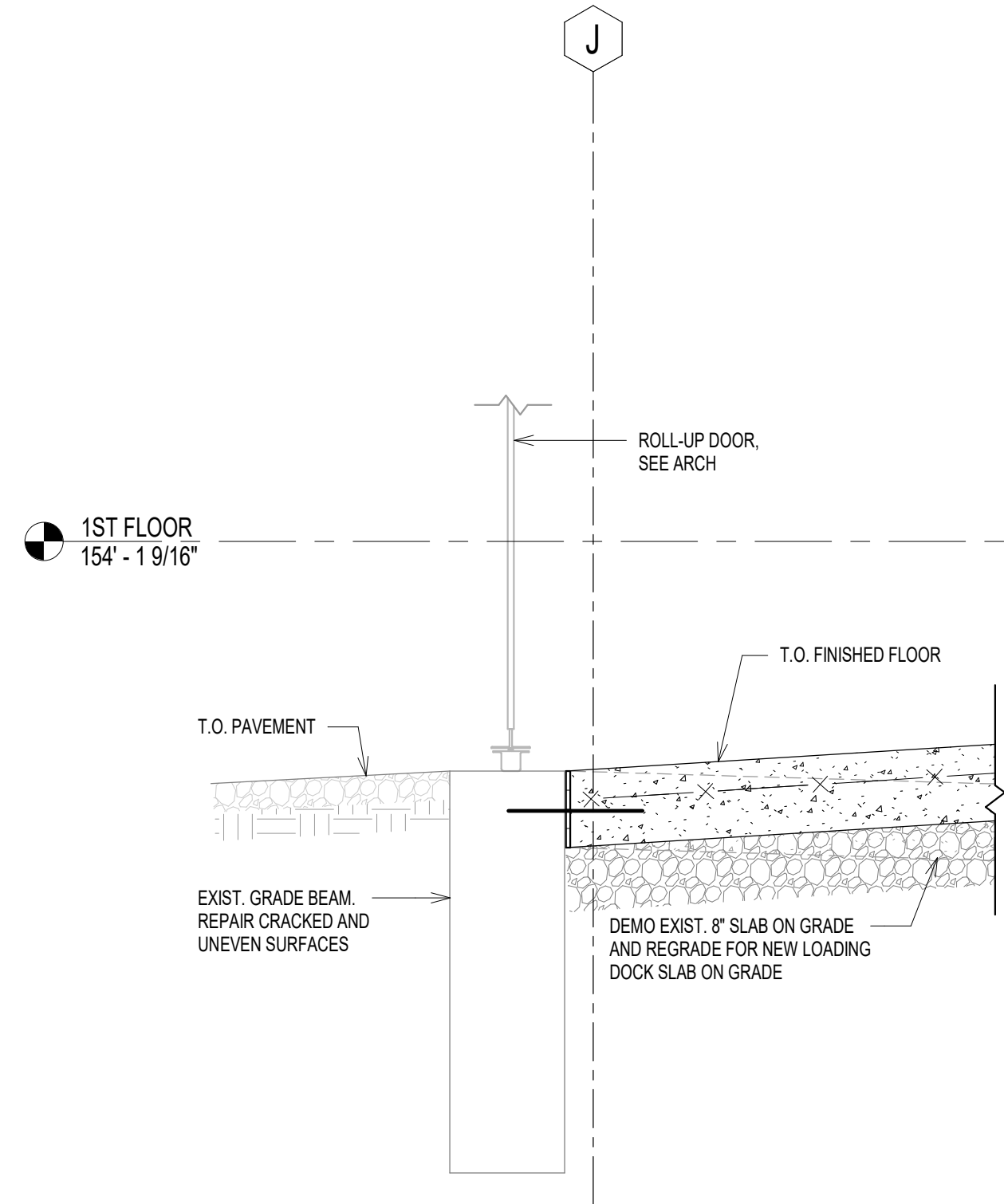
1 FOUNDATION SECTION AT CURTAINWALL
3/4" = 1'-0"



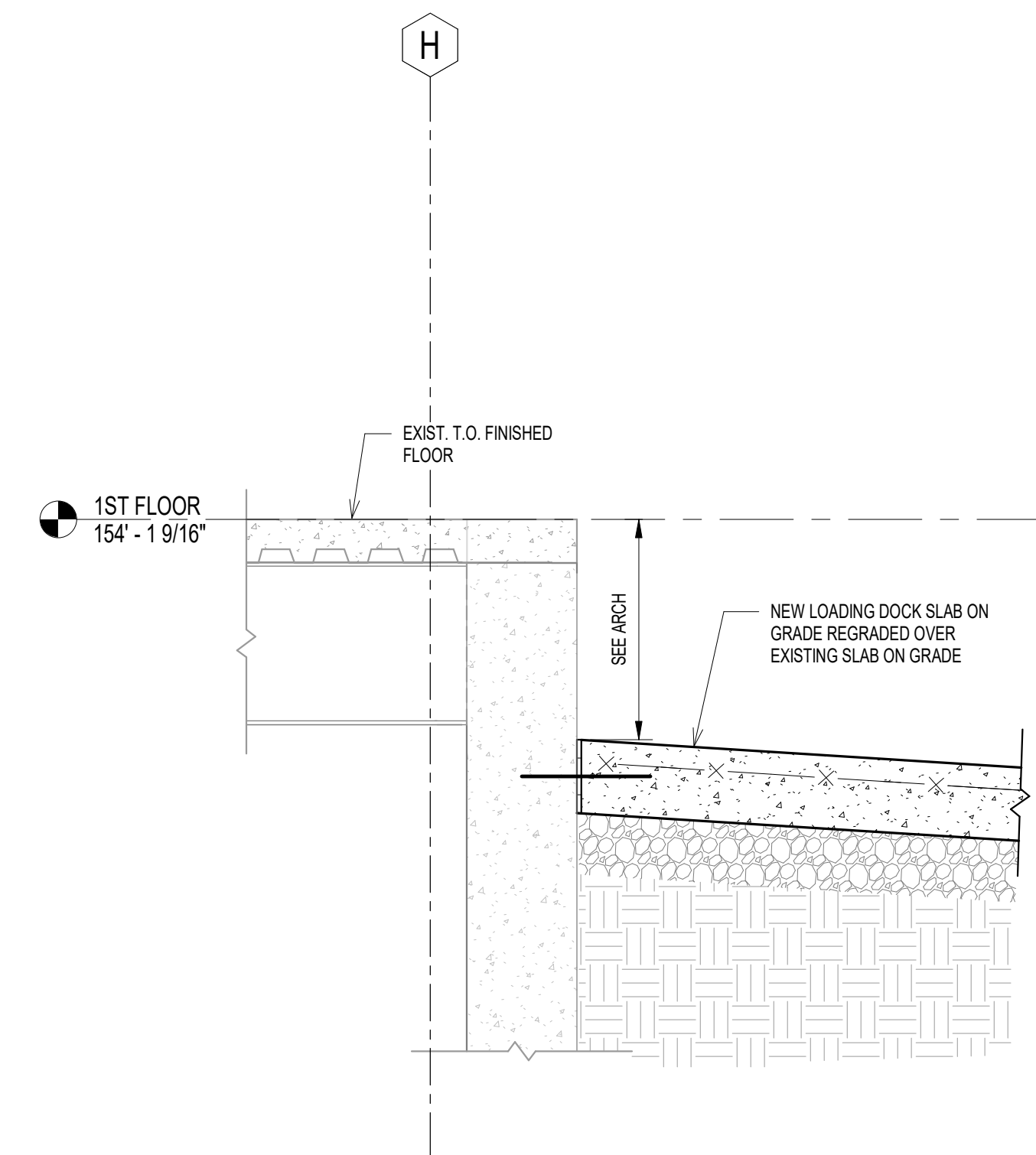
2 SECTION
3/4" = 1'-0"



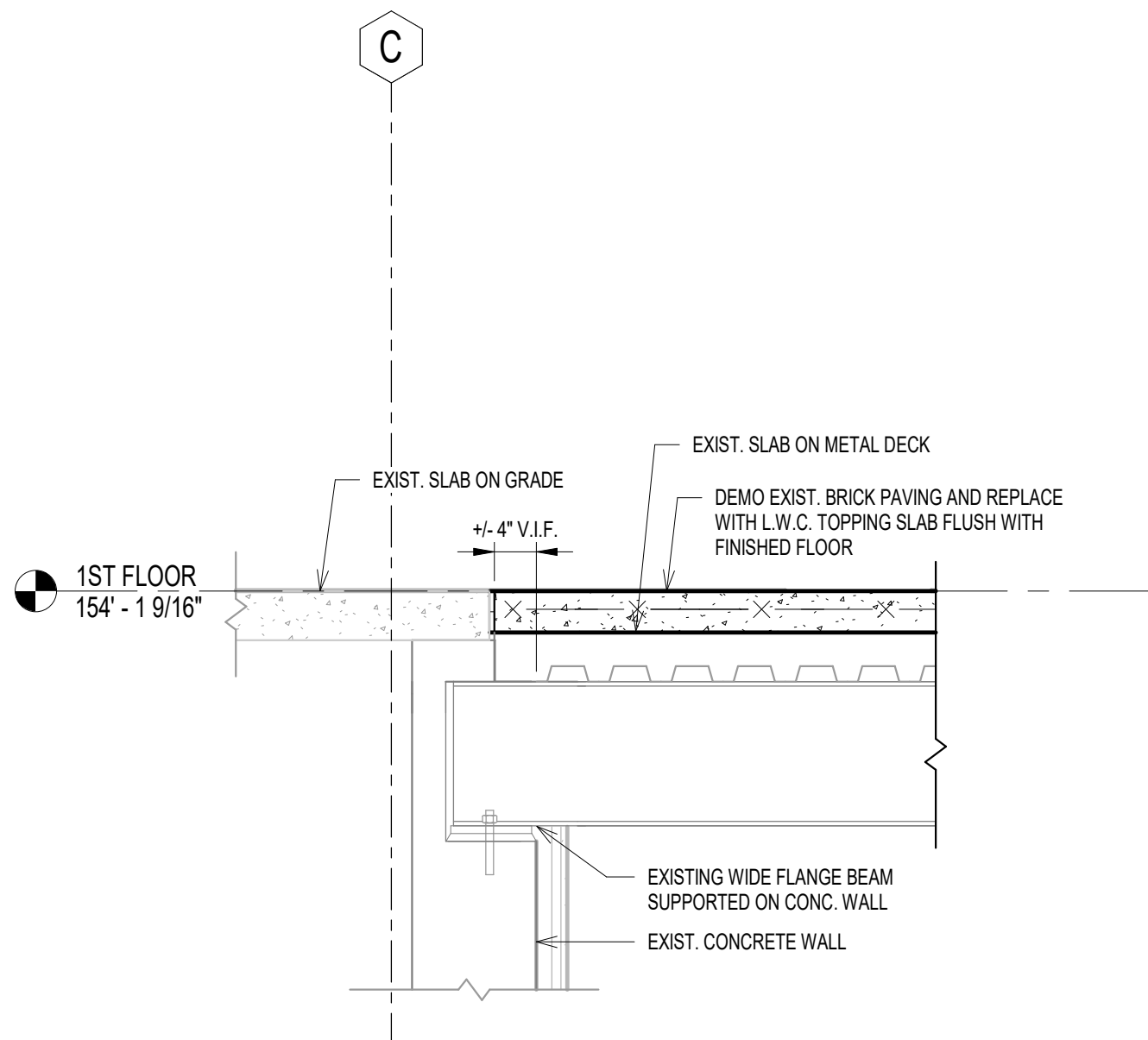
3 SECTION
3/4" = 1'-0"



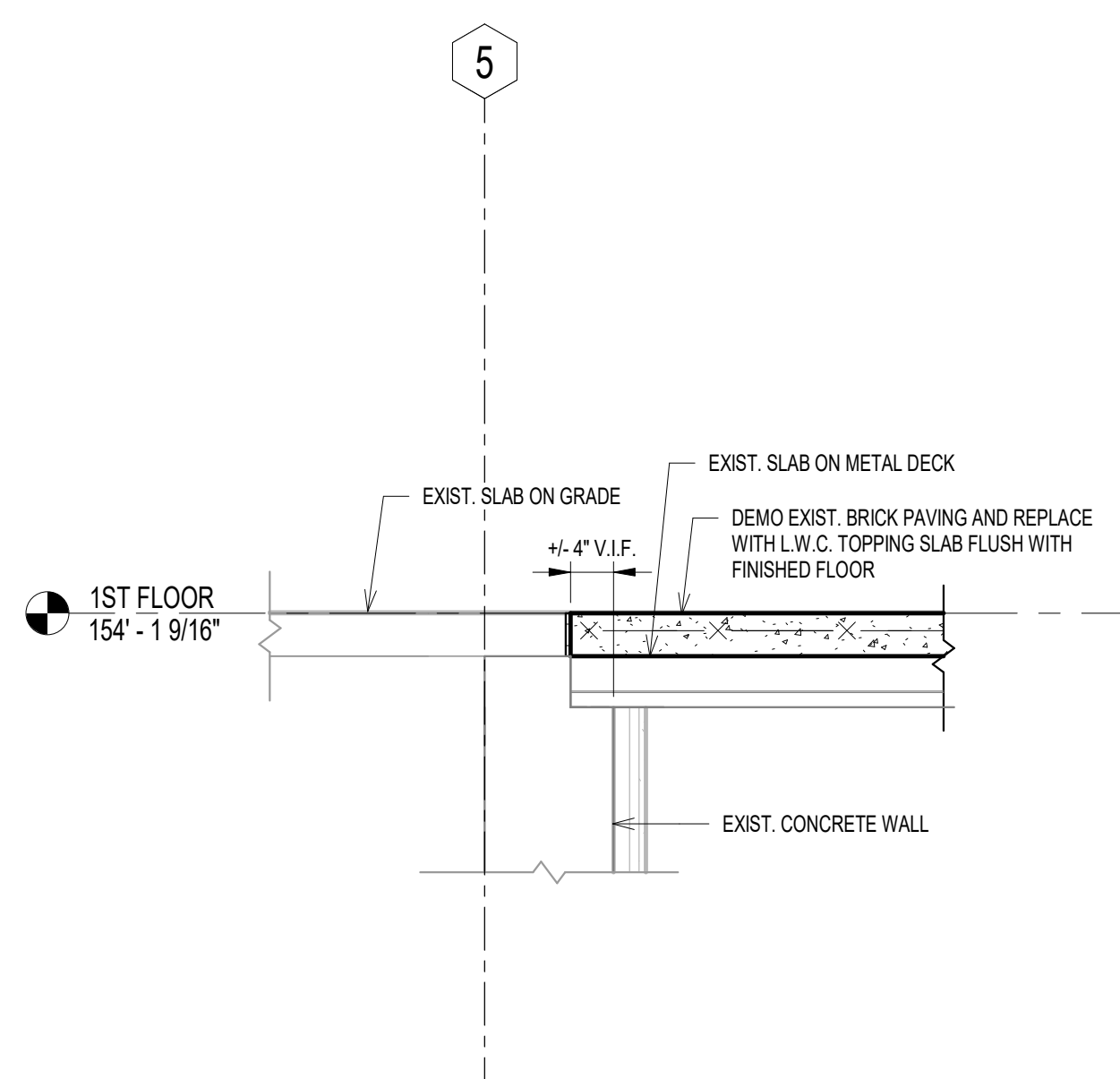
4 SECTION AT LOADING DOCK
3/4" = 1'-0"



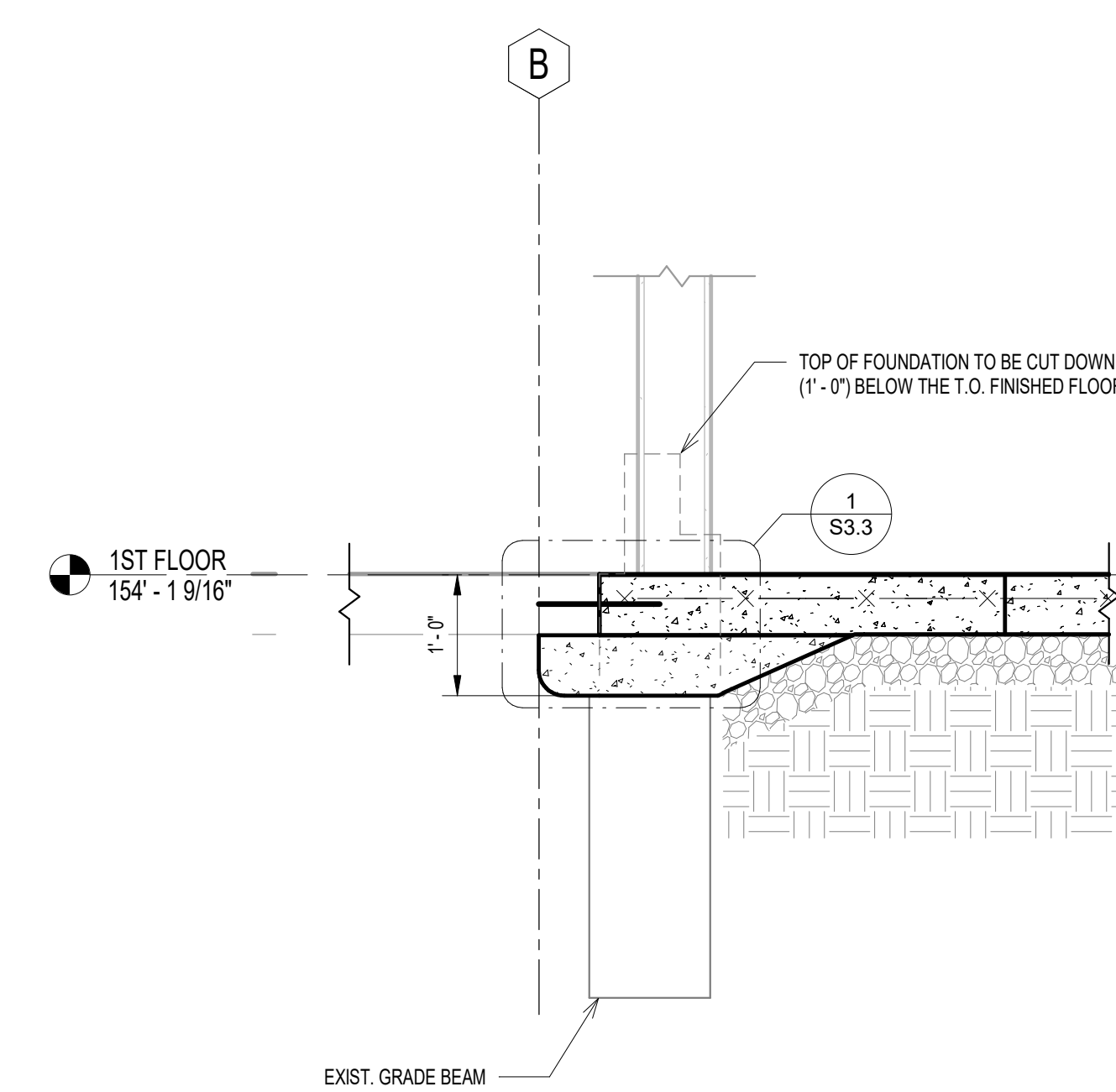
5 SECTION AT LOADING DOCK
3/4" = 1'-0"



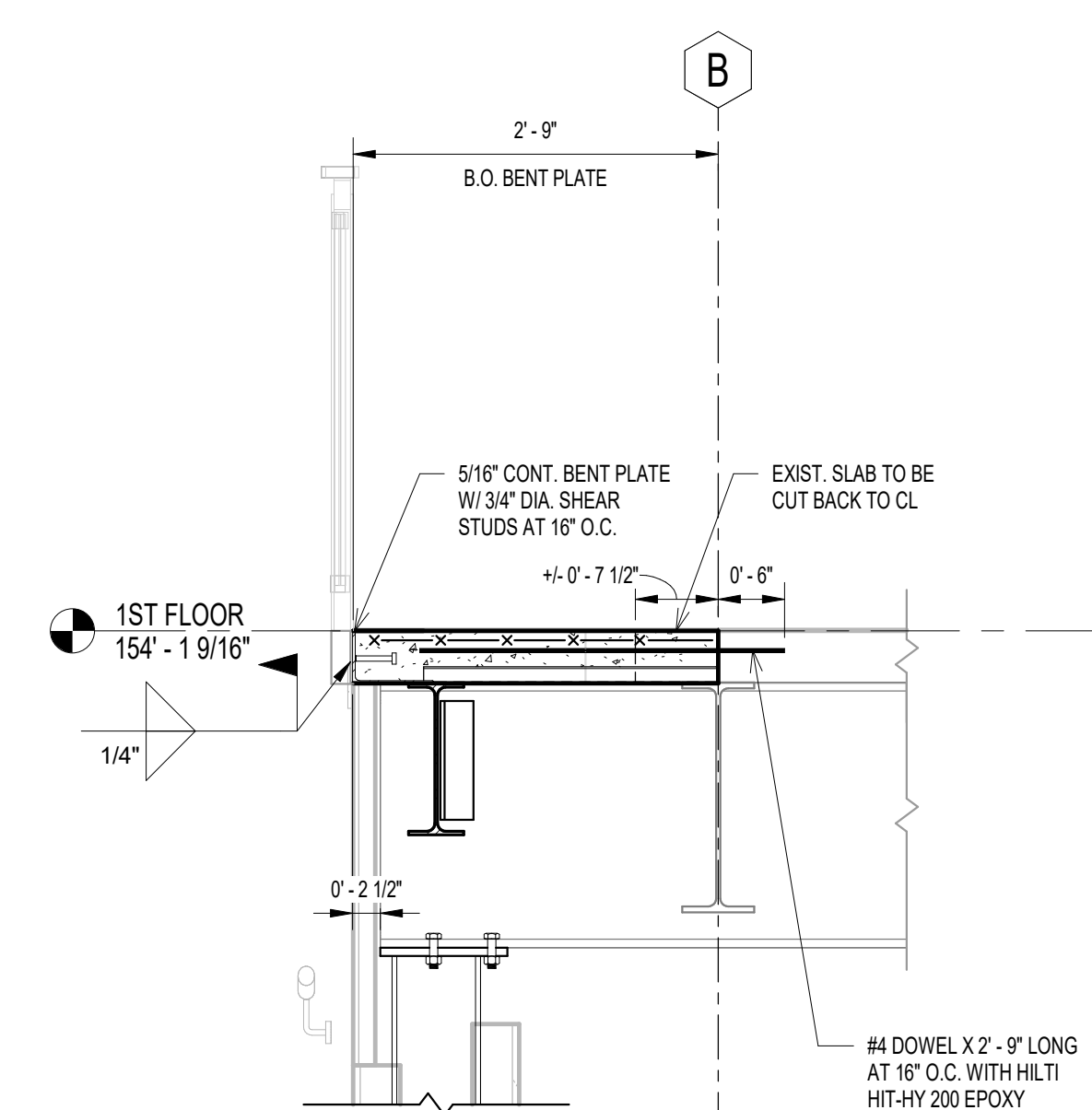
6 SECTION
3/4" = 1'-0"



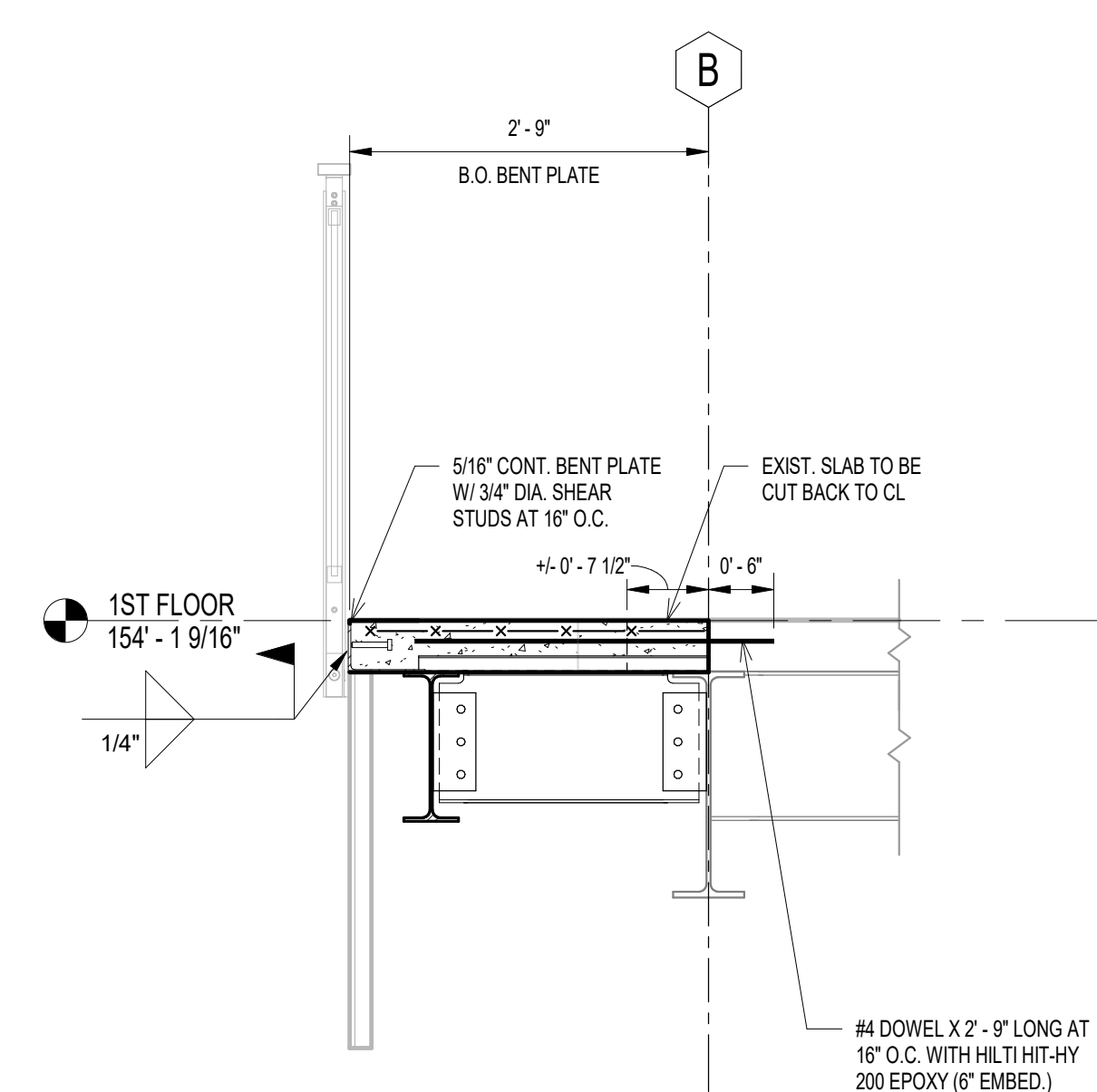
7 SECTION
3/4" = 1'-0"



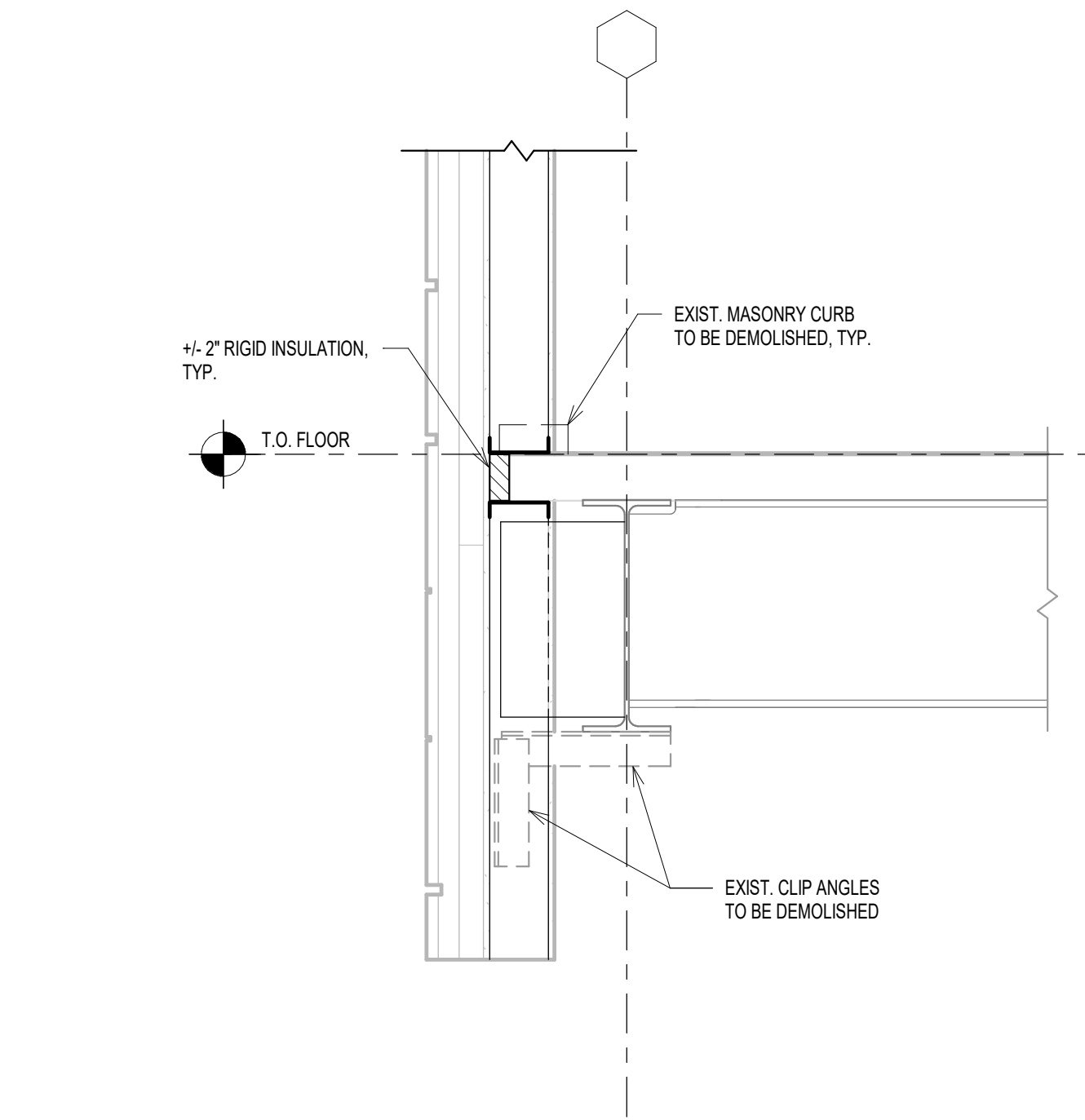
8 SECTION
3/4" = 1'-0"



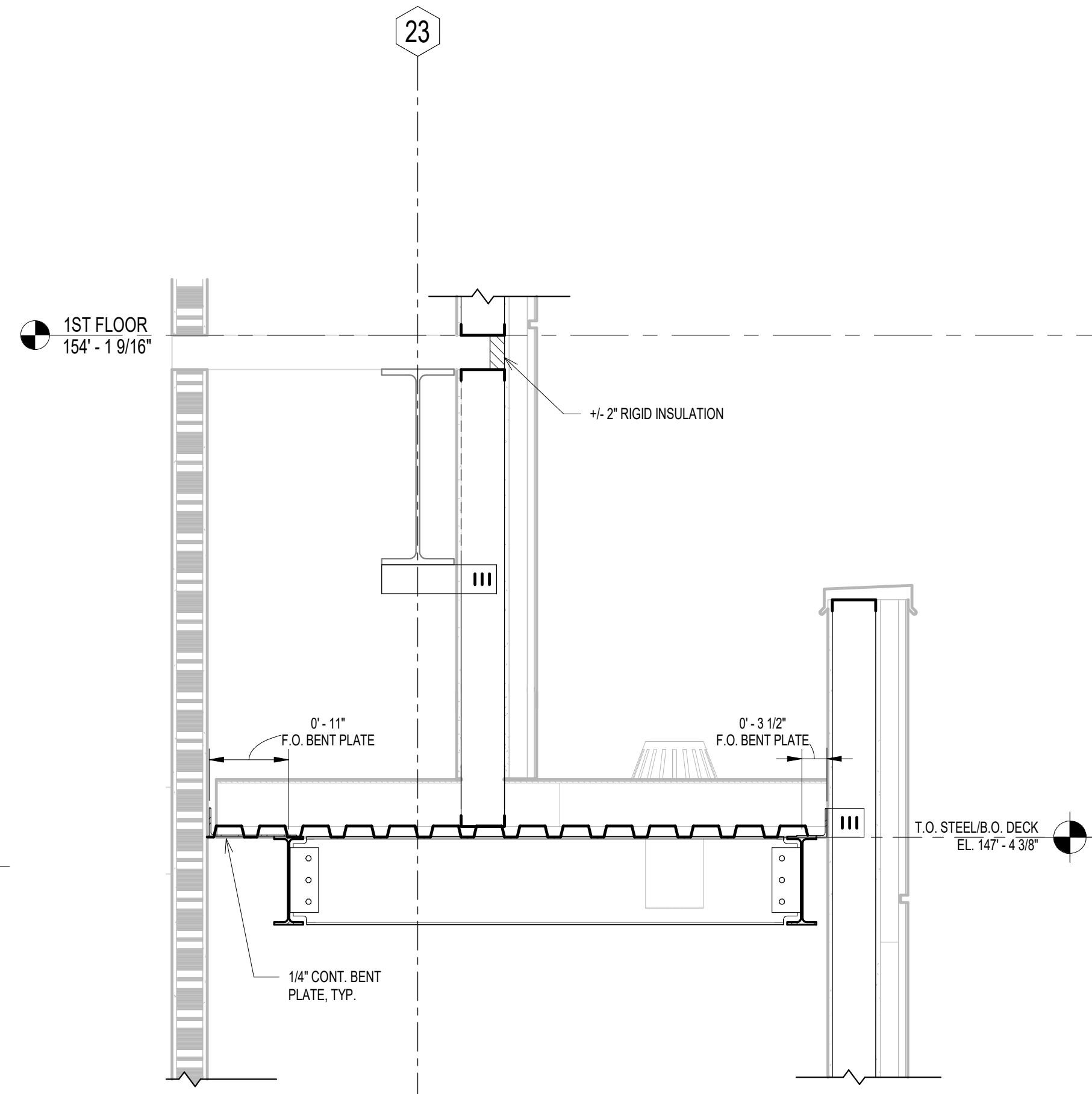
9 SECTION
3/4" = 1'-0"



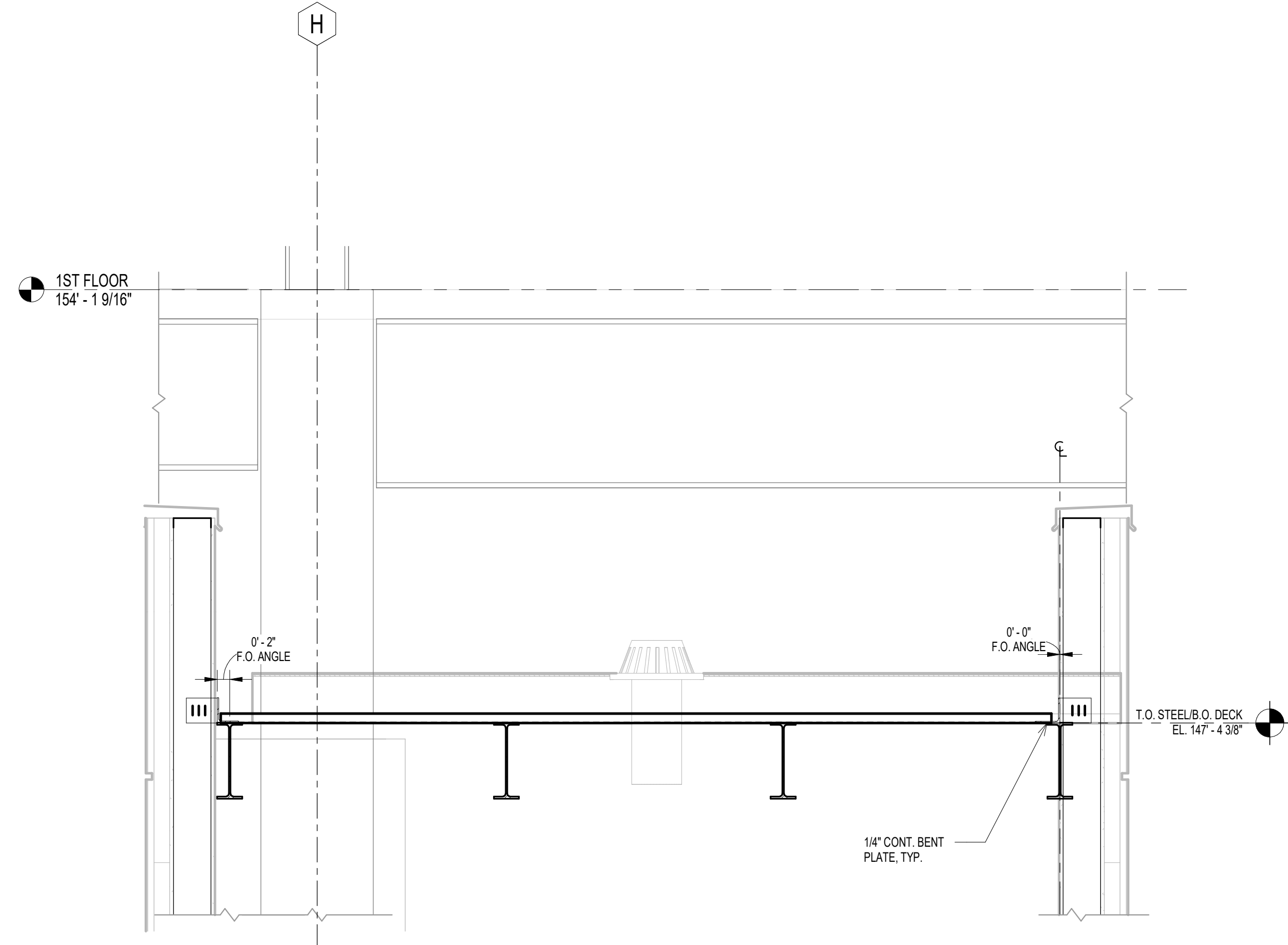
10 SECTION
3/4" = 1'-0"



11 SECTION
3/4" = 1'-0"



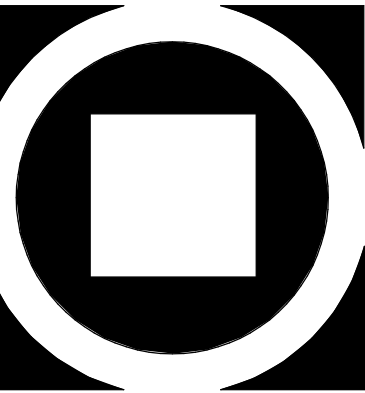
12 SECTION AT EMPLOYEE LOBBY
3/4" = 1'-0"



13 SECTION AT EMPLOYEE LOBBY
3/4" = 1'-0"



14 ROLL UP DOOR POST BASEPLATE
3/4" = 1'-0"



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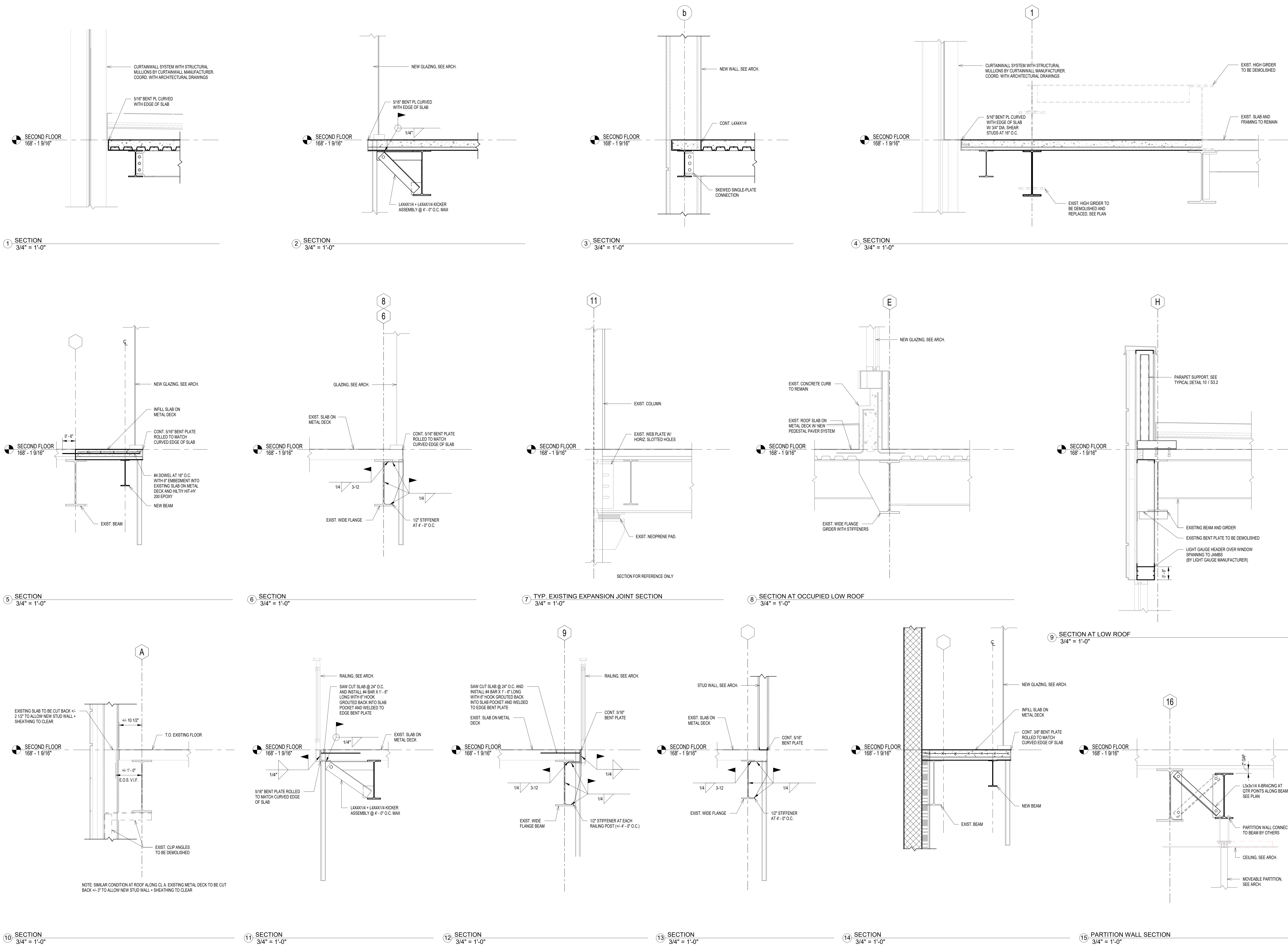


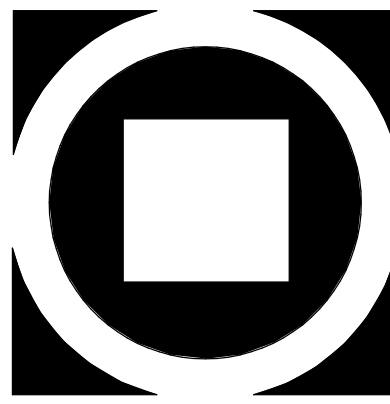
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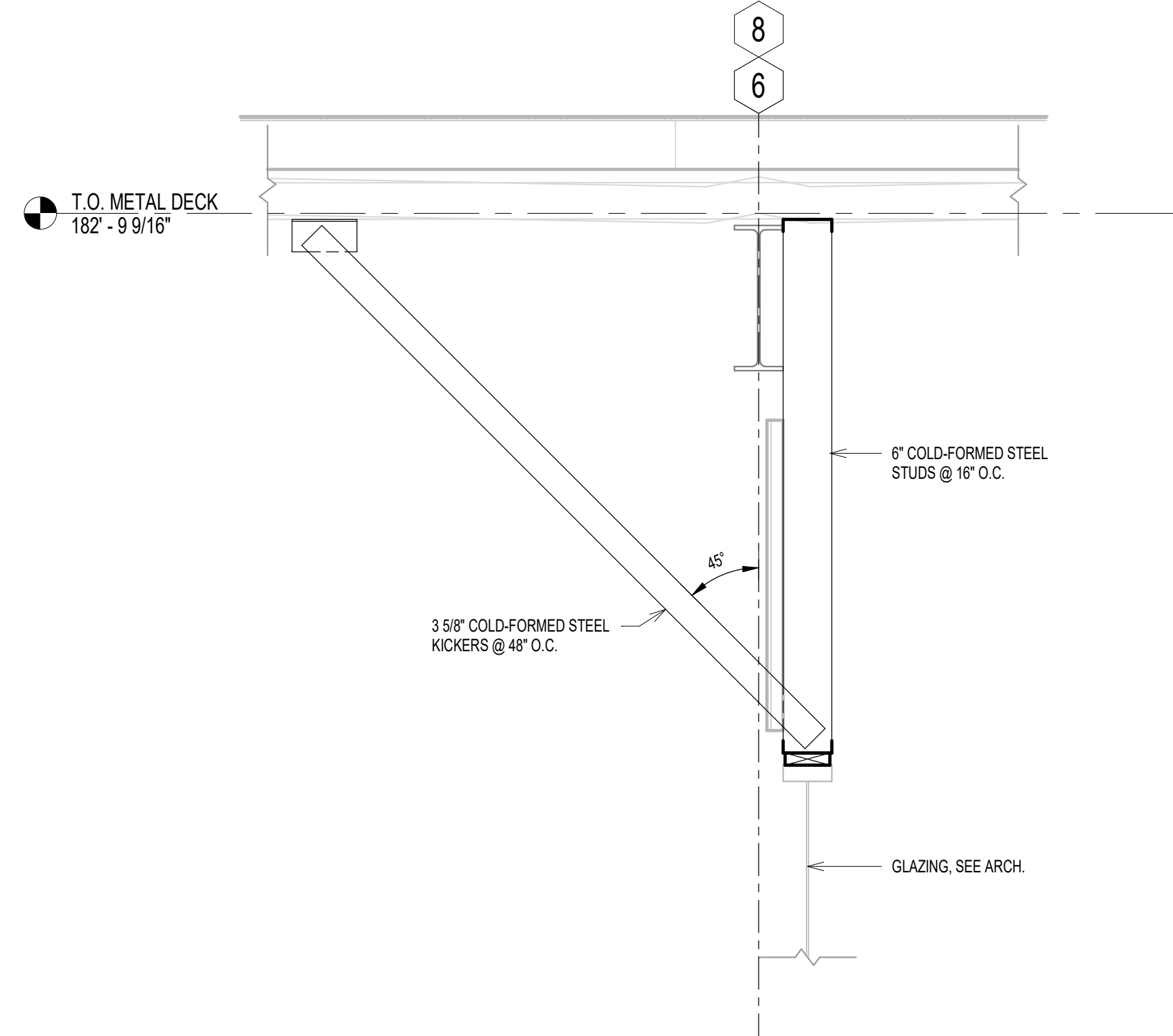
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SCALE:	AS NOTED
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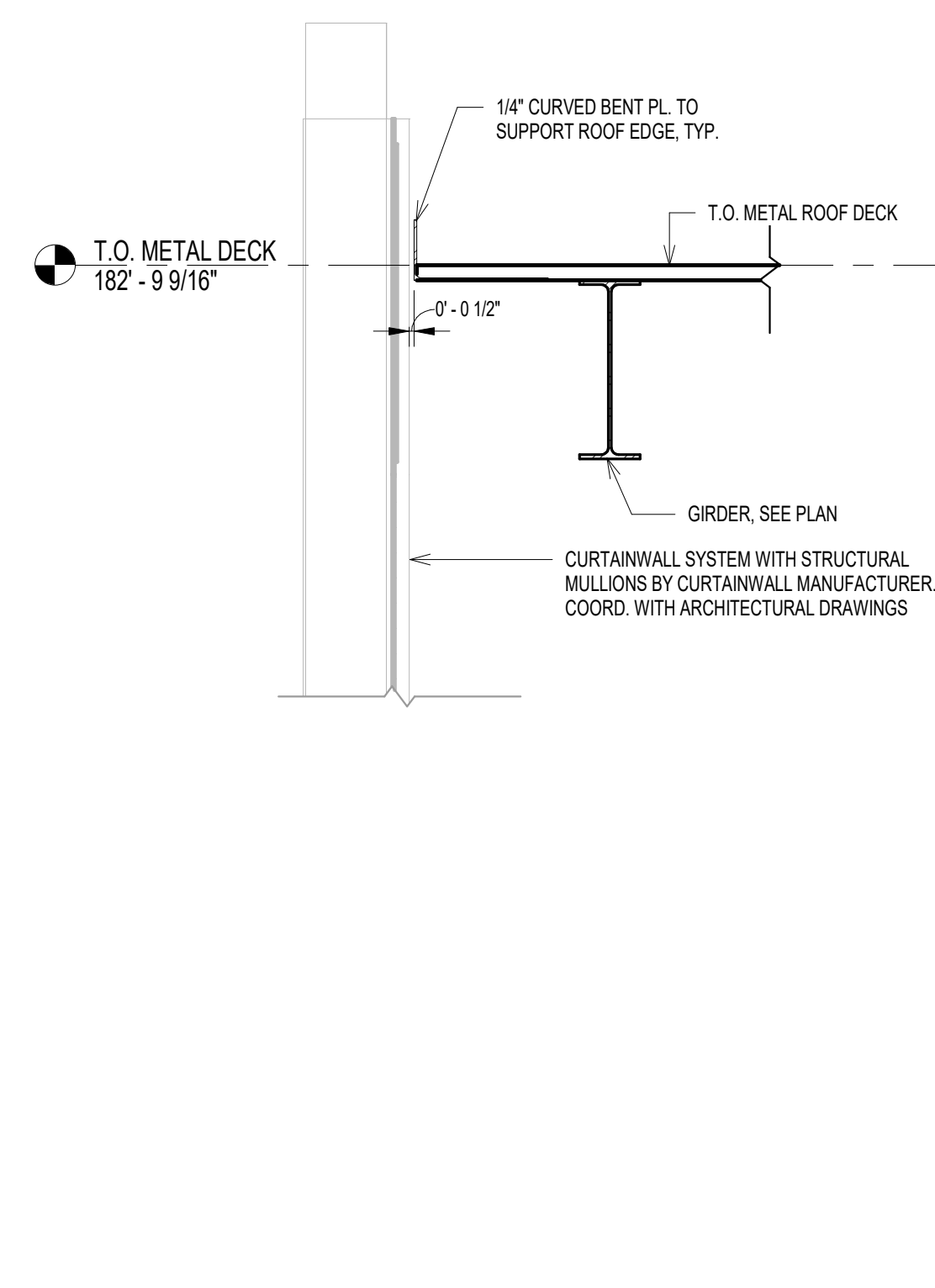
SECTIONS	NO.	DESCRIPTION	DATE
	75% GMP	BID & PERMIT	08/22/25
			10/03/25

S4.3

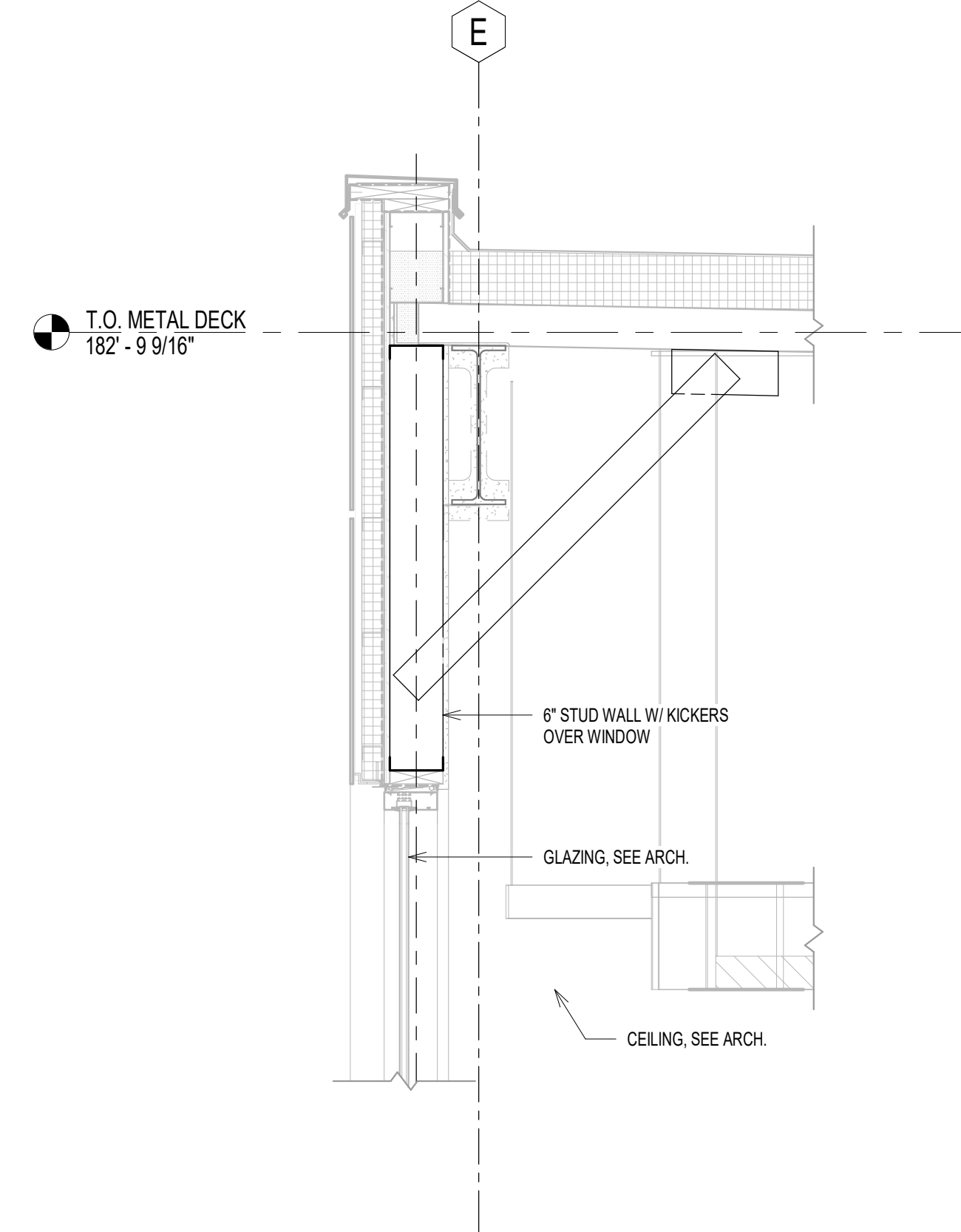
1 SECTION
3/4" = 1'-0"



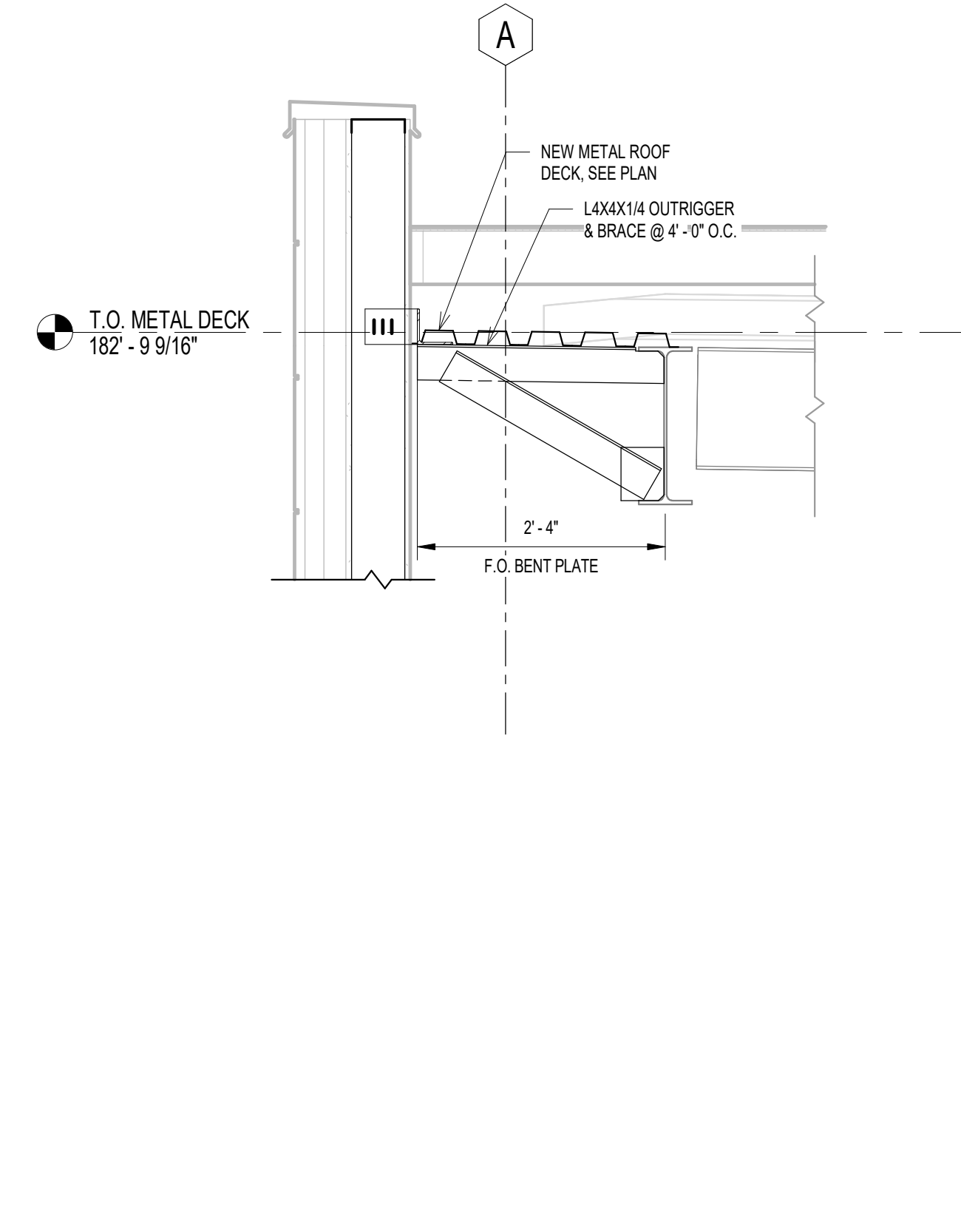
2 SECTION
3/4" = 1'-0"



3 SECTION
3/4" = 1'-0"



4 SECTION
3/4" = 1'-0"



SECTIONS

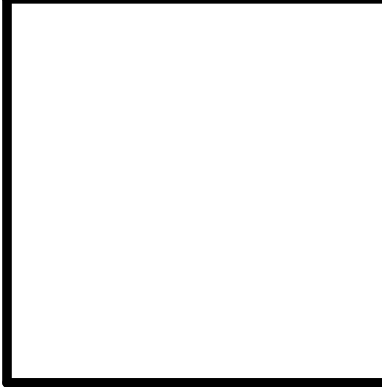
NO.	DESCRIPTION	DATE
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BID & PERMIT		10/03/25

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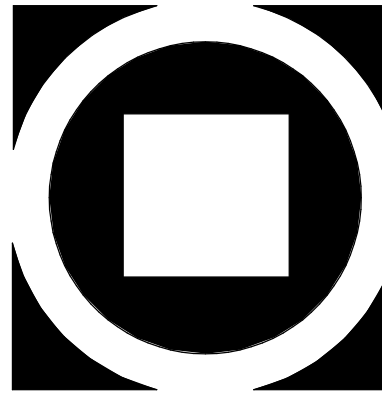
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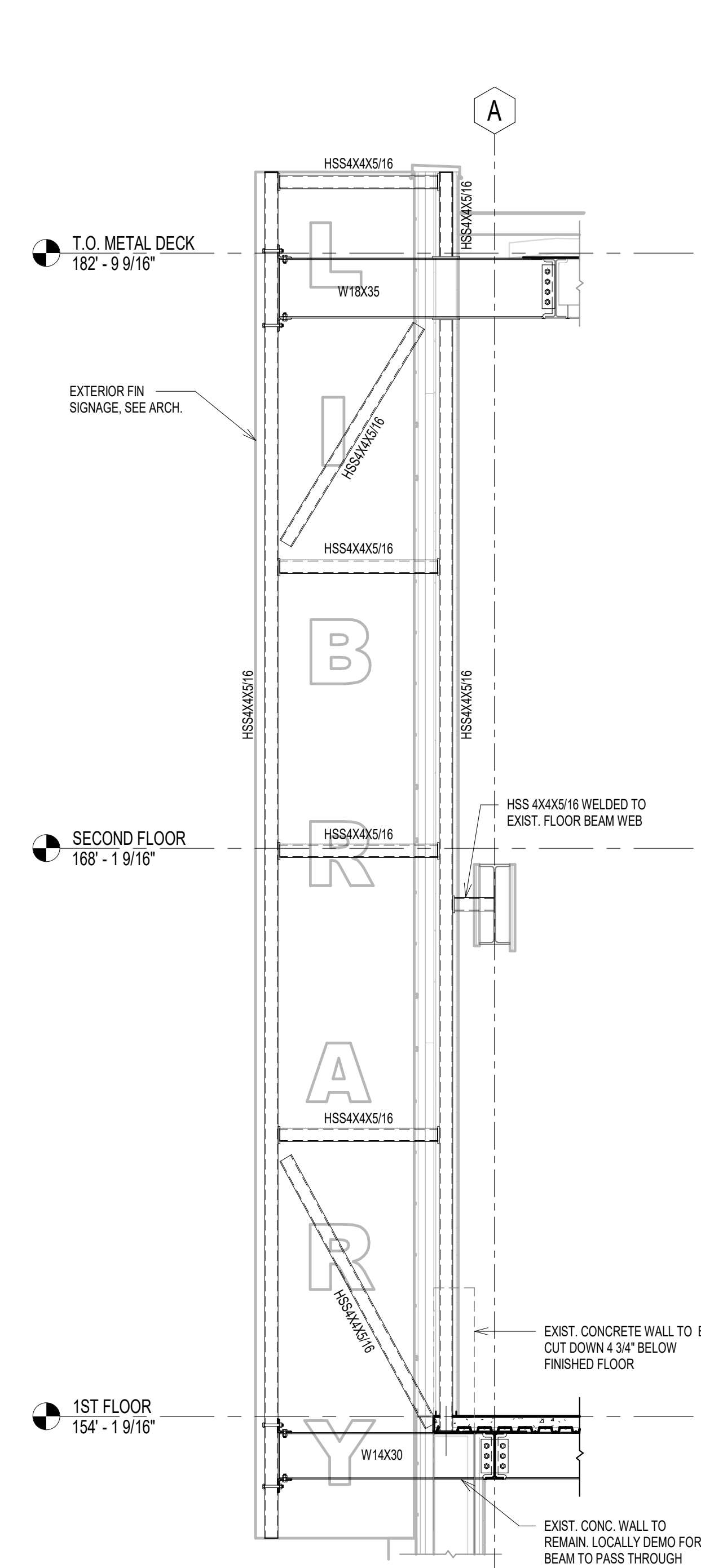


October 3, 2025

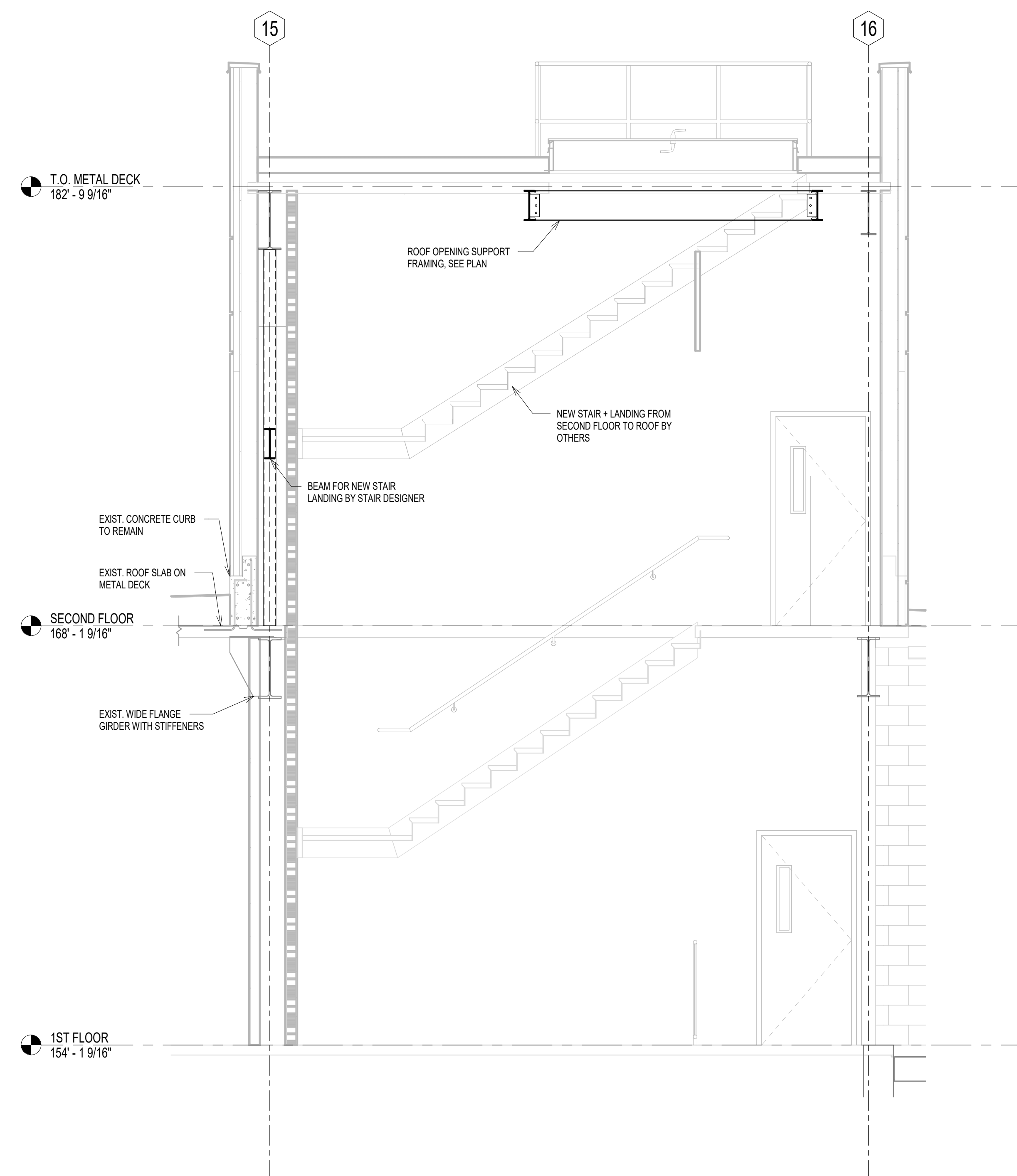


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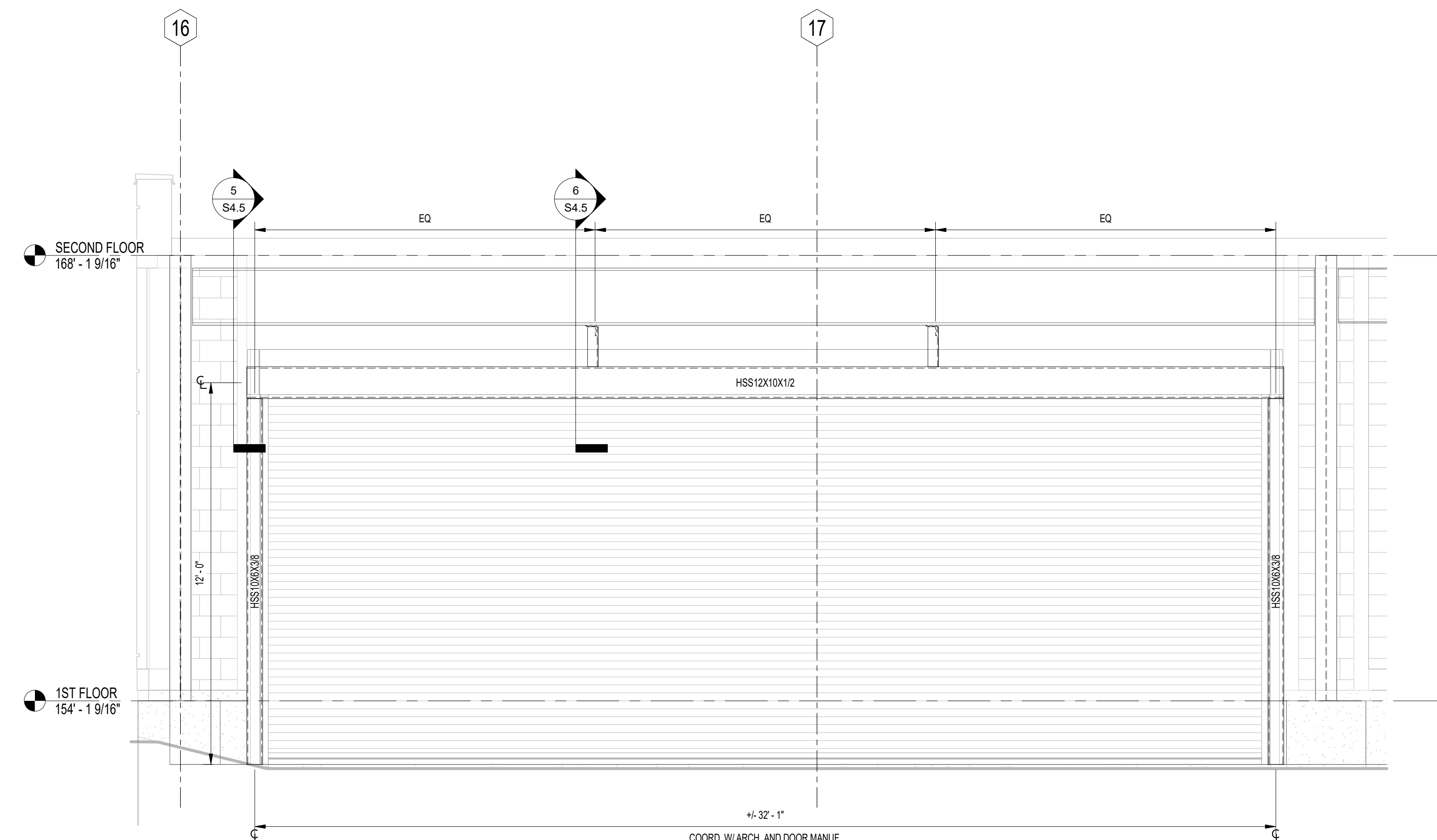




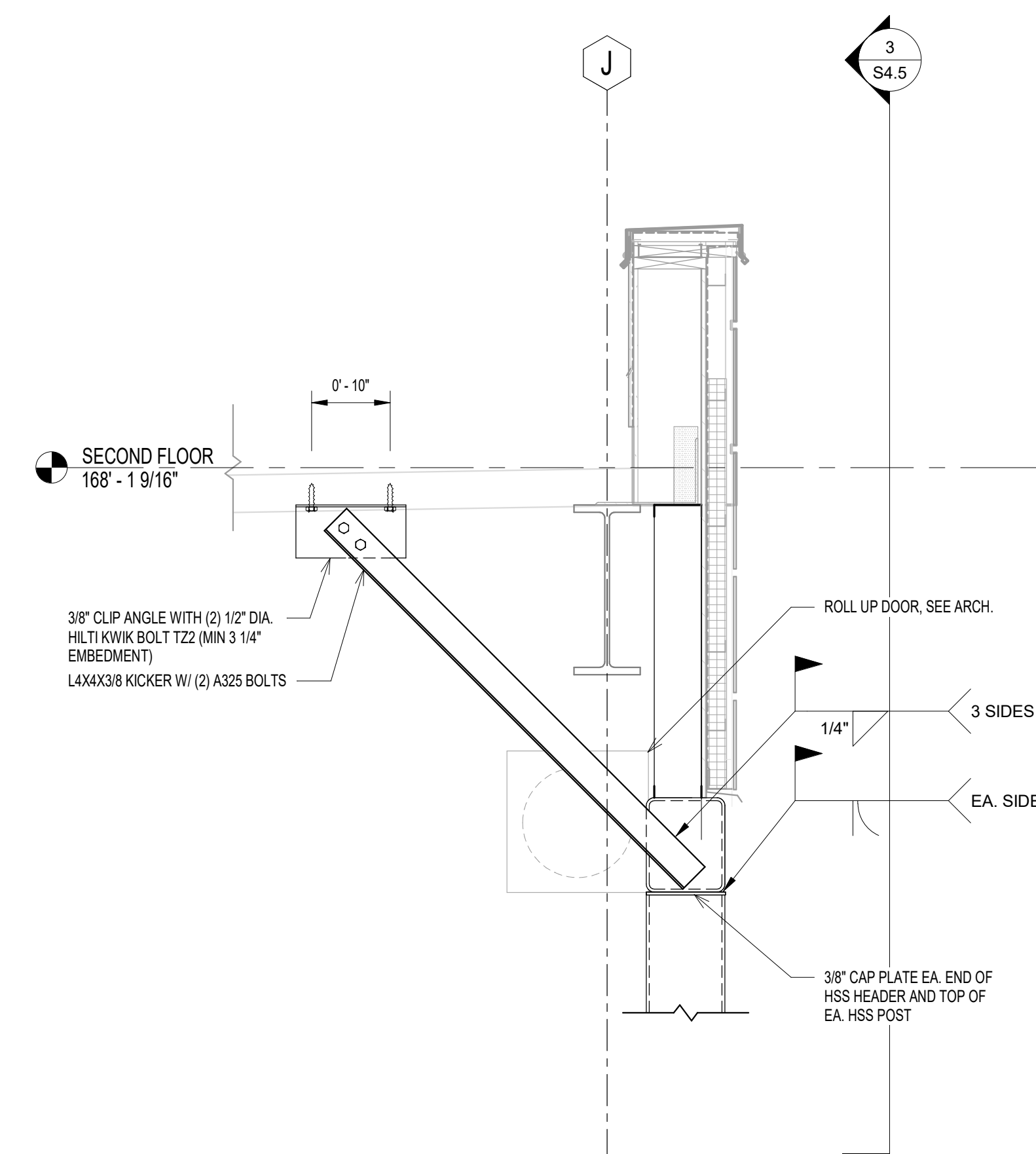
① SECTION AT FIN WALL
3/8" = 1'-0"



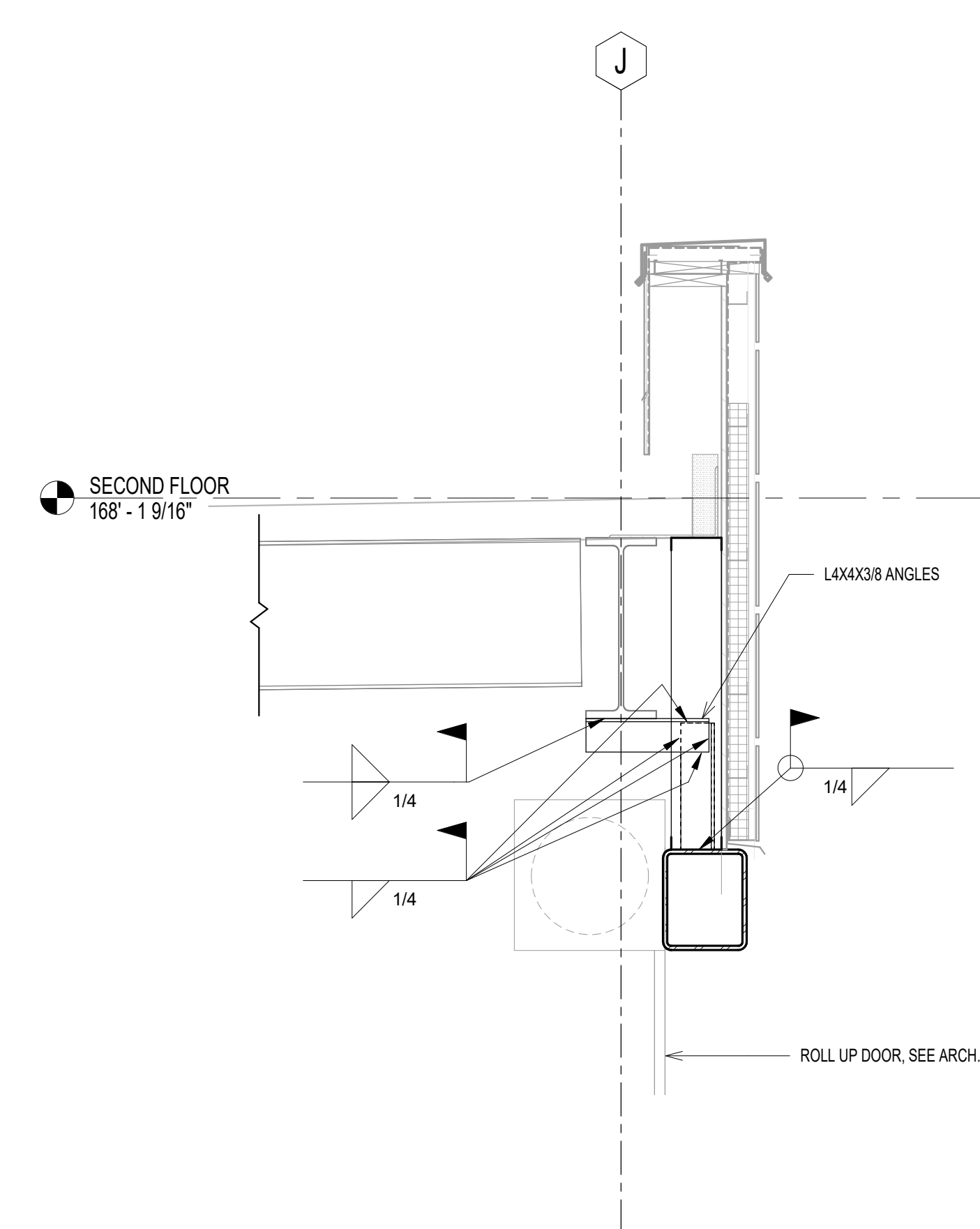
② SECTION AT STAIR EXTENSION
3/8" = 1'-0"



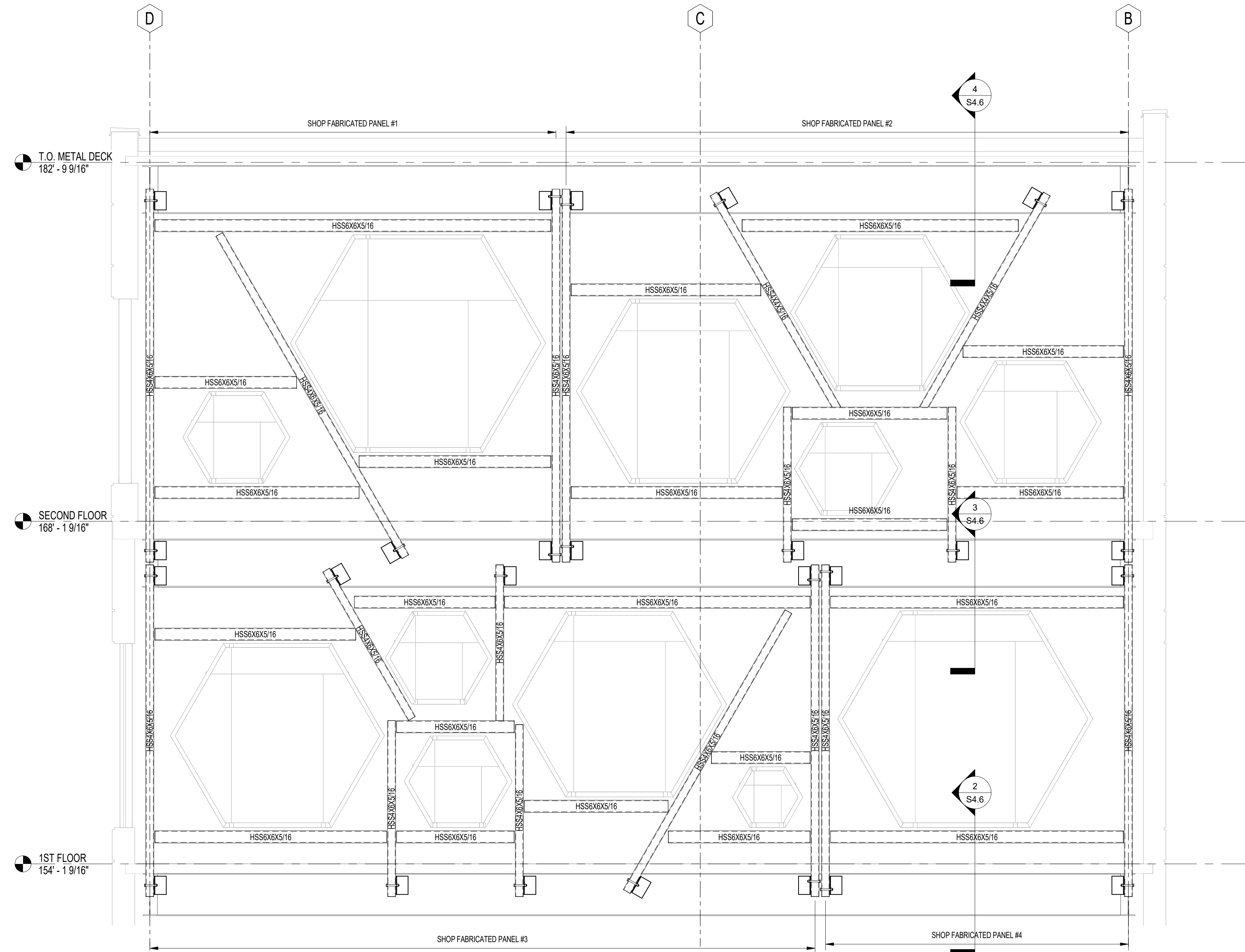
3 ROLL UP DOOR ELEVATION
3/8" = 1'-0"



5 SECTION AT ROLL UP DOOR
3/4" = 1'-0"

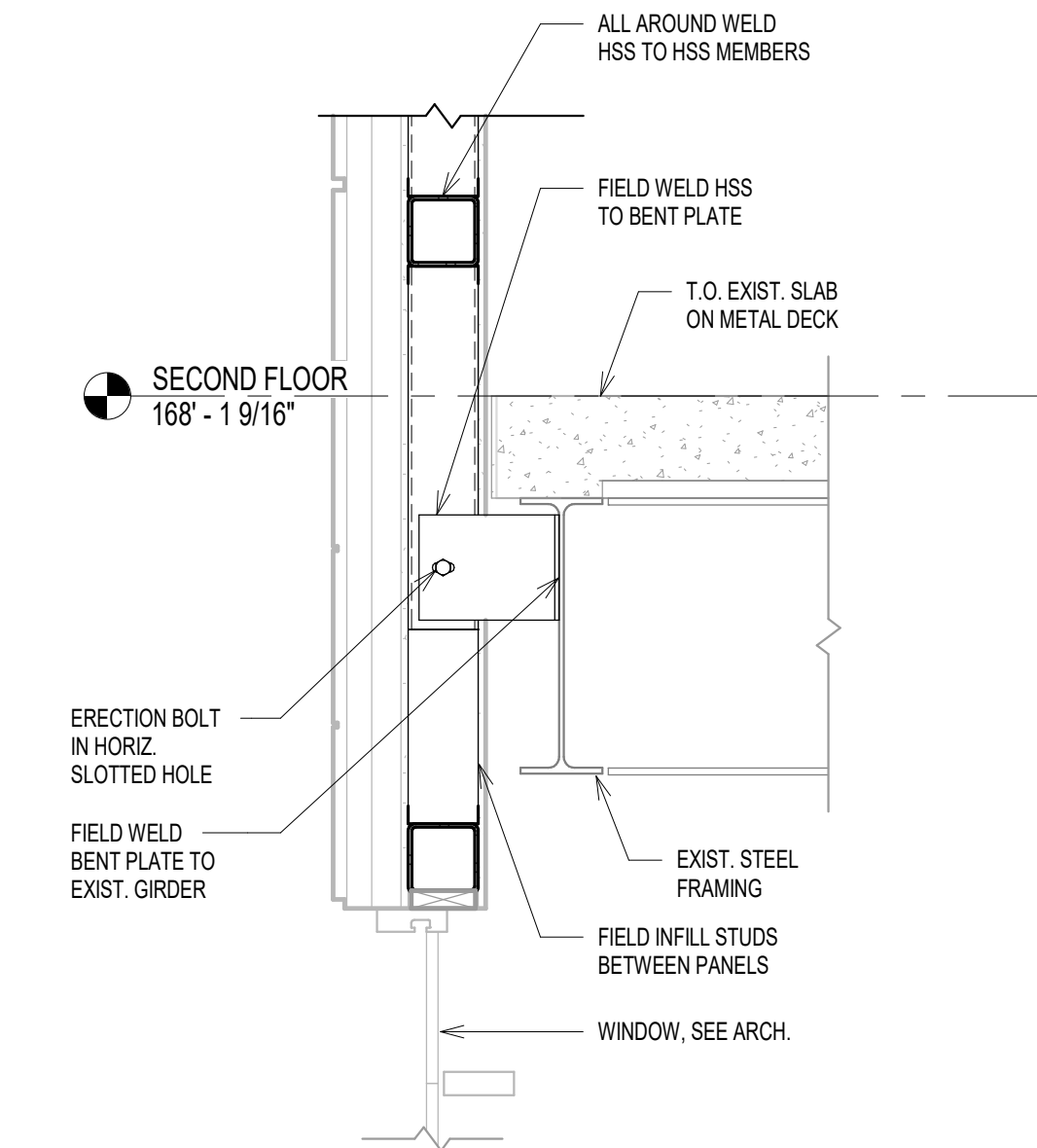


6 SECTION AT ROLL UP DOOR
3/4" = 1'-0"

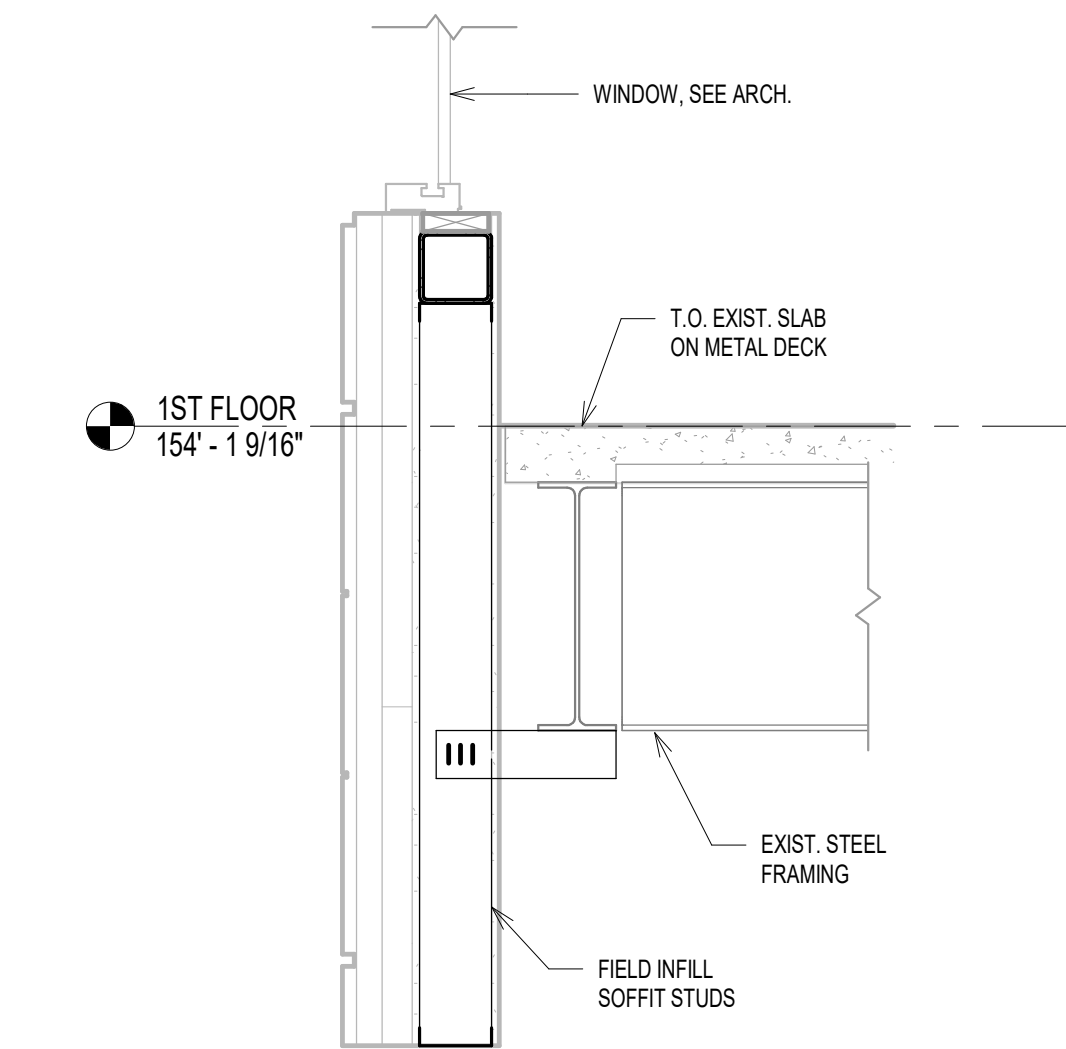


- SUGGESTED COLD-FORMED STUD INFFILL SEQUENCING:
1. SHOP FABRICATE/ASSEMBLE HSS PANELS.
 2. SHOP INSTALL COLD-FORMED STUD INFFILLS AROUND HEXAGON WINDOW OPENINGS WITHIN HSS FRAMEWORK.
 3. FIELD WELD BENT PLATES TO EXISTING STEEL GIRDERS. PROVIDE HORIZONTAL SLOTTED HOLE FOR PANEL INSTALLATION. FIELD WELD PANEL TO BENT PLATE ONCE IN PLACE.
 4. FIELD INSTALL COLD-FORMED STUD INFFILL FOR SOFFIT, PARAPET, AND BETWEEN PANELS.

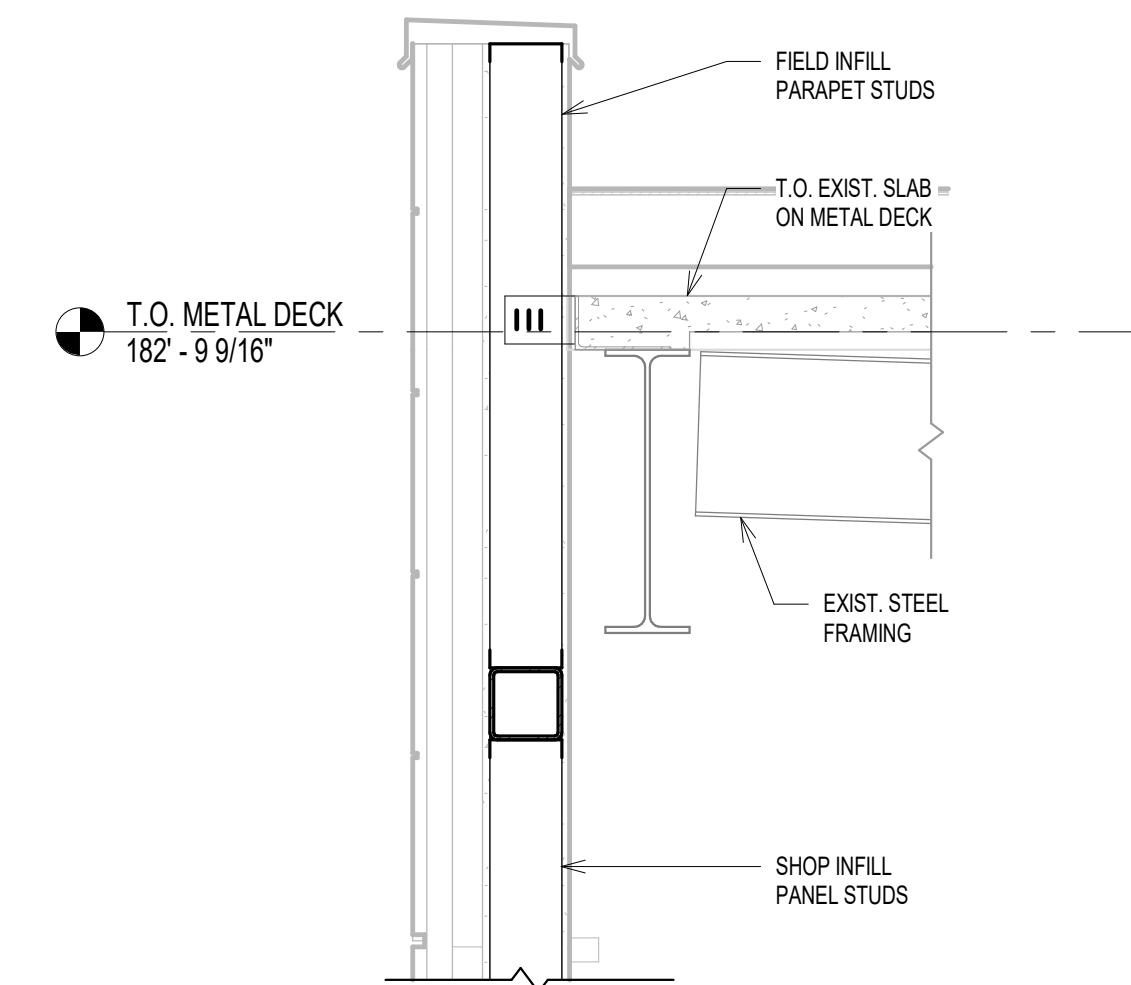
1 HEXAGON WINDOW WALL SUPPORT
3/8" = 1'-0"



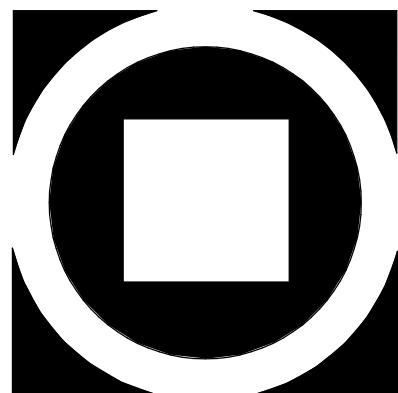
3 SECTION
3/4" = 1'-0"



2 SECTION
3/4" = 1'-0"



4 SECTION
3/4" = 1'-0"



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