

GENERAL NOTES

1. REFER TO THE PROJECT MANUAL FOR GOVERNING JOB REQUIREMENTS AND MATERIAL SPECIFICATIONS. NOTES ARE SUPPLEMENTAL TO THE PROJECT MANUAL AND ARE NOT INTENDED TO REPLACE THEM. IN THE EVENT OF CONFLICTING REQUIREMENTS BETWEEN NOTES AND SPECIFICATIONS, CONTRACTOR IS TO OBTAIN THE ENGINEER'S WRITTEN CLARIFICATION.
2. CONTRACTOR TO VERIFY EXISTING CONDITIONS AND REPORT, IN WRITING, ANY DISCREPANCIES TO THE ENGINEER PRIOR TO COMMENCING WORK.
3. DETERMINE EXACT LOCATION OF EXISTING UTILITIES BEFORE COMMENCING WORK. CONTRACTOR IS RESPONSIBLE FOR DAMAGES WHICH MIGHT BE OCCASIONED BY FAILURE TO LOCATE AND PRESERVE UTILITIES.
4. UNDERPINNING MAY BE REQUIRED AT E LINE. REFER TO DRAWINGS AND SPECIFICATIONS FOR UNDERPINNING REQUIREMENTS OF EXISTING STRUCTURES. COORDINATE WITH GEOTECHNICAL ENGINEER AND INDICATE EXTENTS OF UNDERPINNING ON DRAWINGS.
5. DRAWINGS REPRESENT THE BUILDING'S FINAL CONDITION. CONTRACTOR TO ADEQUATELY BRACE AND SUPPORT THE BUILDING AS REQUIRED THROUGHOUT CONSTRUCTION UNTIL ALL STRUCTURAL ELEMENTS REQUIRED FOR STABILITY ARE EFFECTIVE. THESE ELEMENTS ARE AS FOLLOWS: FLOOR DECK, ROOF DECK, MOMENT CONNECTIONS, ETC. CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF TEMPORARY SYSTEMS AND ARE TO BE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER REGISTERED IN THE COMMONWEALTH OF PENNSYLVANIA AND SUBMITTED TO ENGINEER FOR RECORD.
6. CONSTRUCTION AND ERECTION SEQUENCES ARE TO ACCOUNT FOR THERMAL MOVEMENT OF STRUCTURAL ELEMENTS UNTIL THE BUILDING ENCLOSURE IS TEMPERED.
7. CONTRACTOR IS RESPONSIBLE FOR MEANS, METHODS, SEQUENCE OF CONSTRUCTION, AND ADEQUACY OF THE BUILDING TO SUPPORT LOADS IMPOSED DURING CONSTRUCTION. LOADS IMPOSED ON THE BUILDING DURING CONSTRUCTION ARE NOT TO EXCEED THE DESIGN LOADS DEFINED ON S-001. REPAIRS TO THE BUILDING DURING CONSTRUCTION IS THE CONTRACTOR'S RESPONSIBILITY AND IS TO BE SUBMITTED, IN WRITING, FOR THE ENGINEER'S WRITTEN ACCEPTANCE.
9. CONTRACTOR TO REPORT, IN WRITING, ANY INCORRECTLY FABRICATED, DAMAGED, OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS TO THE ENGINEER FOR ACCEPTANCE PRIOR TO REMEDIAL OR CORRECTIVE WORK.
10. SECTIONS/DETAILS APPLY TO SIMILAR LOCATIONS AND CONDITIONS UNLESS OTHERWISE INDICATED.
11. (REFER TO THE ARCHITECTURAL, MECHANICAL, PLUMBING, FIRE PROTECTION, AND ELECTRICAL DRAWINGS FOR LOCATION AND SIZE OF CHASES, INSERTS, OPENINGS, SLEEVES, WASHES, DRIPS, REVEALS, DEPRESSIONS AND OTHER PROJECT REQUIREMENTS.)
12. COORDINATE LOCATION AND SIZE OF EQUIPMENT PADS WITH MECHANICAL, PLUMBING, FIRE PROTECTION, AND ELECTRICAL CONTRACTORS AND MANUFACTURERS.
13. EQUIPMENT OPERATING WEIGHTS AND/OR DIMENSIONS INDICATED ON DRAWINGS ARE ASSUMED. NOTIFY ENGINEER, IN WRITING AND PRIOR TO PURCHASING, IF ACTUAL WEIGHTS AND/OR DIMENSIONS ARE DIFFERENT.
14. REFER TO ARCHITECTURAL, MECHANICAL, PLUMBING, FIRE PROTECTION, AND ELECTRICAL DRAWINGS FOR LOCATION AND DIMENSIONS OF OPENINGS. REPORT, IN WRITING, OPENINGS WITH A SIDE DIMENSION OF 2'-0" OR LARGER WHICH ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS TO THE ENGINEER FOR REVIEW.

CAST-IN-PLACE CONCRETE NOTES

1. REFER TO DIVISION 3 OF THE PROJECT MANUAL FOR ADDITIONAL REQUIREMENTS.
2. DETAIL, FABRICATE, LABEL, SUPPORT AND PLACE CONCRETE REINFORCEMENT IN ACCORDANCE WITH ACI 315 "GUIDE TO PRESENTING REINFORCING STEEL DESIGN DETAILS" AND ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," APPLICABLE EDITIONS. PLACE CONCRETE IN ACCORDANCE WITH ACI 117 "SPECIFICATION FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS AND COMMENTARY," APPLICABLE EDITION.
3. A QUALITY CONTROL PROGRAM OF SPECIAL INSPECTIONS AND TESTING WILL BE PERFORMED ON STRUCTURAL CONCRETE WORK IN ACCORDANCE WITH THE SPECIFICATIONS. SCHEDULE WORK AND PROVIDE ACCESS TO ALLOW THE TESTING REQUIREMENTS TO BE COMPLETED.
4. SUBMIT ENGINEERED CONCRETE MIX DESIGNS, INCLUDING REQUIRED BACKUP DATA, FOR EACH TYPE OF CONCRETE PROPOSED FOR USE TO THE ENGINEER/ARCHITECT FOR REVIEW. ALLOW ADEQUATE TIME FOR REVIEW PRIOR TO PERFORMING CONCRETE WORK.
5. SUBMIT DETAILED SHOP DRAWINGS INDICATING REINFORCEMENT SIZE, SPACING AND PLACEMENT TO THE ENGINEER/ARCHITECT FOR REVIEW PRIOR TO FABRICATION. INCLUDE DETAILS AND LOCATIONS OF ALL CURBS, CONSTRUCTION JOINTS, SLAB DEPRESSIONS, SLEEVES, OPENINGS, ETC.
6. PROVIDE CONSTRUCTION AND CONTROL JOINTS AS INDICATED. HORIZONTAL CONSTRUCTION JOINTS ARE NOT PERMITTED UNLESS SPECIFICALLY NOTED. SUBMIT JOINT PLAN TO ENGINEER/ARCHITECT PROPOSED JOINT PLAN FOR REVIEW.
7. PROVIDE CORNER BARS IN STRIP FOOTINGS AND WALLS. THE SAME SIZE AND NUMBER AS CONTINUOUS REINFORCEMENT UNLESS NOTED OTHERWISE.
8. PROVIDE 3/4"x3/4" CHAMFER AT ALL EXPOSED CORNERS UNLESS NOTED OTHERWISE.
9. REINFORCEMENT LAP SPICE ARE TO BE CLASS B UNLESS NOTED OTHERWISE.
10. LAP WELDED WIRE REINFORCEMENT ONE FULL MESH +2 INCHES PANEL EXCEPT AT CONSTRUCTION JOINTS. PROVIDE TWO FULL MESH PANELS.
11. FOR STEEL FRAMED FLOORS, PROVIDE ADDITIONAL CONCRETE AS NECESSARY TO FINISH THE FLOORS TO WITHIN SPECIFIED TOLERANCES WHILE ACCOUNTING FOR METAL DECK AND STEEL BEAM DEFLECTIONS. ALLOW FOR AN AVERAGE OF AT LEAST 1/2 INCH EXTRA OF CONCRETE FOR EACH FLOOR.
12. PROVIDE FOUNDATION DOWELS TO MATCH WALL REINFORCEMENT SIZE AND SPACING.
13. COORDINATE WITH RESPECTIVE DISCIPLINE CONTRACTORS TO LOCATE OPENINGS/SLEEVES THROUGH WALLS AND SLABS. SHOW OPENINGS ON SHOP DRAWINGS AND PROVIDE ADDITIONAL REINFORCEMENT AS INDICATED. ADDITIONAL OPENINGS/SLEEVES ARE NOT PERMITTED BEYOND THOSE SHOWN ON THE REVIEWED SHOP DRAWINGS.
14. NO HOLES OR OPENINGS ARE PERMITTED THROUGH CONCRETE SLABS, OR WALLS EXCEPT AS FOLLOWS:
A. WHERE INDICATED AND AS DETAILED ON DRAWINGS.
B. MISCELLANEOUS HOLES THROUGH SLABS OR WALLS WHICH DO NOT DISPLACE REINFORCEMENT. THESE DO NOT REQUIRE ADDITIONAL REINFORCEMENT.
15. DO NOT PLACE CONDUITS WITHIN THE STRUCTURAL SLABS.
16. CENTER SINGLE LAYER OF VERTICAL WALL REINFORCEMENT WITHIN WALL UNLESS NOTED OTHERWISE.
17. REFER TO ACI 305 FOR REQUIREMENTS FOR PLACING CONCRETE IN HOT WEATHER AND TO ACI 306 FOR REQUIREMENTS FOR PLACING CONCRETE IN COLD WEATHER.
18. PLACE PIERS WHICH ARE INTEGRAL WITH FOUNDATION WALLS MONOLITHICALLY.
19. CONCRETE THAT WILL BE PLACED FURTHER THAN 16 FEET FROM THE END OF A CONCRETE TRUCK SHALL BE PUMPED WITH A COMMERCIAL CONCRETE PUMPING TRUCK OR OTHER PLACEMENT METHOD APPROVED BY THE ENGINEER. THE CONCRETE TRUCK SHALL NOT BE ALLOWED TO DRIVE OVER THE SUBGRADE OR THE SLAB REINFORCEMENT.
20. DO NOT WELD OR FIELD BEND REINFORCEMENT WITHOUT ENGINEER'S WRITTEN ACCEPTANCE.
21. PROVIDE A ROUGH CONCRETE SURFACE (1/4" MINIMUM AMPLITUDE) AT THE INTERSECTION OF CONCRETE WALLS, STEM WALLS, AND PLASTER WITH THE TOP OF FOOTINGS. DO NOT PROVIDE A KEYWAY UNLESS SHOWN OR NOTED ON THE DRAWINGS.
22. LOCATE ADDITIONAL CONSTRUCTION JOINTS AS REQUIRED TO FACILITATE CONSTRUCTION AS ACCEPTABLE TO ENGINEER. LOCATE WALL CONSTRUCTION JOINTS AT MASONRY CONTROL JOINTS WHERE POSSIBLE. PLACE REINFORCEMENT CONTINUOUSLY THROUGH JOINT. DETAIL JOINT AND SHOW ON SHOP DRAWINGS.
23. WHERE MASONRY VENEER IS LOCATED ADJACENT TO WALLS, STEM WALLS, & COLUMNS GREATER THAN 24 INCHES HIGH, PROVIDE FULL HEIGHT DOVETAIL SLOT INSERTS AT 2'-0" OC FOR MASONRY ANCHORS. PLACE ANCHORS AT 16" OC VERTICALLY. COORDINATE WITH ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.
24. NO PIPING OR CONDUITS SHALL BE INSTALLED IN ANY CONCRETE WITHOUT THE APPROVAL OF THE ENGINEER.

PROVIDE CLEAR COVER FOR ALL REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE IN THE DETAILS:

SPECIFIED COVER, INCHES				
CAST AGAINST EARTH AND PERMANENTLY IN CONTACT WITH GROUND			3	
EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: NO. 6 THROUGH NO. 18 BARS			2	
NO. 5 BAR AND SMALLER OR WELDED WIRE REINFORCEMENT			1 1/2	
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, JOISTS, AND WALLS NO. 14 AND NO. 18 BARS			1 1/2	
NO. 11 BAR AND SMALLER			3/4	
BEAMS, COLUMNS, PEDESTALS, AND TENSION TIES				
PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, AND HOOPS			1 1/2	

26. PROVIDE CONCRETE AND REINFORCING MATERIALS OF THE TYPES AND GRADES LISTED IN THE TABLE BELOW:

	EXPOSURE CLASS	28 DAY MINIMUM COMPRESSIVE STRENGTH		NOMINAL MAXIMUM AGGREGATE SIZE	AIR CONTENT PERCENT
		F2	F4		
FOUNDATIONS/FOOTINGS	F2	3,000 PSI	0.45	3/4"	6%
WALLS	F2	4,000 PSI	0.45	3/4"	6%
SLAB-ON-GROUND (INTERIOR)	F2	4,500 PSI	0.45	1"	6%
SLAB-ON-GROUND (EXTERIOR)	F2	4,500 PSI	0.45	1 1/2"	5.5%
FLOOR SLAB OVER METAL DECK	F2	4,000 PSI	0.45	1"	6%

REINFORCEMENT	
TYPICAL BARS	ASTM A615, GRADE 60
WELDED BARS	ASTM A706, GRADE 60
WELDED WIRE REINFORCEMENT	ASTM A1064

STEEL NOTES

1. REFER TO DIVISION 5 SPECIFICATION SECTION - STRUCTURAL STEEL FRAMING - FOR ADDITIONAL INFORMATION.
2. DETAIL, FABRICATE, AND ERECT STRUCTURAL STEEL FRAMING IN ACCORDANCE WITH THE LATEST AISC STEEL CONSTRUCTION MANUAL AND ANSI/AISC 360, SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, APPLICABLE EDITIONS.
3. CONFORM TO AWS D1.1, STRUCTURAL WELDING CODE-STEEL, APPLICABLE EDITION FOR ALL WELDED CONNECTIONS.
4. ANY CONNECTIONS WITHOUT WELD SYMBOLS SHALL BE AT A MINIMUM WELDED ALL AROUND WITH THE MINIMUM FILLET OR BUTT WELD SIZE.
5. STRUCTURAL STEEL ANGLES, PLATES, ETC. SHALL CONFORM TO ASTM A36 REQUIREMENTS (36 KSI). STRUCTURAL STEEL W AND C SHAPES SHALL CONFORM TO ASTM A992 (50 KSI). STRUCTURAL TUBING AND PIPES SHALL CONFORM TO THE ASTM A500 GRADE C REQUIREMENTS (50 KSI).
6. DO NOT PLACE HOLES THROUGH STRUCTURAL STEEL MEMBERS EXCEPT AS SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.
7. CONNECTIONS:
A. ALL BOLTED CONNECTIONS ARE TO BE 3/4" MINIMUM DIAMETER A325 TYPE N OR SC BOLTS IN STANDARD HOLES UNLESS NOTED OTHERWISE OR AS DETERMINED BY THE CONNECTION DESIGNER OR NOTED ON THE PLANS. DESIGN USING STANDARD HOLES UNLESS OTHERWISE NOTED OR REQUIRED FOR ERECTION.
B. MINIMUM CAPACITY OF BEAM CONNECTIONS: FOR CONNECTIONS NOT DETAILED, PROVIDE CONNECTION CAPACITY OF AT LEAST THAT REQUIRED BY PART 3 OF THE AISC MANUAL. IN THE SECTION "MAXIMUM TOTAL UNIFORM LOAD TABLES" FOR LRFD DESIGN OR "ALLOWABLE LOADS ON BEAMS" FOR ALLOWABLE STRESS DESIGN, FOR THE GIVEN MEMBER AND STEEL SPECIFICATIONS. CONCENTRATED LOADS NEAR SUPPORTS MUST BE ADDED.
C. THE DESIGN SHEAR FOR EACH CONNECTION UNLESS NOTED ON THE DRAWINGS SHALL BE 110% OF THE REACTION FROM A UNIFORM LOAD OVER THE SPAN WHICH CREATES THE MAXIMUM DESIGN MOMENT FOR ROOF/FLOOR BEAM CONNECTIONS. THE MINIMUM REACTION SHALL BE 14 KIPS.
D. INDICATOR BOLTS EQUAL TO TENSION FOR CONTROL BOLTS OF THE L&L/ENCOMPANY OF BURNSVILLE, PENNSYLVANIA. (800-857-2558) SHALL BE USED.
E. CONNECTION DESIGN BY FABRICATOR WILL BE SUBJECT TO REVIEW AND APPROVAL BY ENGINEER. USE MINIMUM OF TWO 3/4" DIAMETER A325 BOLTS PER CONNECTION.
8. FRAME ALL OPENINGS IN ROOF DECK 8" OR LARGER WITH A STRUCTURAL STEEL MEMBER ON ALL SIDES EXCEPT WHERE A SIDE MAY BE WITHIN 1'-0" OR ANOTHER FRAMING MEMBER. FRAMING NOT REQUIRED FOR ROOF OPENINGS SMALLER THAN 18"
9. PROVIDE 3/8" STIFFENER PLATES ON EACH SIDE OF THE WEB OF ALL BEAMS AT ALL SUPPORTS THAT ARE BELOW THE BEAM, AND AT ALL COLUMNS THAT ARE ABOVE THE BEAM, OMITTING WHERE BEAMS INTERSECT.
10. SUBMITTALS:
A. PRODUCT DATA OR MANUFACTURER'S SPECIFICATIONS AND INSTALLATION INSTRUCTIONS FOR THE FOLLOWING PRODUCTS. INCLUDE LABORATORY TEST REPORTS AND OTHER DATA TO SHOW COMPLIANCE WITH SPECIFICATIONS:
a. STRUCTURAL STEEL INCLUDING CERTIFIED COPIES OF MILL REPORTS COVERING CHEMICAL AND PHYSICAL PROPERTIES.
b. HIGH STRENGTH BOLTS (EACH TYPE), INCLUDING NUTS AND WASHERS, FOR INFORMATION ONLY.
c. STRUCTURAL STEEL PRIMER PAINT.
d. SHRINKAGE RESISTING GROUT.
B. SHOP DRAWINGS PREPARED UNDER SUPERVISION OF A LICENSED STRUCTURAL ENGINEER, INCLUDING COMPLETE DETAILS AND SCHEDULES FOR FABRICATION AND ASSEMBLY. DUPLICATION OF CONTRACT DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.

STEELDECK NOTES

1. STEEL DECK SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE CURRENT SPECIFICATION OF THE STEEL DECK INSTITUTE.
2. SHOP DRAWINGS SHALL INDICATE THE FINISH, TYPE, GAGE, DIMENSIONS, AND LAYOUT OF ALL DECK AND ACCESSORIES. DRAWINGS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.

FLOOR DECK NOTES

1. TYPICAL STEEL FLOOR DECK SHALL BE COMPOSITE STEEL FLOOR DECK. THIS DECK SHALL BE 1 1/2" DEEP, 20 GAUGE AND 9/16" DEEP, 20 GAUGE GALVANIZED DECK. REFER TO PROJECT SPECIFICATION.
2. THE STEEL FLOOR DECK SHALL BE SUPPLIED IN MINIMUM LENGTHS REQUIRED TO PROVIDE A "3 SPAN" CONDITION. END CLOSURES, CLOSURES AT PENETRATIONS, AND ALL OTHERS ACCESSORIES NECESSARY FOR A COMPLETE INSTALLATION ARE REQUIRED.
3. STEEL FLOOR DECK SHALL BE WELDED TO THE SUPPORTING STEEL (FRAME FASTENING) WITH 5/8" DIAMETER ARC SPOT FUSION WELDS IN A 36X PATTERN. INTERMEDIATE SIDE CONNECTIONS (STITCH FASTENING) AND PERIMETER EDGE FASTENING SHALL BE MADE WITH TWO (2) #10 SELF-TAPPING SCREW OR 1 1/2" LONG FILLET WELD PER SPAN. SPACING OF STITCH FASTENERS SHALL NOT EXCEED 18" ON CENTER, MAX.
4. CONCRETE FILL IN COMPOSITE METAL FLOOR DECK TO BE 2 1/2" THICK ABOVE DECK FLUTES/4" OVERALL THICKNESS OF DECK AND CONCRETE). REINFORCE CONCRETE WITH 6#6-W2.9/2.9 WELDED WIRE FABRIC SHEETS (ROLLS NOT PERMITTED) AT 3/4" BELOW TOP OF SLAB.
5. CONCRETE FILL IN METAL FORM DECK TO BE 2 3/4" THICK ABOVE DECK FLUTES 3 1/2" OVERALL THICKNESS OF DECK AND CONCRETE. REINFORCE WITH 6#6 W2.9/2.9 WELDED WIRE FABRIC SHEETS (ROLLS NOT PERMITTED) AT 3/4" BELOW TOP OF SLAB.

STEEL JOIST AND ROOF DECK NOTES

1. STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS FOR OPEN WEB JOISTS. MATERIAL SHALL BE DOMESTIC STEEL WITH ANGLES FOR BOTTOM CHORDS.
2. ROOF DECK SHALL BE TYPE B-20 GAGE AS SPECIFIED BY THE STEEL DECK INSTITUTE AND SHALL BE PAINTED. DECK SHALL BE WELDED TO SUPPORTS AT 6 INCHES ON CENTER AT LAPS AND ENDS, AND AT 12 INCHES ON CENTER SUPPORTS BETWEEN LAPS, WITH SIDELAP SCREWS AT MIDSPAN. DECK SHALL BE MODIFIED AS REQUIRED AT EDGE SUPPORTS PARALLEL TO THE RIBS SO THAT THE DECK CAN BE WELDED TO THE SUPPORT AT 24 INCHES ON CENTER. WELDS SHALL BE 9/8" FUSION WELDS WITH A MAXIMUM BURN-THROUGH OF 30 PERCENT AND WITH A MAXIMUM OF 10 PERCENT FAILING. ALL SCREWS SHALL BE TAKEN BY ITW BUILD-EX, OR APPROVED EQUAL. ALL SCREWS IN EXTERIOR WALLS SHALL BE ZINC PLATED WITH A TYPE II ASTM B 633 COATING.
3. ROOF DECK SHALL BE TYPE B-20 GAGE AS SPECIFIED BY THE STEEL DECK INSTITUTE AND SHALL BE PAINTED. DECK SHALL BE POWDER ACTUATED (RAMNET) TO SUPPORTS AT 8 INCHES ON CENTER AT LAPS, AT THE END OF THE SHEETS, AND AT 12 INCHES ON CENTER AT SUPPORTS BETWEEN LAPS WITH SIDELAP SCREWS AT MIDSPAN. DECK SHALL BE MODIFIED AS REQUIRED AT EDGE SUPPORTS PARALLEL TO THE RIBS SO THAT THE DECK CAN BE DRIVEN TO THE SUPPORT AT 24 INCHES ON CENTER. SIDELAP SCREWS SHALL BE #12 T&S. CONSULT HILTI FOR RECOMMENDED SYSTEM AND PIN SPECIFICATIONS OF POWDER DRIVEN NAILS THROUGH DECK ONTO STEEL OF EACH YIELD STRENGTH. ALL POWDER DRIVEN NAILS SHALL BE INSTALLED BY PERSONS TRAINED AND LICENSED BY HILTI. ALL SCREWS SHALL BE TAKEN BY ITW BUILD-EX, OR APPROVED EQUAL. ALL SCREWS IN EXTERIOR WALLS SHALL BE ZINC PLATED WITH A TYPE II ASTM B 633 COATING.
4. ALL JOISTS SHALL HAVE BOTTOM CHORD UPLIFT BRIDGING AT EACH END AND BE DESIGNED FOR 25 POUNDS PER SQUARE FOOT NET UPLIFT.
5. PROVIDE L3X3X1/4 AROUND ALL ROOF PENETRATIONS NOT SHOWN, 8 INCHES OR GREATER, FOR METAL DECK SUPPORT.
6. LOCATE CONCENTRATED LOADS ON JOISTS AND JOIST GIRDERS AT PANEL POINTS. PROVIDE ANGLE WEB MEMBERS TO CREATE INTERMEDIATE PANEL POINTS AS REQUIRED. MANUFACTURER SHALL DIRECT INSTALLER AS TO METHOD OF INSTALLATION AND MATERIAL REQUIRED. JOISTS SHALL BE SHOP REINFORCED FOR ALL LOADS PROVIDED ON DRAWINGS. FIELD REINFORCING SHALL BE PROVIDED AS DETAILED ON THE DRAWINGS.
7. VERIFY SIZE, LOCATION, AND NUMBER OF ROOF OPENINGS WITH MECHANICAL AND ELECTRICAL PLANS AND CONTRACTORS.
8. PROVIDE BRIDGING ANCHORS FIRMLY ANCHORED TO MASONRY WALLS, BEAMS, AND COLUMNS AT EACH END OF EACH ROW OF BRIDGING, TOP AND BOTTOM.
9. INSTALL JOISTS WITHIN A HORIZONTAL SWEEP TOLERANCE OF 1/4" IN 10 FEET.
10. STEEL JOIST SUBMITTALS:
A. PRODUCT DATA AND INSTALLATION INSTRUCTIONS FOR EACH TYPE OF JOIST AND ACCESSORY. INCLUDE MANUFACTURER'S CERTIFICATION THAT JOISTS COMPLY WITH SJI SPECIFICATIONS.
B. SHOP DRAWINGS SHOWING JOIST LAYOUT, CONNECTIONS AND LOCATION AND SPACING OF BRIDGING. DUPLICATION OF CONTRACT DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
11. ROOF DECK SUBMITTALS:
A. PRODUCT DATA AND INSTALLATION INSTRUCTIONS FOR EACH TYPE OF DECKING AND ACCESSORIES.
B. SHOP DRAWINGS SHOWING LAYOUT AND TYPES OF DECK UNITS, ANCHORAGE DETAILS AND CONDITIONS REQUIRING CLOSURE STRIPS AND OTHER ACCESSORIES. DUPLICATION OF CONTRACT DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.

DESIGN CRITERIA

DESIGN LOADS ARE BASED ON THE ASCE 7.16 BUILDING DESIGN LOADS.

SECOND FLOOR DEAD LOADS:

2.9" SLAB ON 8" METAL DECK:	41 PSF
METAL DECK/PLUMB:	5 PSF
JOISTS/BEAMS:	9 PSF
SUSPENDED CEILING:	2 PSF
TOTAL:	57 PSF

ROOF DEAD LOADS:

EPDM ROOFING:	2 PSF
JOISTS/BEAMS:	8 PSF
METAL DECK:	2 PSF
METAL DECK/PLUMB:	5 PSF
INSULATION:	3 PSF
TOTAL:	20 PSF

FLOOR LIVE LOADS:

	UNIFORM	CONCENTRATED (UNIFORMLY DISTRIBUTED OVER 2 1/2 FEET BY 2 1/2 FEET)
GARAGE	250 PSF	16,000 POUNDS
OFFICE BUILDINGS		
CORRIDOR ABOVE FIRST FLOOR:	100 PSF	2,000 POUNDS
LOBBIES AND FIRST-FLOOR CORRIDORS:	100 PSF	2,000 POUNDS
SECOND FLOOR:	100 PSF	2,000 POUNDS
STAIRS AND EXITS:	100 PSF	300 POUNDS

ROOF LIVE LOAD:

20 PSF

ROOF SNOW LOAD:

GROUND SNOW LOAD, P _g	30 PSF
FLAT-ROOF SNOW LOAD, P _f	25.2 PSF
SNOW EXPOSURE FACTOR, C _e	1.0
SNOW LOAD IMPORTANCE FACTOR, I _s	1.2
TEMPERATURE FACTOR, C _t	1.0
SLOPE FACTOR(S), C _s	1.0
DRIFT SURCHARGE LOADS, P _d AND WIDTH OF SNOW DRIFTS, W, REFER TO \$2.04 .	

WIND DESIGN DATA:

BASIC DESIGN WIND SPEED (3 SECOND GUST), V ₁	128 MPH
ALLOWABLE STRESS DESIGN WIND SPEED, V ₁₀	77 MPH
RISK CATEGORY:	IV
WIND EXPOSURE CATEGORY:	B
COORDINATE COMPONENTS AND CLADDING WIND PRESSURE, p, 27 PSF	+/- 0.18

EARTHQUAKE DESIGN DATA:

RISK CATEGORY:	IV
SEISMIC IMPORTANCE FACTOR, I _e	1.0
MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS:	
SITE CLASS:	S ₁ = 0.184
DESIGN SPECTRAL RESPONSE ACCELERATION:	S ₁ = 0.048
PARAMETERS:	S ₂ = 0.196
SEISMIC DESIGN CATEGORY:	S ₂ = 0.077
BASIC SEISMIC FORCE-RESISTING SYSTEM(S):	C
	ORDINARY MOMENT RESISTING FRAME

DESIGN BASE SHEAR(S):

21 KIPS

SEISMIC RESPONSE COEFFICIENT(S), C_s:

0.098

RESPONSE MODIFICATION FACTOR(S), R:

3 (STEEL)

ANALYSIS PROCEDURE USED:

EQUVALENT LATERAL FORCE ANALYSIS

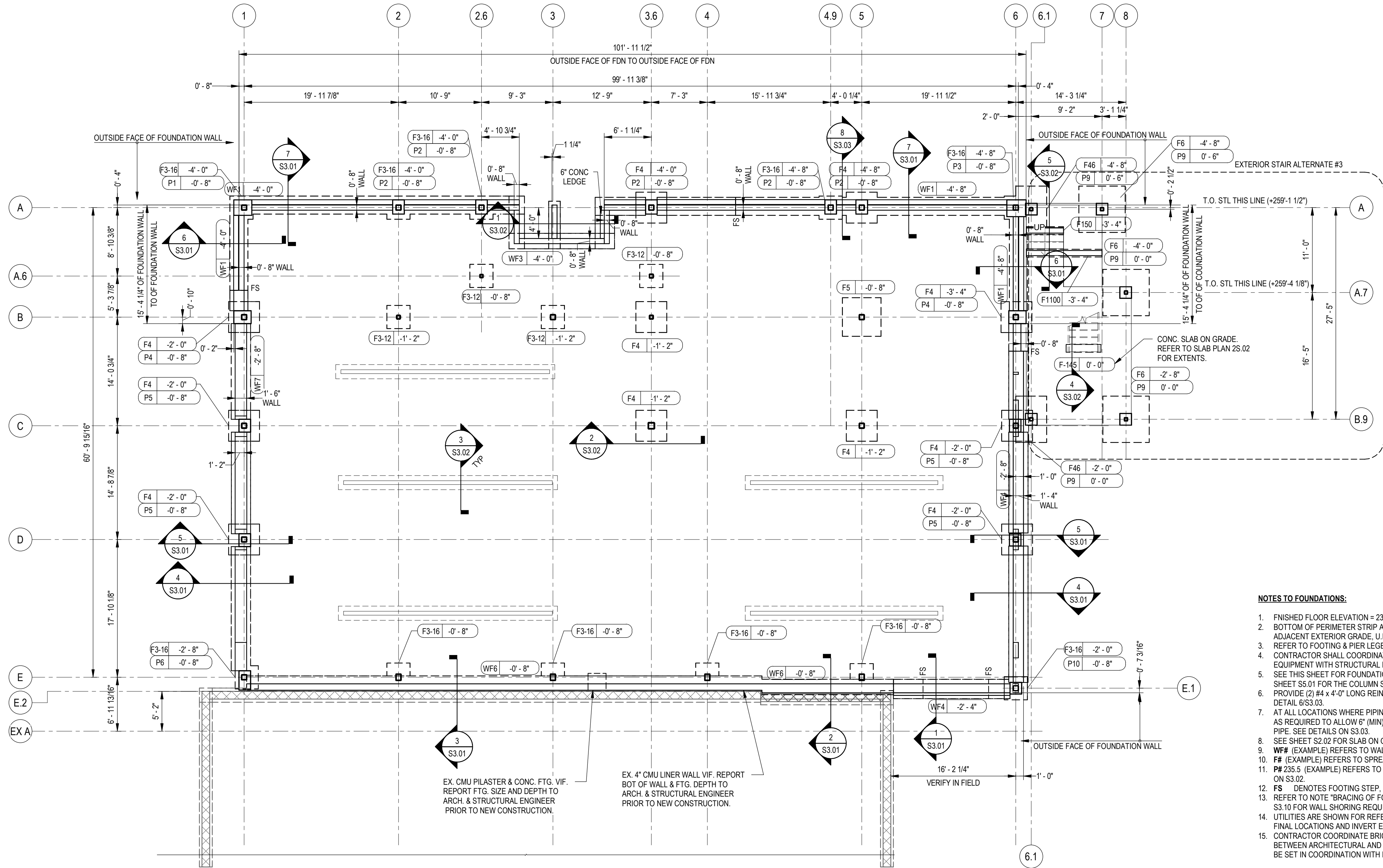
GEOTECHNICAL INFORMATION:

DESIGN ALLOWABLE SOIL BEARING PRESSURE:	4,000 PSF
MOIST UNIT WEIGHT OF BACKFILL:	125 PCF
INTERNAL FRICTION ANGLE OF BACKFILL:	34 DEGREES
COEFFICIENT OF "AT-REST" LATERAL EARTH PRESSURE:	0.56
COEFFICIENT OF "ACTIVE" LATERAL EARTH PRESSURE:	0.28
COEFFICIENT OF "PASSIVE" LATERAL EARTH PRESSURE:	3.54
MINIMUM SAFETY FACTOR AGAINST SLIDING AND OVERTURNING:	2.0

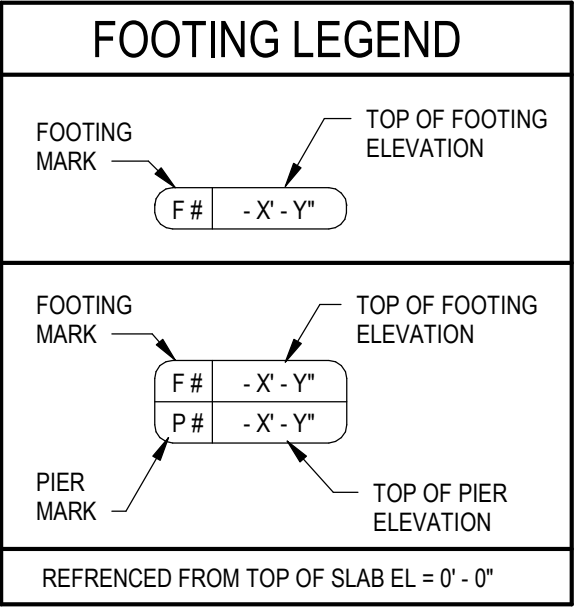
THIS BUILDINGS NOT DESIGNED FOR FLOOD LOADS.

ABBREVIATIONS

AT	A	MAX	MAXIMUM	M
ADDL ADDITIONAL	A	MECH	MECHANICAL	M
ADH ADHESIVE	A	MEP	MECHANICAL - ELECTRICAL - PLUMBING	M
ADJ ADJACENT	A	MFR	MANUFACTURER	M
AFT ABOVE FINISHED FLOOR	A	MIN	MINIMUM	M
ANCH ANCHOR	A	MO	MASONRY OPENING	M
ALT ALTERNATE	A	MTL	METAL	M
ALUM ALUMINUM	A			
APPROX APPROXIMATE	A			
AR ANCHOR ROD	A	NIC	NOT IN CONTRACT	N
ARCH ARCHITECT	A	NOM	NOMINAL	N
		NTS	NOT TO SCALE	N
B/B BACK TO BACK	B	NWT	NORMAL WEIGHT	N
BEV BEVEL	B			
BLDG BUILDING	B	OC	ON CENTER	O
BLKG BLOCKING	B	OD	OUTSIDE DIAMETER	O
BO BOTTOM OF	B	OF	OUTSIDE FACE	O
BOS BOTTOM OF STEEL	B	OPNG	OPENING	O
BOT BOTTOM	B	OPF	OPPOSITE	O
BP BASE PLATE	B			
BRDG BRIDGING	B	PAF	POWDER ACTUATED FASTENER	P
BRG BEARING	B	POC	PRECAST CONCRETE	P
BSMT BASEMENT	B	PEMB	PRE ENGINEERED METAL BUILDING	P
CANTL CANTILEVER	C	PL	PLATE	P
CFMF COLD FORMED METAL FRAMING	C	PLAT	PLATFORM	P
CHFR CHAMFER	C	PLF	POUNDS PER LINEAR FOOT	P
CJ CONTROL JOINT	C	PREFAB	PREFABRICATED	P
C/P COMPLETE JOINT PENETRATION	C	PRELIM	PRELIMINARY	P
CL CENTER LINE	C	PSF	POUNDS PER SQUARE FOOT	P
CLKJ CULKED JOINT	C	PT	PRESSURE TREATED	P
CLR CLEAR	C	PUR	PURLINS	P
CMU CONCRETE MASONRY UNIT	C	PVMT	PAVEMENT	P
CO CLEAN OUT	C			
COL COLUMN	C	QC	QUALITY CONTROL	Q
CONC CONCRETE	C			
CONN CONNECTION	C	R	RADIUS	R
CONST JT CONSTRUCTION JOINT	C	RD	ROOF DRAIN	R
CONT CONTINUOUS	C	RIG INS	RIGID INSULATION	R
COORD COORDINATE	C	REF	REFERENCE	R
CSK COUNTER SUNK	C	REINF	REINFORCING	R
CU FT CUBIC FEET	C	REM	REMAINDER	R
CU YD CUBIC YARD	C	RECD	REQUIRED	R
		RETAIN	RETAINING	R
		RH	ROOF HATCH	R
		RO	ROUGH OPENING	R
DBL DOUBLE	D			
DEMO DEMOLITION	D			
DET DETAIL	D			
DIA DIAMETER	D	SCHED	SCHEDULE	S
DM DIMENSION	D	SF	SQUARE FEET	S
DL DEAD LOAD	D	SIM	SIMILAR	S
DN DOWN	D	SOG	SLAB ON GROUND	S
DWG DRAWING	D	SPA	SPACE(S)	S
		SPEC	SPECIFICATION	S
EA EACH	E	SQ	SQUARE	S
EE EACH END	E	SST	STAINLESS STEEL	S
EF EACH FACE	E	STA	STATION	S
EJ EXPANSION JOINT	E	STD	STANDARD	S
EL ELEVATION	E	STL	STEEL	S
ELEC ELECTRICAL	E			
EOD EDGE OF DECK	E	T&B	TOP AND BOTTOM	T
EOS EDGE OF SLAB	E	THK	TRENCH DRAIN	T
EPS EXPANDED POLYSTYRENE	E	TEMP	TEMPORARY	T
EQ EQUAL	E	THK	THICKNESS	T
EQUIP EQUIPMENT	E	THRU	THROUGH	T
EW EACH WAY	E	TO	TOP OF	T
EX EXIST	E	TOC	TOP OF CONCRETE	T
EXP EXISTING	E	TOS	TOP OF STEEL	T
EXT EXTENSION	E	TOSL	TOP OF SLAB	T
		TOW	TOP OF WALL	T
		TYP	TYPICAL	T
FAB FABRICATE	F			
FBO FURNISHED BY OTHERS	F	UNEX	UNEXCAVATED	U
FD FLOOR DRAIN	F	UNO	UNLESS NOTED OTHERWISE	U
FDTN FOUNDATION	F			
FOM FLOOR	F	VERT	VERTICAL	V
FOR FACE OF MASONRY	F	VIF	VERIFY IN FIELD	V
FOW FACE OF WALL	F	VR	VAPOR RETARDER	V
FRMG FRAMING	F			
FS FOOTING STEP	F	W	WITH	W
FT FOOT	F	WO	WITHOUT	W
FTG FOOTING	F	WBL	WOOD BLOCKING	W
FW FIRE WALL	F	WF	WALL FOOTING	W
		WP	WORKING POINT	W
GA GAGE	G	WS	WALL STEP	W
GALV GALVANIZED	G	WWF	WELDED WIRE FABRIC	W
GC GENERAL CONTRACTOR	G			
GDR GUARD RAIL	G			
GR BM GRADE BEAM	G	X BRACE	CROSS BRACE	X
		XS	EXTRA STRONG	X
		XXS	DOUBLE EXTRA STRONG	X
HNDRL HANDRAIL	H			
HORIZ HORIZONTAL	H			
HP HIGH POINT	H			
	I			
ISD INSIDE DIAMETER	I			
IF INSIDE FACE	I			
IN INCHES	I			
INFO INFORMATION	I			
INSUL INSULATION	I			
INV INVERT	I			
ISO ISOLATION JOINT	I			
	J			
JBE JOIST BEARING ELEVATION	J			
	K			
K KIP FEET	K			
K-FT KIP FEET	K			
KB KNEE BRACE	K			
KO KNOCKOUT	K			
KPL KICK PLATE	K			
KSF KIPS PER SQUARE FOOT	K			
KWY KEY WAY	K			
	L			
LBS POUNDS	L			
LD BRG LOAD BEARING	L			
LG LONG	L			
LL LIVE LOAD	L			
LEBB LONG LEG BACK TO BACK	L			
LLH LONG LEG HORIZONTAL	L			
LLV LONG LEG VERTICAL	L			
LNTL LENTIL	L			
LONG LONGITUDINAL	L			
LP LOW POINT	L			
LWT LIGHTWEIGHT	L			



FOOTING SCHEDULE			
MARK	SIZE (LxWxTHICKNESS)	REINFORCING	REMARKS
F3-12	3'-0" x 3'-0" x 1'-0"	(5) - #5 EW BOT	
F3-16	3'-0" x 3'-0" x 1'-4"	(5) - #5 EW BOT	
F4	4'-0" x 4'-0" x 1'-4"	(5) - #5 EW BOT	
F5	5'-0" x 5'-0" x 1'-4"	(7) - #5 EW BOT	
F6	6'-0" x 6'-0" x 1'-6"	(9) - #5 EW BOT	
F46	4'-0" x 6'-0" x 1'-6"	(9) - #5 SW BOT (6) - #5 LW BOT	
F150	1'-0" x 5'-0" (F150)	(3) - #4 LW (7) - #4 SW	
F1100	1'-0" x 10'-0" (F1100)	(3) - #4 LW (14) - #4 SW	
F-145	1'-0" x 4'-6" x 3'-2"	#5 @ 12" VERT. (4) #4 LONGWAY	
WF1	2'-4" x 1'-4"		
WF2	2'-4" x 1'-0"		
WF3	2'-0" x 1'-0"		
WF4	2'-0" x 1'-4"		
WF6	2'-0" x 1'-7 1/2" CUSTOM		
WF7	2'-6" x 1'-0"		



PIER SCHEDULE									
MARK	SIZE			TYPE	REINFORCEMENT		ANCHOR BOLT		REMARKS
	"A"	"B"	"C"		VERT	TIES	TYPE	QTY	
P1	22'	18'	10'	T2	8-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	D=10"
P2	18'	20'	10'	T1	8-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	
P3	20'	30'	10'	T2	10-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	D=10"
P4	22'	20'	10'	T1	8-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	
P5	18'	20'	10'	T1	8-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	
P6	18'	20'	10'	T1	8-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	
P7	38'	24'	17'	T1	16-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	
P8	43'	24'	17'	T1	18-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	
P9	18'	18'	10'	T3	6-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	
P10	18'	18'	10'	T2	8-#7	#4 TIES @ 8" O.C.	SEE COLUMN SCHEDULE	SEE COLUMN SCHEDULE	D=10"

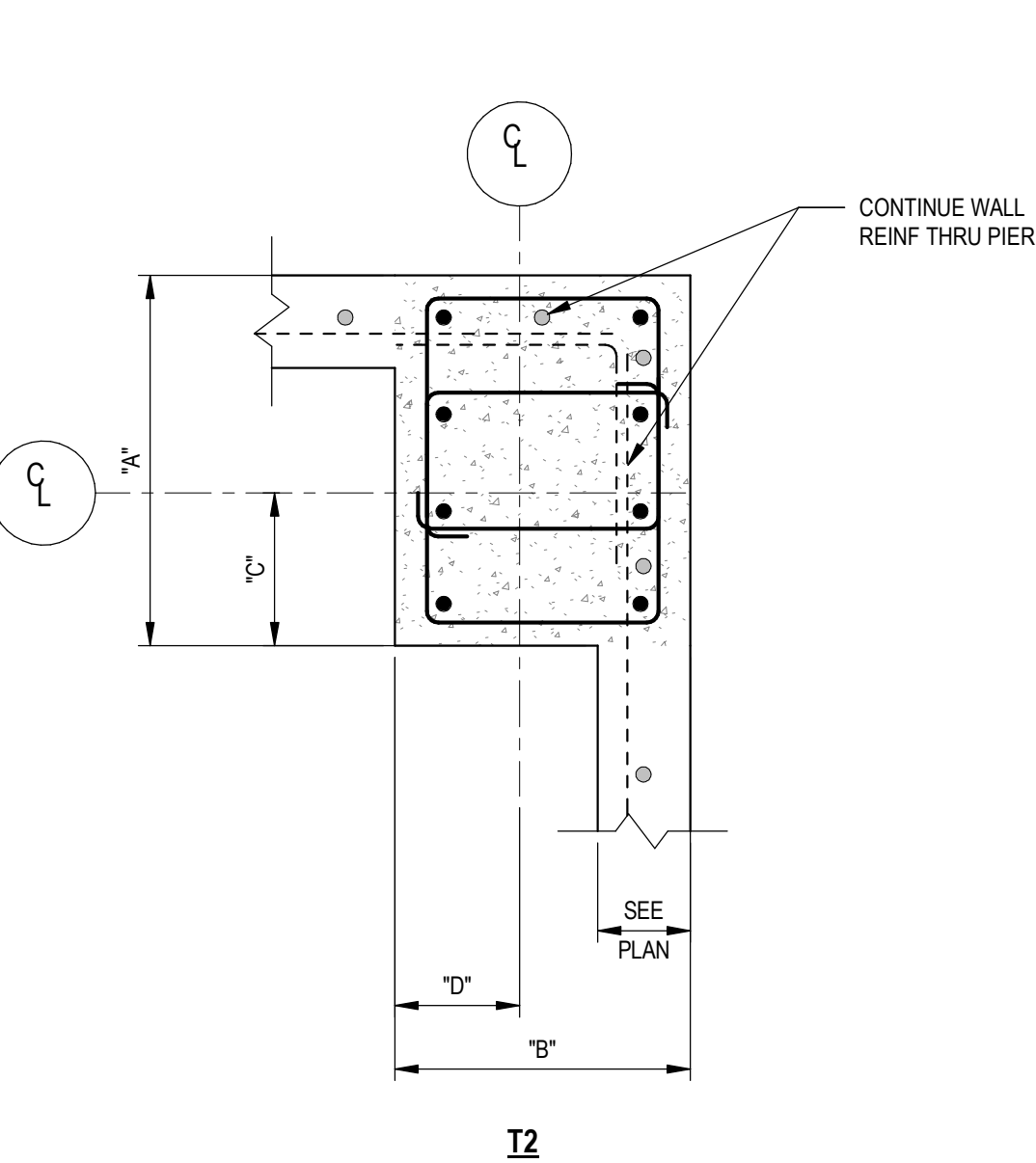
- NOTES:
- DIMENSIONS "A" AND "B" REPRESENTS LIMIT OF PIER REINFORCING.
 - COORDINATE DIMENSIONS "A" AND "B" WITH COLUMN SCHEDULE ON DRAWING S5.01 AND BASE PLATE DIMENSIONS TO MAINTAIN A MINIMUM OF 2" DIMENSION FROM EDGE OF BASE PLATE TO EDGE OF PIER, VERIFY DIMENSIONS PRIOR TO FABRICATION OF REINFORCING AND CONSTRUCTION OF PIERS.
 - REINFORCE TOP OF PIER WITH ADDITIONAL TIES @ 3" OC FULL DEPTH OF ANCHOR RODS.

- NOTES TO FOUNDATIONS:
- FINISHED FLOOR ELEVATION = 235.5' REFERENCE FINISHED FLOOR ELEVATION = 0'-0"
 - BOTTOM OF PERIMETER STRIP AND SPREAD FOOTINGS TO BE (-) 3'-0" (MIN.) FROM FINISHED ADJACENT EXTERIOR GRADE, U.N.O.
 - REFER TO FOOTING & PIER LEGEND THIS DRAWING FOR TOP OF FOOTING & PIER NOTATIONS.
 - CONTRACTOR SHALL COORDINATE ALL VENDOR REQUIREMENTS FOR SPECIALIZED EQUIPMENT WITH STRUCTURAL ENGINEER PRIOR TO PLACING CONCRETE.
 - SEE THIS SHEET FOR FOUNDATION NOTES. SEE SHEET S0.01 FOR GENERAL NOTES AND SHEET S5.01 FOR THE COLUMN SCHEDULE.
 - PROVIDE (2) #4 x 4'-0" LONG REINFORCING BARS AT ALL RE-ENTRANT SLAB CORNERS. SEE DETAIL 6S3.03.
 - AT ALL LOCATIONS WHERE PIPING RUNS THROUGH FOUNDATION WALLS, DEEPEN FOOTINGS AS REQUIRED TO ALLOW 6" (MIN.) CLEARANCE BETWEEN PIPE AND TOP OF FOOTING. SLEEVE PIPE. SEE DETAILS ON S3.03.
 - SEE SHEET S2.02 FOR SLAB ON GRADE PLAN.
 - WF# (EXAMPLE) REFERS TO WALL FOOTING MARK. SEE SCHEDULE ON THIS SHEET.
 - F# (EXAMPLE) REFERS TO SPREAD FOOTING MARK. SEE SCHEDULE ON THIS SHEET.
 - P# 235.5 (EXAMPLE) REFERS TO PIER TYPE AND TOP OF PIER ELEVATION. SEE PIER DETAIL ON S3.02.
 - FS# DENOTES FOOTING STEP. SEE DETAIL ON S3.02.
 - REFER TO NOTE "BRACING OF FOUNDATION WALLS PRIOR TO BACKFILLING" ON DRAWING S3.10 FOR WALL SHORING REQUIREMENTS AT FOUNDATION AND RETAINING WALLS.
 - UTILITIES ARE SHOWN FOR REFERENCE ONLY. REFER TO SPECIFIC MEP DRAWINGS FOR FINAL LOCATIONS AND INVERT ELEVATIONS.
 - CONTRACTOR COORDINATE BRICK LEDGE ELEVATIONS AT PERIMETER OF BUILDING BETWEEN ARCHITECTURAL AND CIVIL DRAWINGS. FINAL BRICK LEDGE ELEVATIONS ARE TO BE SET IN COORDINATION WITH FINAL SITE GRADING.

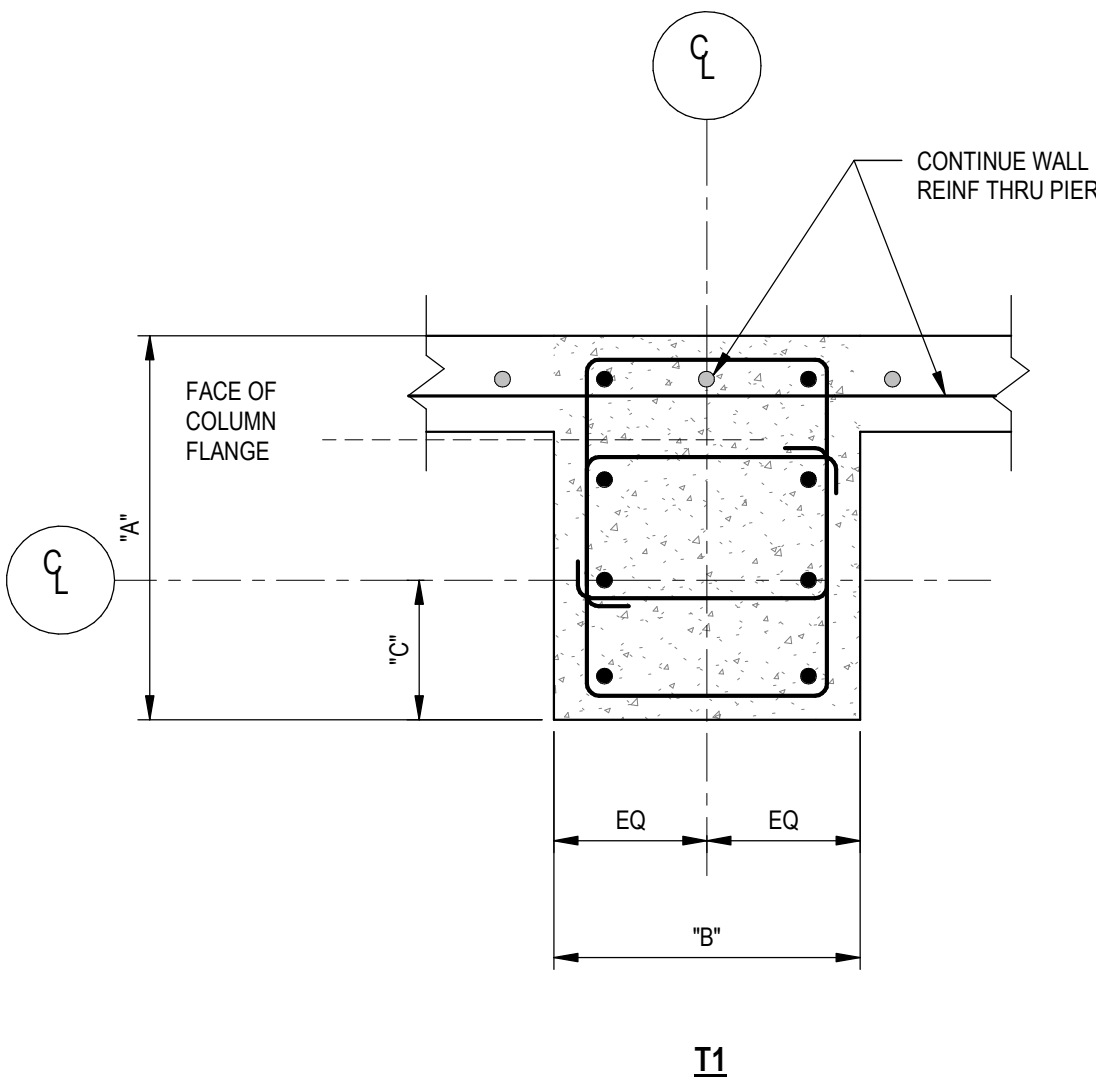
NOTE: ARCHITECTURAL PLAN. BACKGROUND INFORMATION IS SHOWN FOR REFERENCE AND COORDINATION ONLY. THE CONTRACTOR SHALL VERIFY ALL BACKGROUND DIMENSIONS, BUILDING COMPONENTS AND CONDITIONS WITH THE FINAL SET OF ARCHITECTURAL DRAWINGS ISSUED FOR CONSTRUCTION.

FOUNDATION PLAN

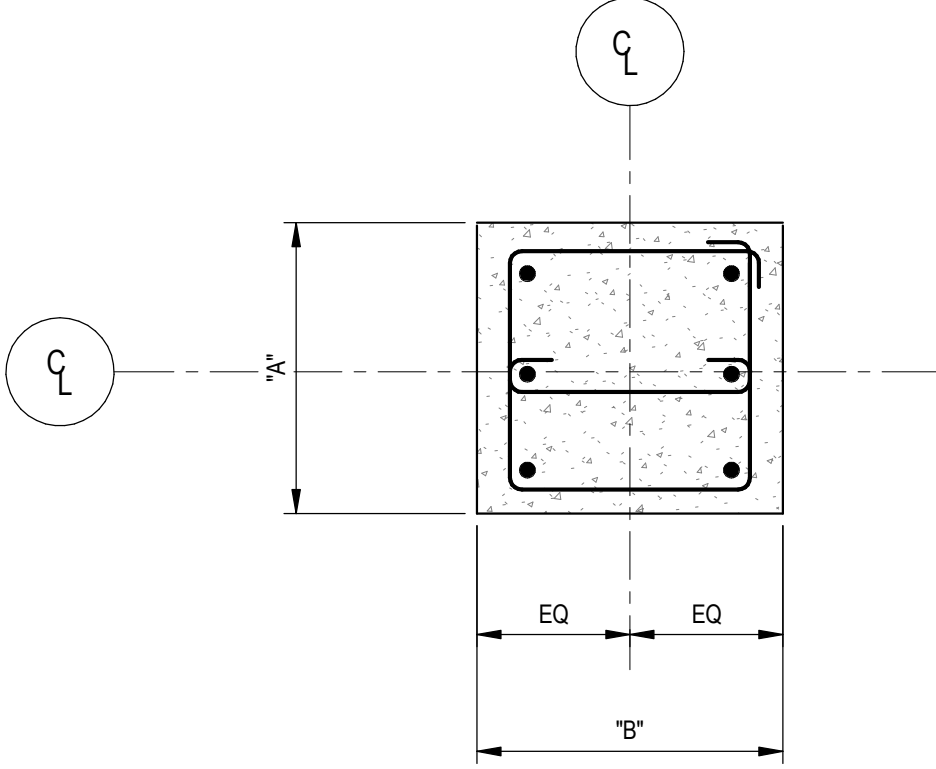
1/8" = 1'-0"



PIER TYPE T2
NOT TO SCALE



PIER TYPE T1
NOT TO SCALE



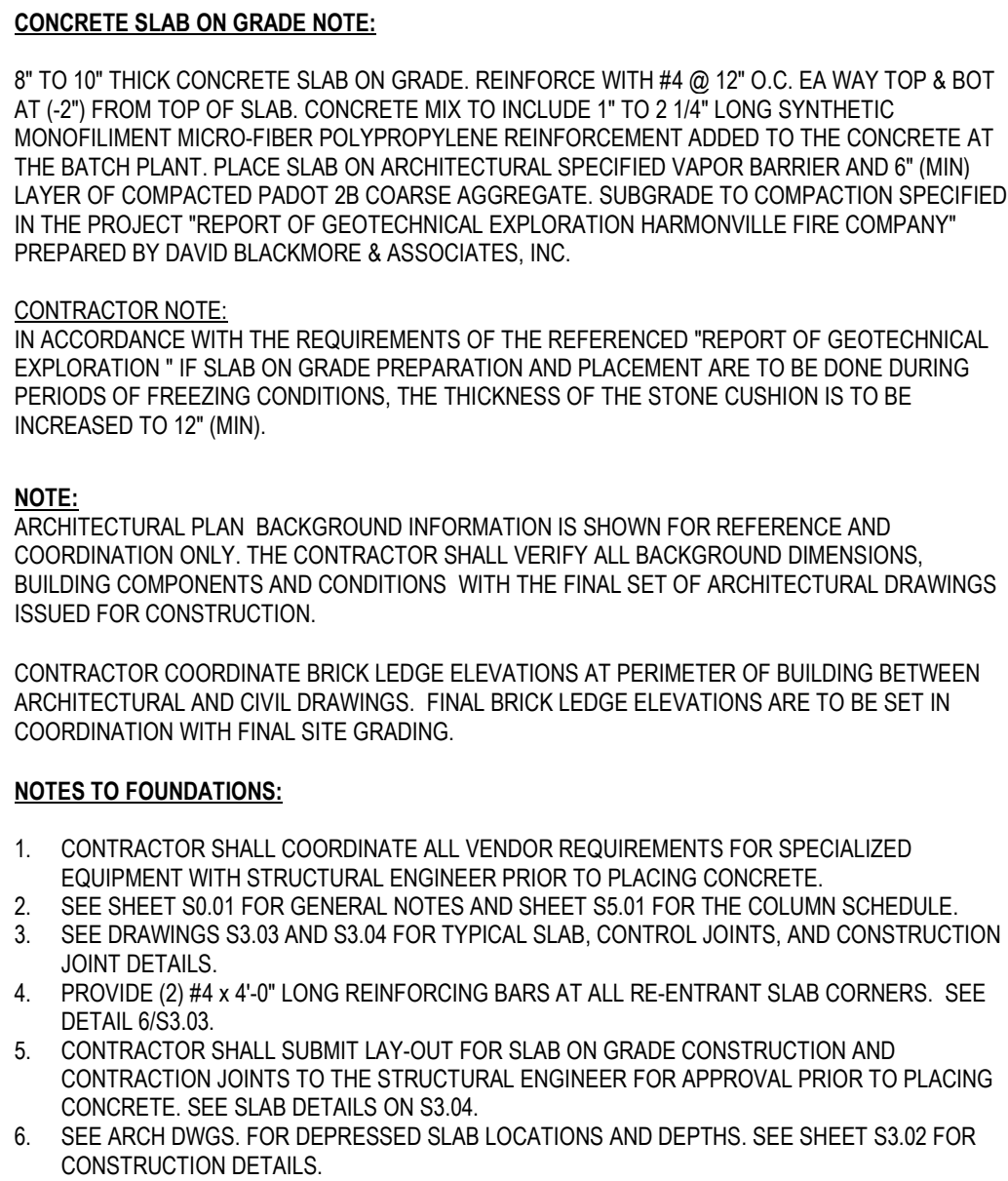
PIER TYPE T3
NOT TO SCALE

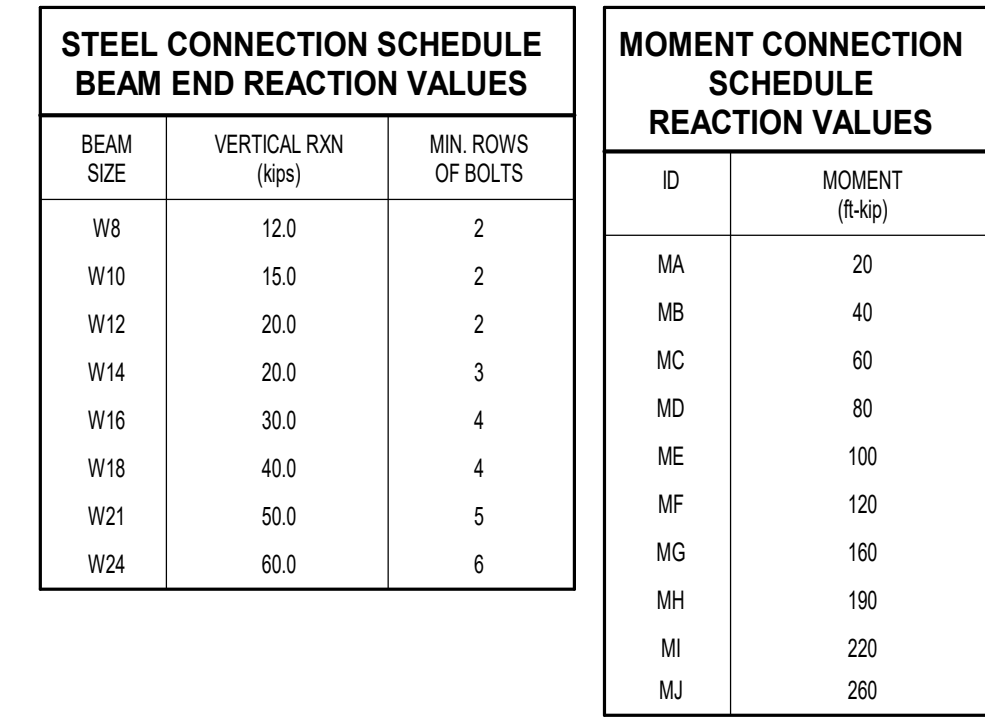
REVISIONS	MARK	DATE	DESCRIPTION

SHEET TITLE

FOUNDATION	JOB NO.	91242
	DATE	03/06/2025
	SHEET NO.	

S2.01

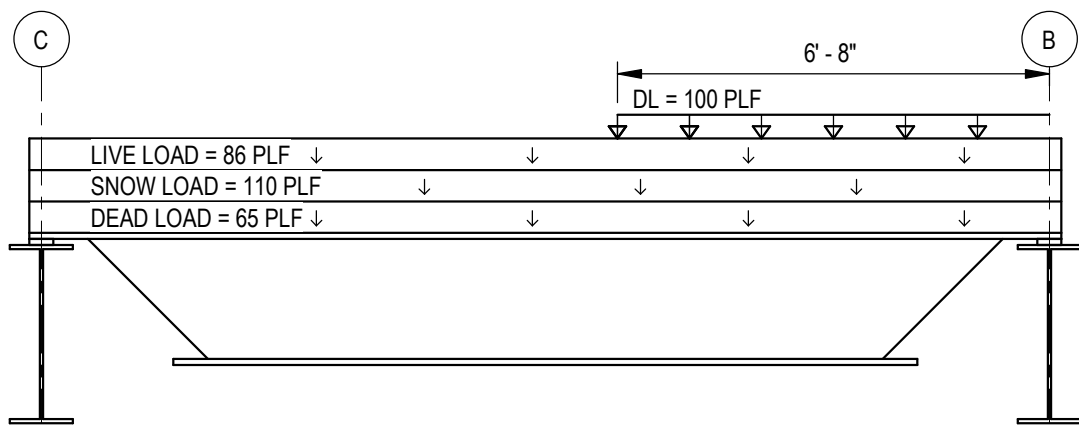




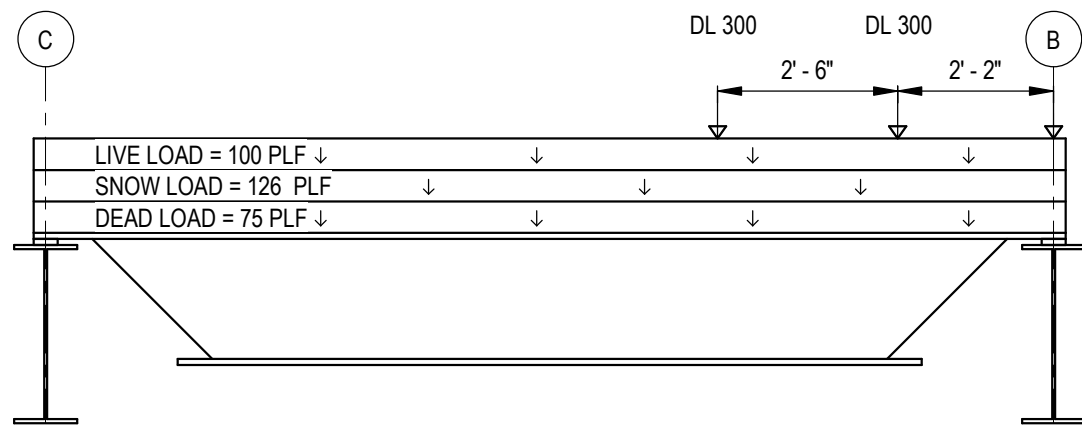
Architectural floor plan of the 1st floor of a building. The plan shows a rectangular layout with various rooms, corridors, and structural elements. Key features include:

- Grid Lines:** Horizontal (1-8) and Vertical (A-E).
- Dimensions:** Overall dimensions of 99'-11 3/8" by 144'-0 1/4". Room dimensions and offsets are provided throughout.
- Rooms and Areas:** Labeled with codes like MB, MA, MD, MC, ME, MF, MH, MI, MJ, MK, ML, MN, MO, MP, MQ, MR, MS, MT, MU, MV, MW, MX, MY, MZ, and various 'V' and 'W' codes.
- Structural Elements:** Columns (C), beams (B), and walls (W) are indicated.
- Stairs:** Stair openings and alternate stairs are shown.
- Exterior:** Outside face of the building is indicated on the left and bottom.
- Notes:** Various notes and callouts are present, including "SS.04" and "E.O.S."

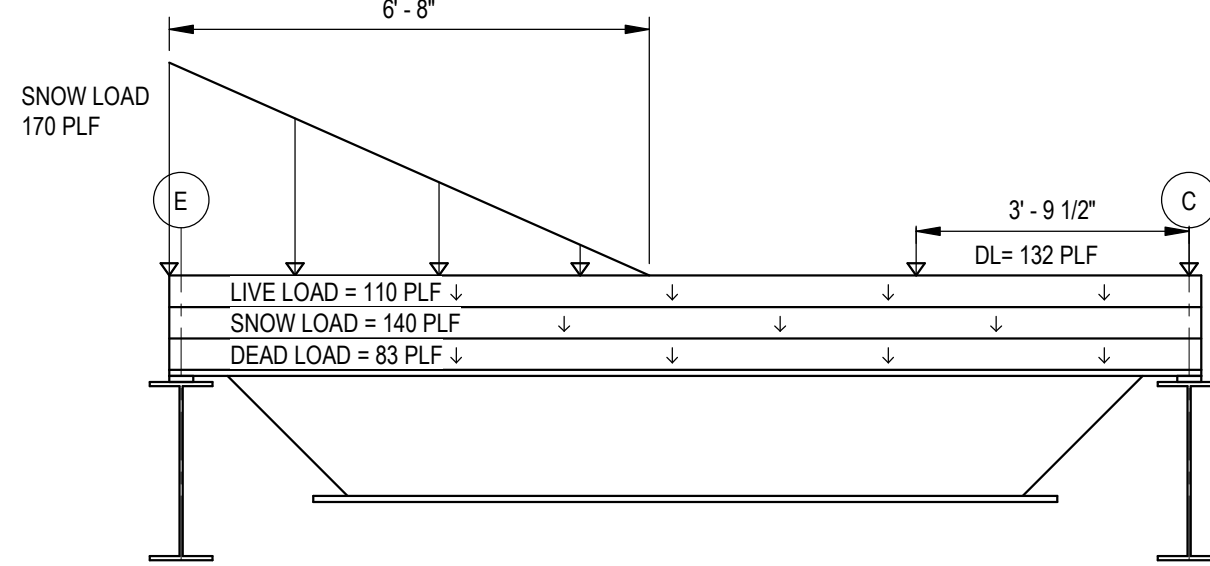
1 SECOND FLOOR FRAMING PLAN



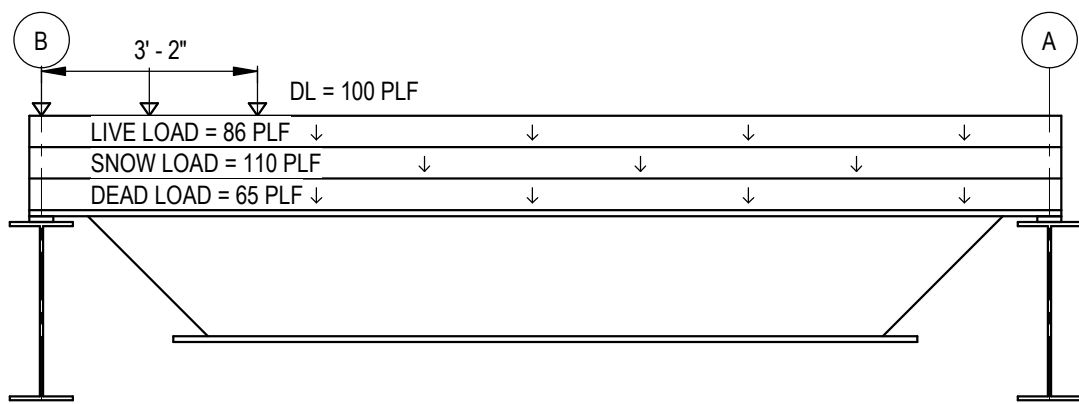
2 SPECIAL JOIST LOADING DIAGRAM 10KSP1
SCALE: 3/8" = 1'-0"



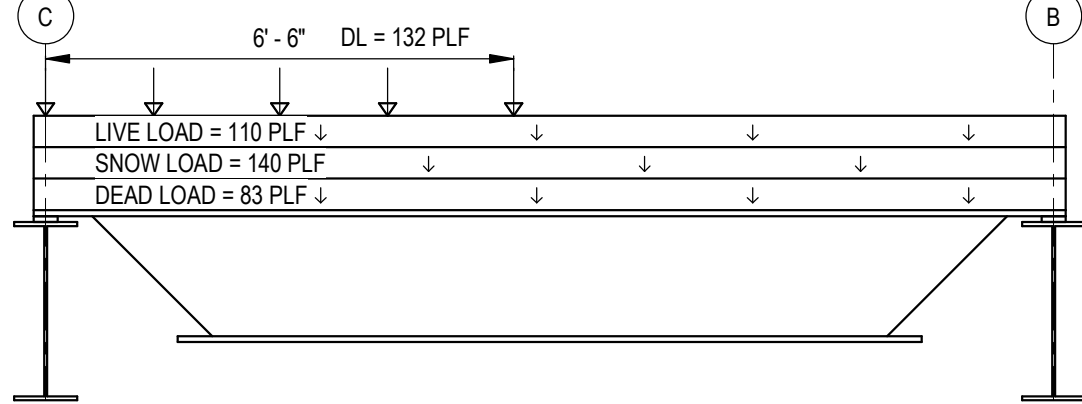
5 SPECIAL JOIST LOADING DIAGRAM 10KSP4
SCALE: 3/8" = 1'-0"



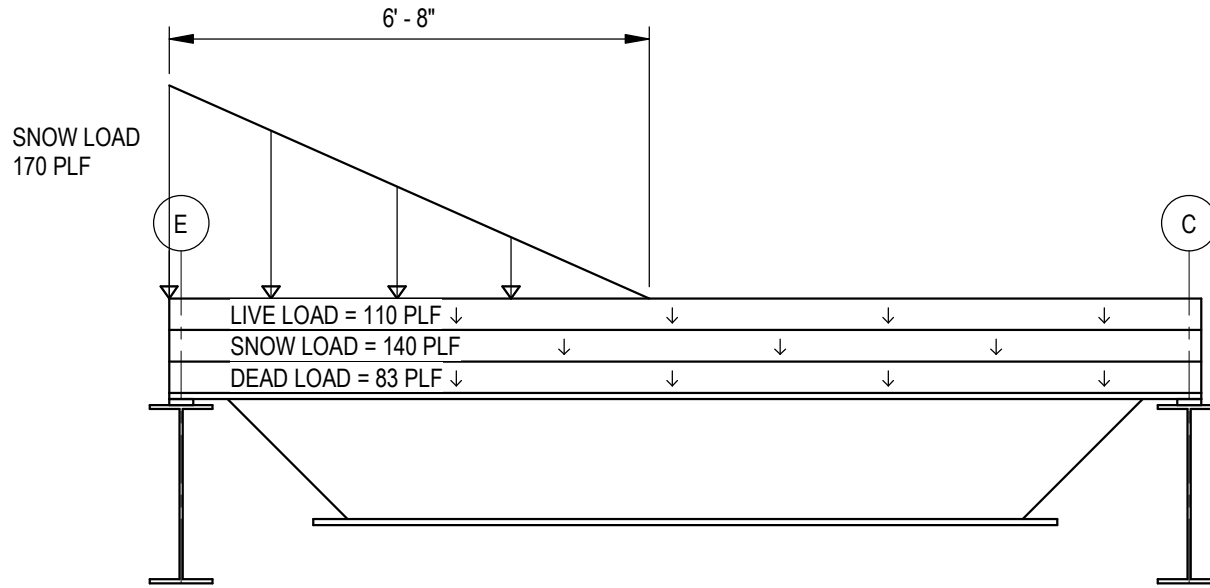
6 SPECIAL JOIST LOADING DIAGRAM 24KSP5
SCALE: 3/8" = 1'-0"



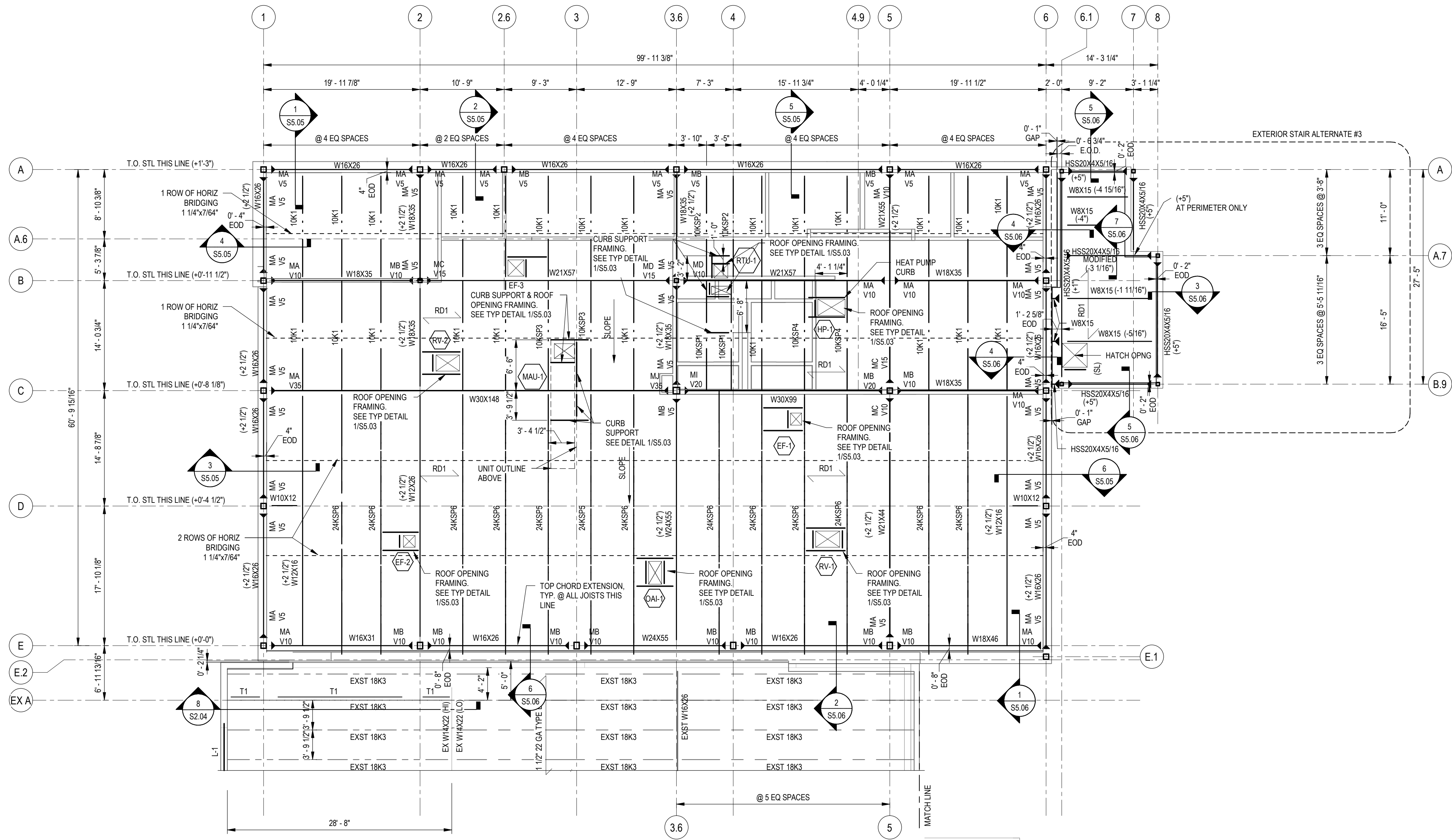
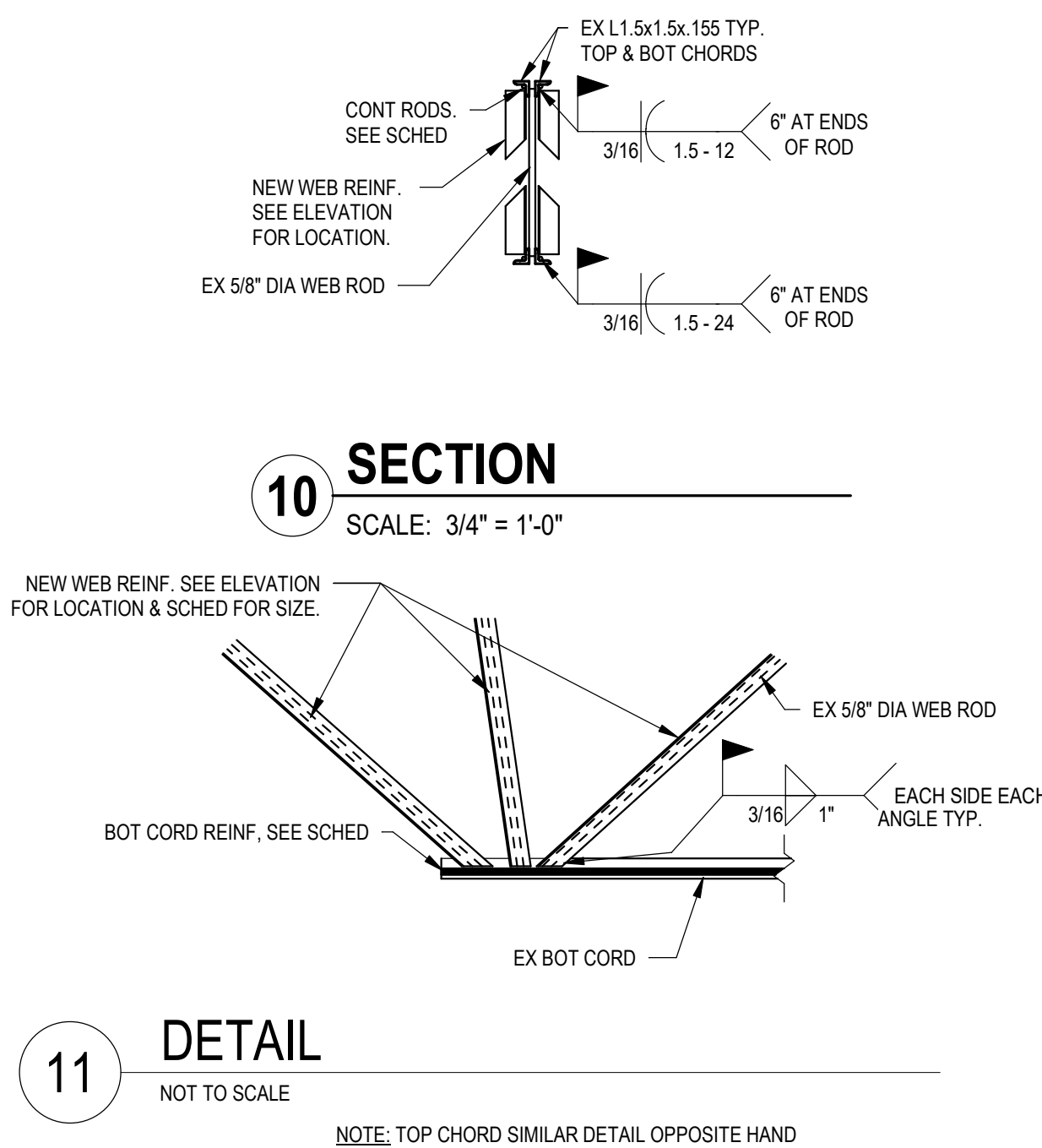
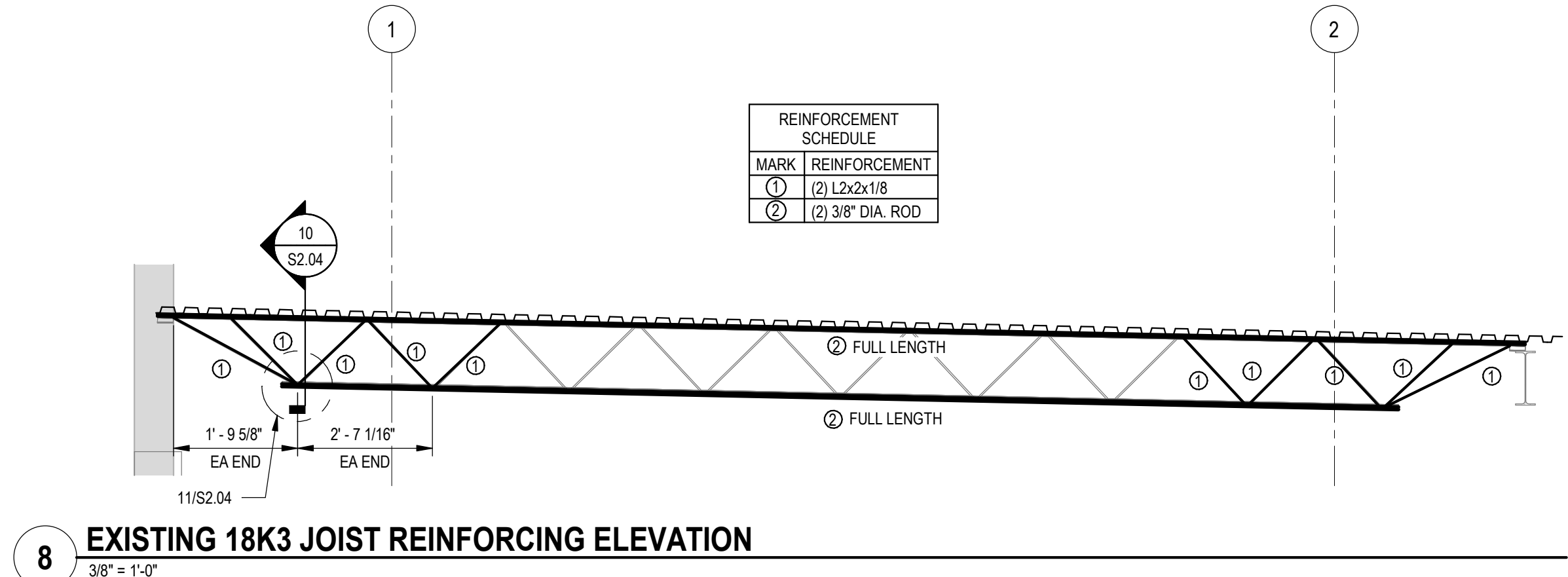
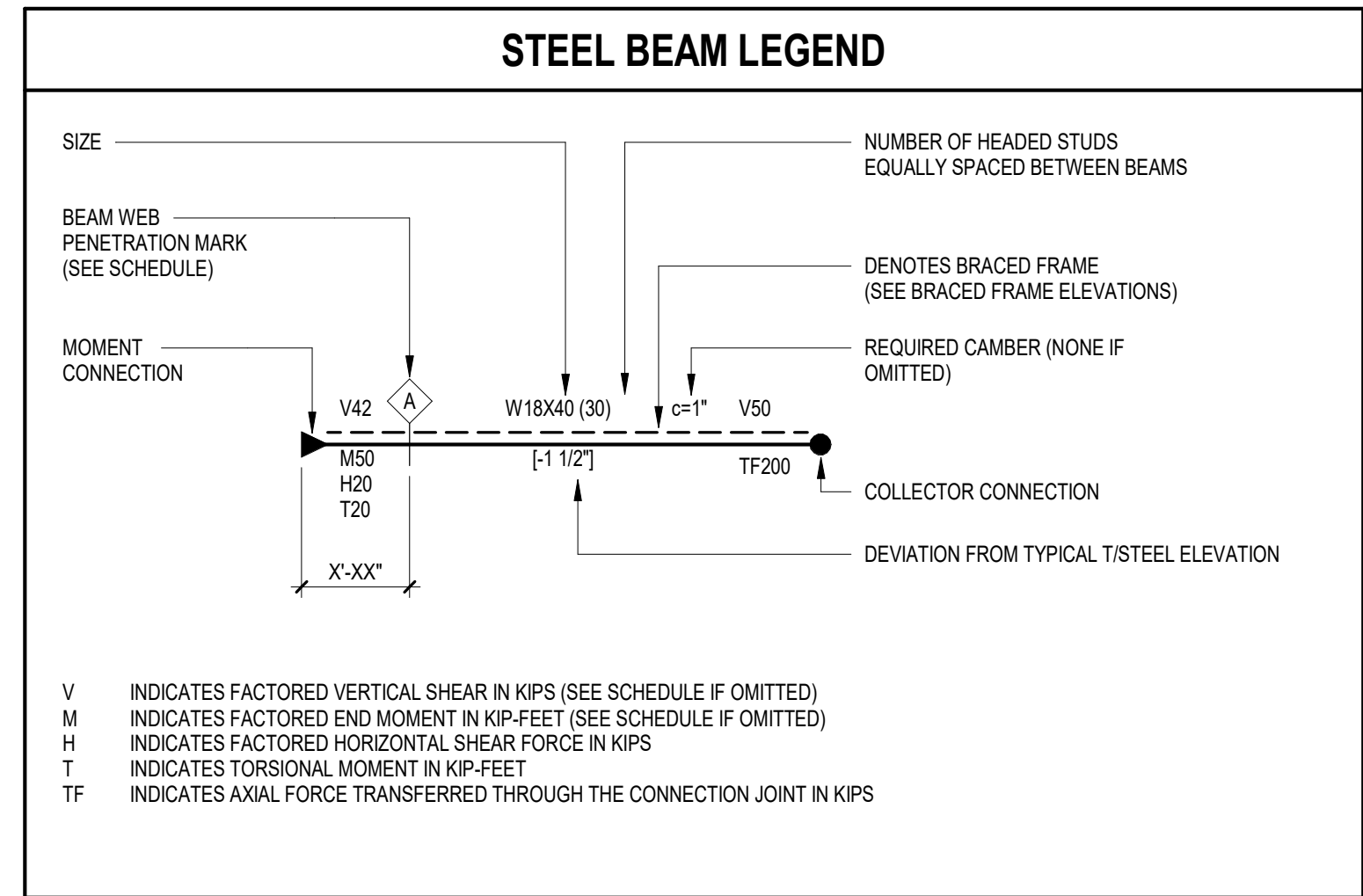
3 SPECIAL JOIST LOADING DIAGRAM 10KSP2
SCALE: 3/8" = 1'-0"



4 SPECIAL JOIST LOADING DIAGRAM 10KSP3
SCALE: 3/8" = 1'-0"



7 SPECIAL JOIST LOADING DIAGRAM 24KSP6
SCALE: 3/8" = 1'-0"



1 ROOF FRAMING PLAN
1/8" = 1'-0"

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ROOF FRAMING PLAN

JOB NO. 91242
DATE 03/06/2025
SHEET NO.

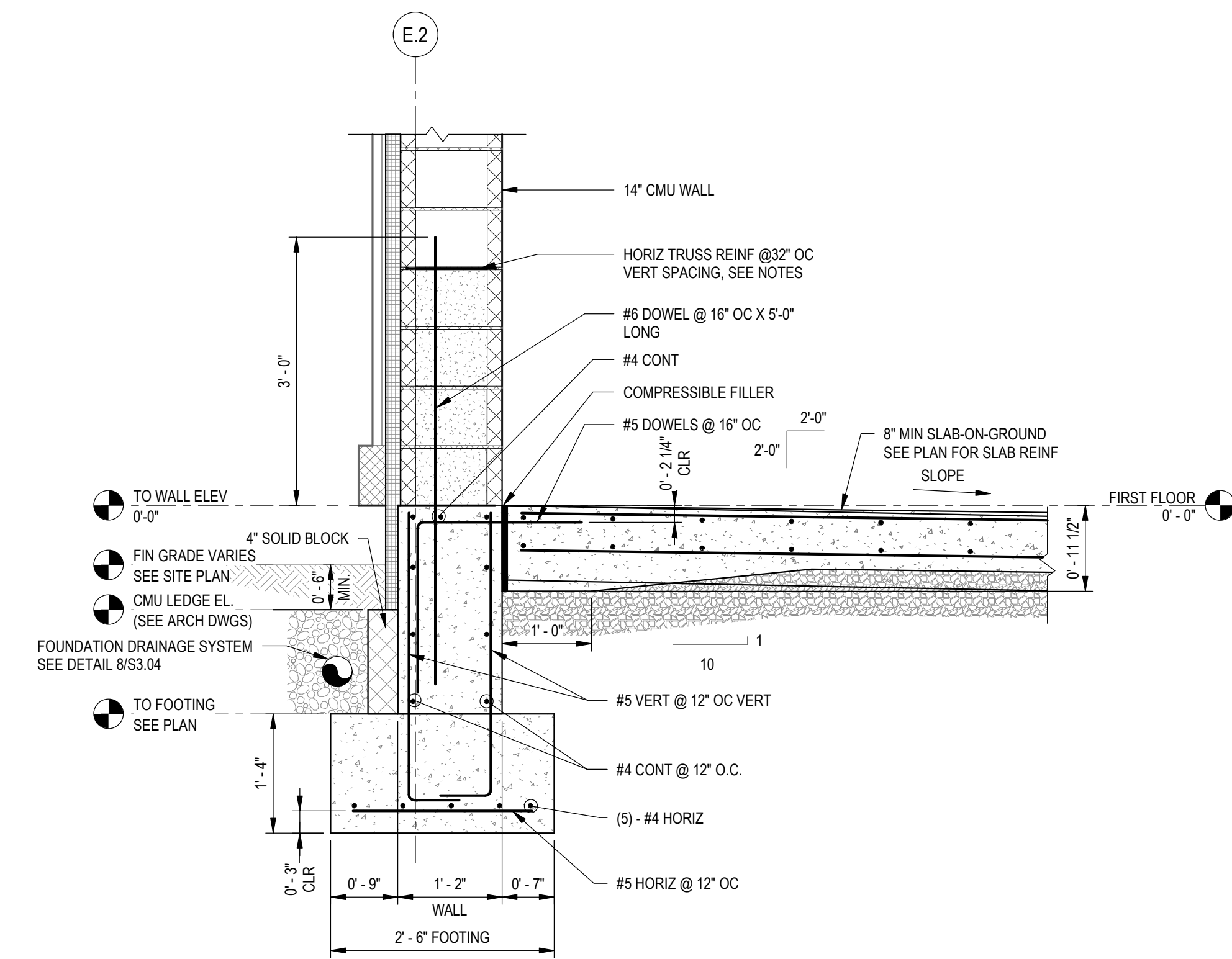
S2.04

NEW FIRE STATION
FOR THE
THE HARMONVILLE FIRE COMPANY
TOWNSHIP OF PLYMOUTH MONTGOMERY COUNTY, PA

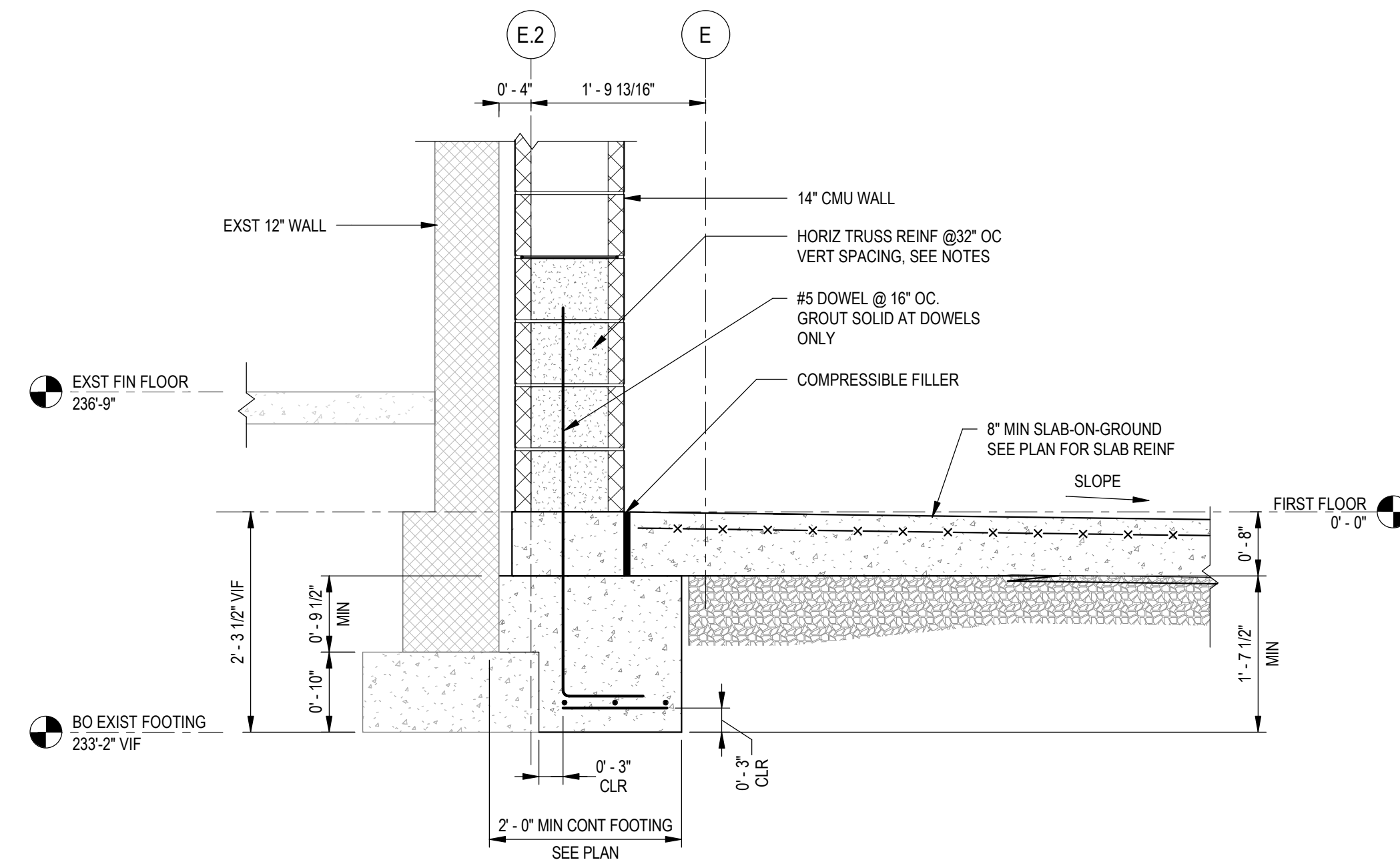
REVISIONS	DATE	DESCRIPTION

JOB NO. 91242
DATE 03/06/2025
SHEET NO.

S2.04

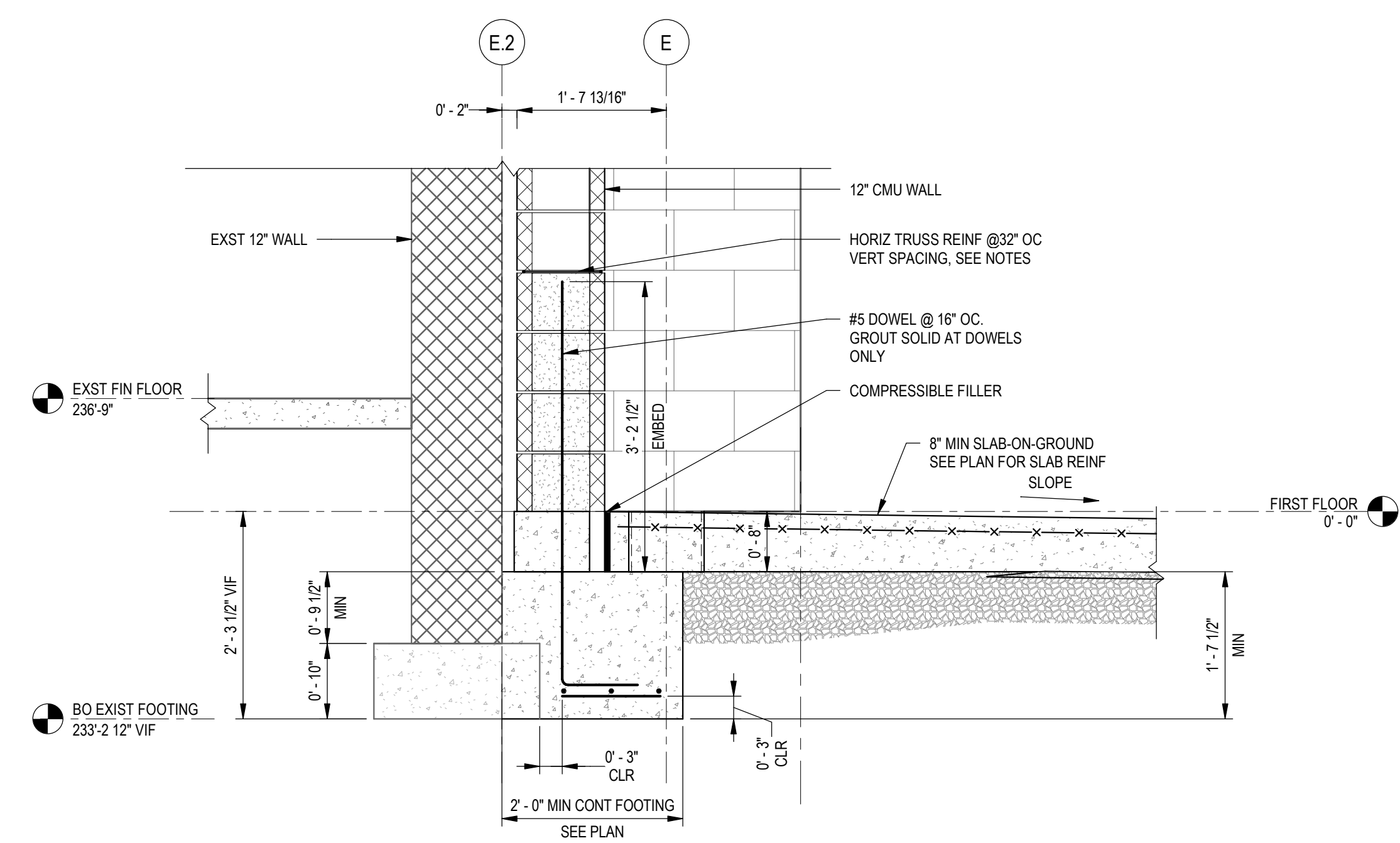


1 FOUNDATION SECTION



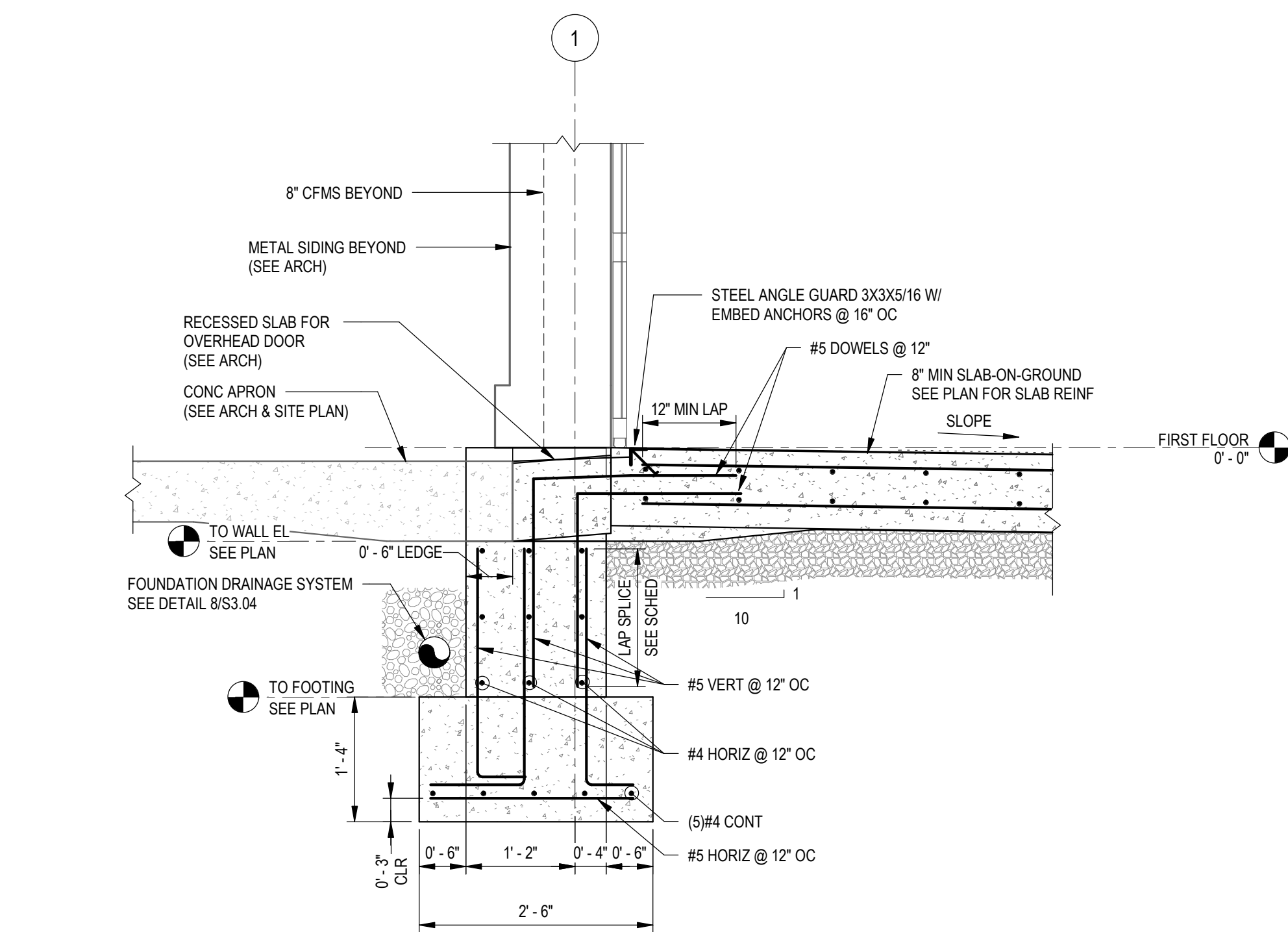
NOTE: BOTTOM OF FOOTING TO MATCH EXISTING. VERIFY IN FIELD AND NOTIFY ENGINEER IF 1'-4" THICK IS NOT AVAILABLE. UNDERPINNING MAY BE REQUIRED.

2 FOUNDATION TO EXISTING FOUNDATION SECTION

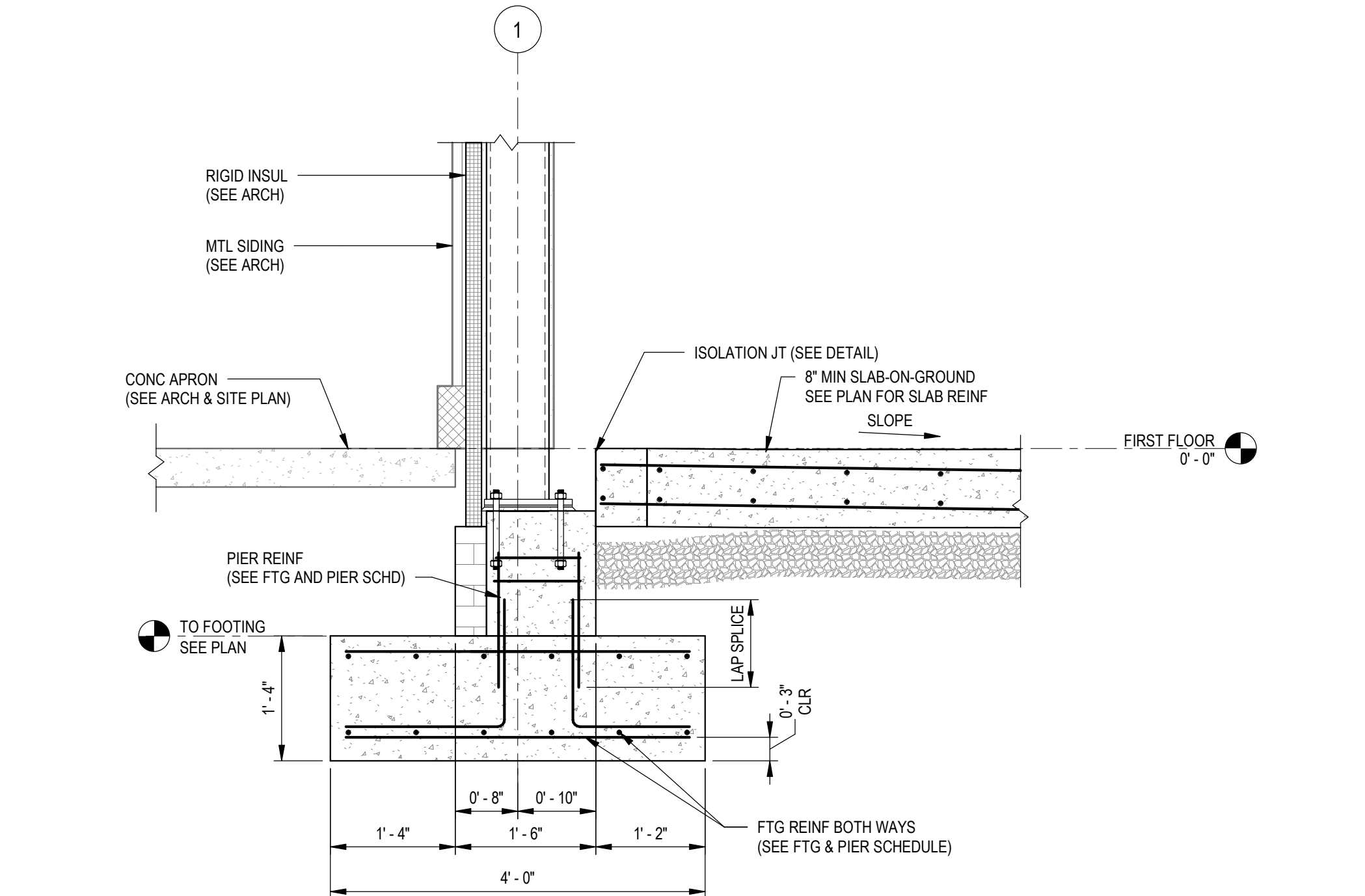


NOTE: BOTTOM OF FOOTING TO MATCH EXISTING. VERIFY IN FIELD AND NOTIFY ENGINEER IF 1'-4" THICK IS NOT AVAILABLE. UNDERPINNING MAY BE REQUIRED.

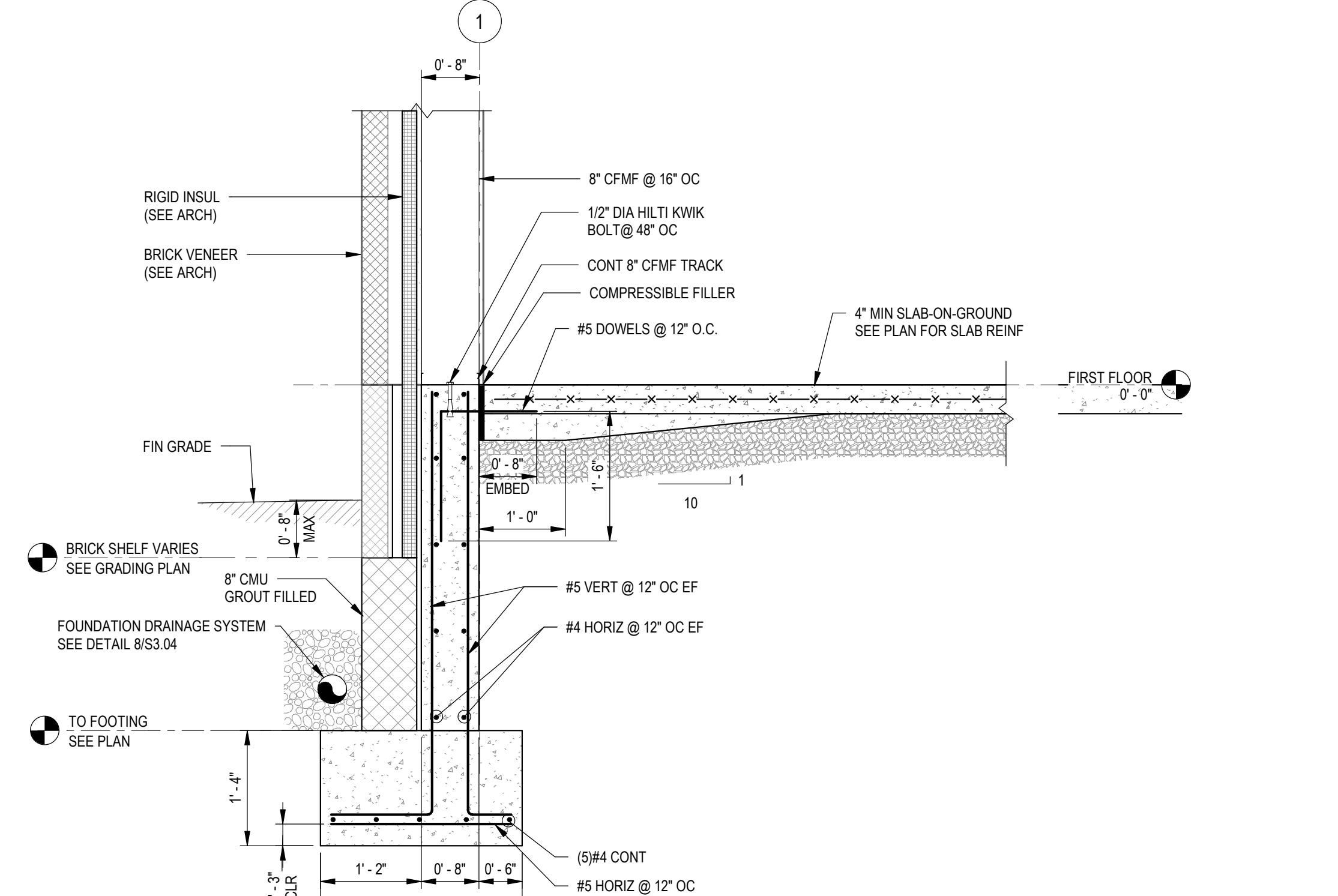
3 FOUNDATION TO EXISTING FOUNDATION SECTION



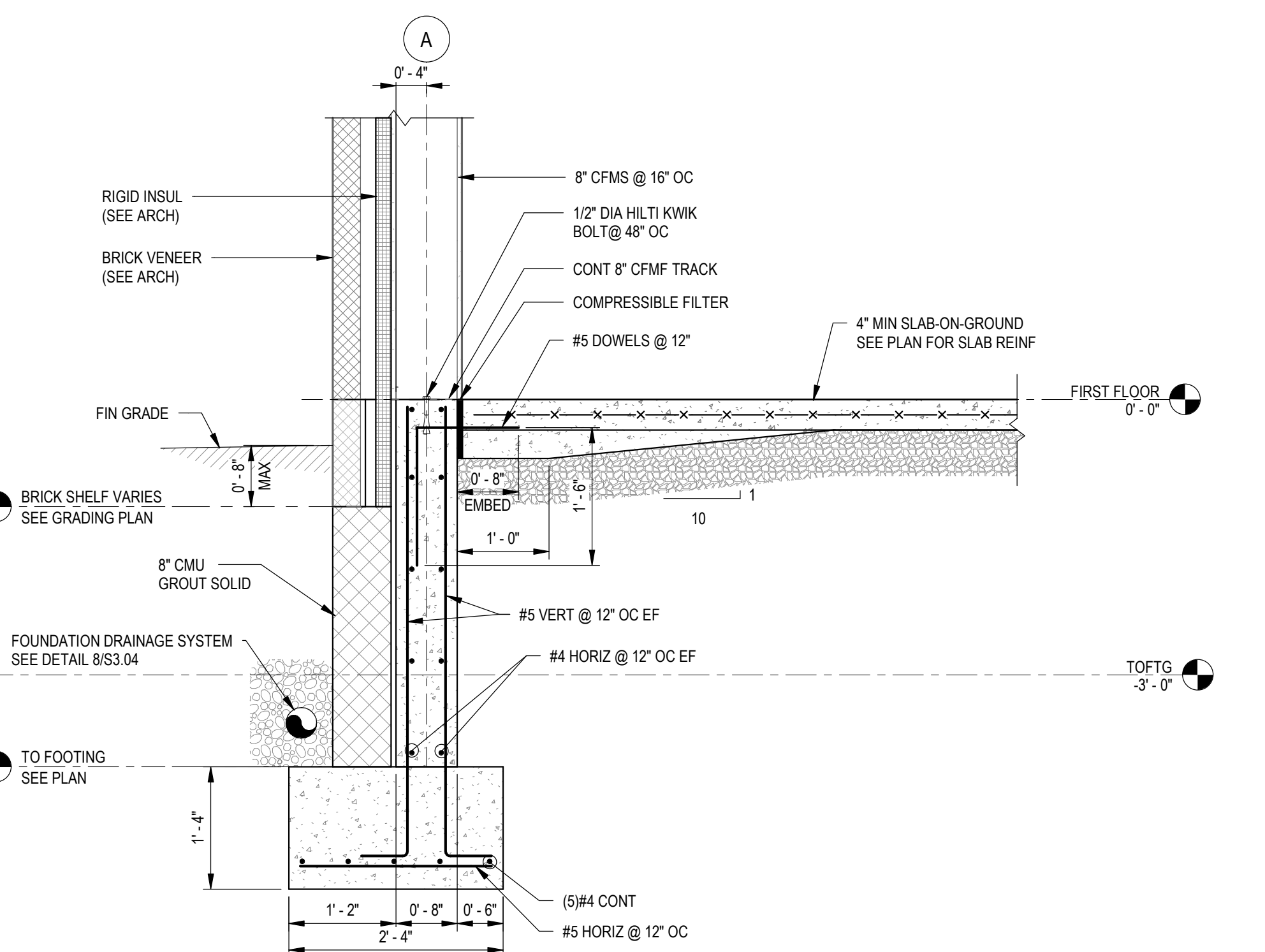
4 FOUNDATION SECTION @ OH DOOR



5 PIER SECTION @ OH DOOR WALL



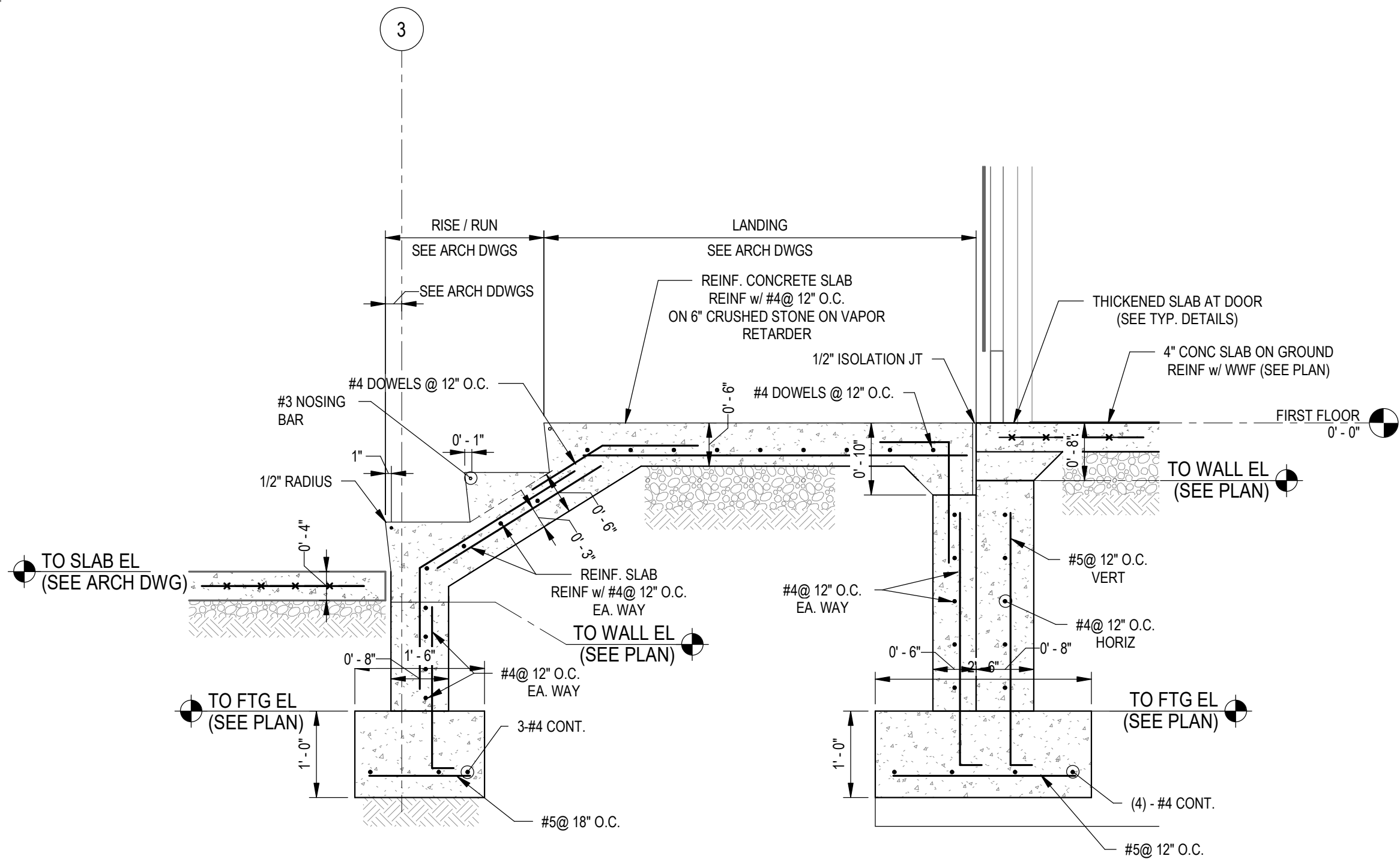
6 FOUNDATION SECTION



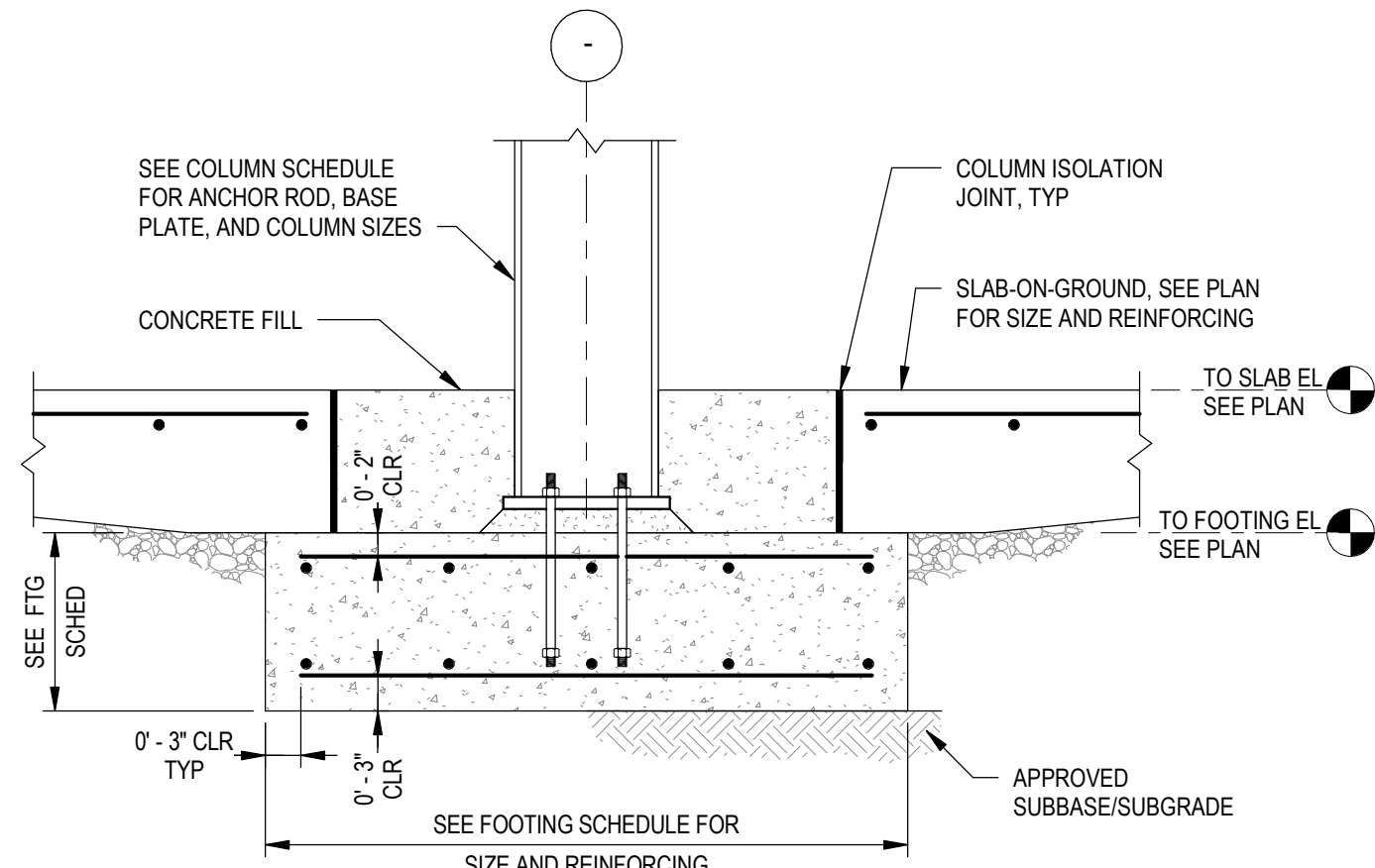
7 FOUNDATION SECTION

REVISIONS	DATE	DESCRIPTION
MARK		

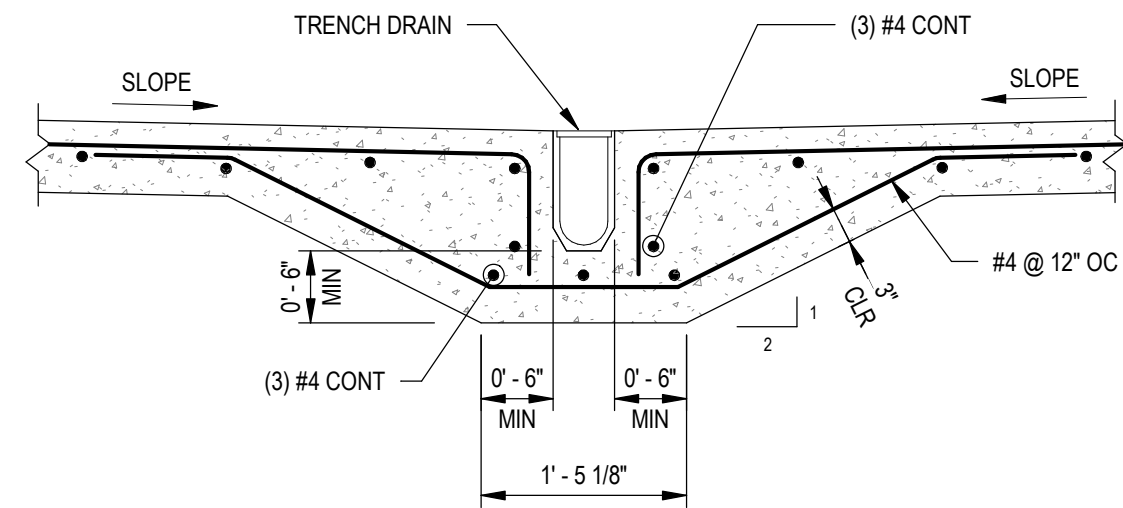
SECTIONS AND DETAILS	SHEET TITLE
JOB NO. 91242	
DATE 03/06/2025	
SHEET NO.	



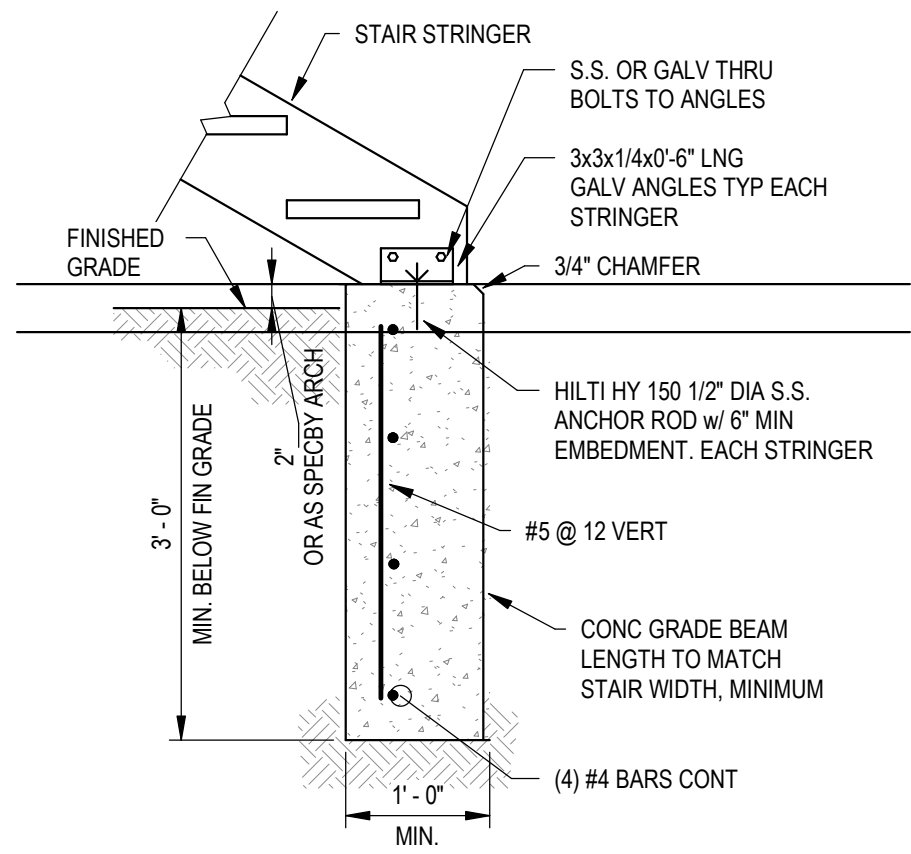
1 STAIR SECTION



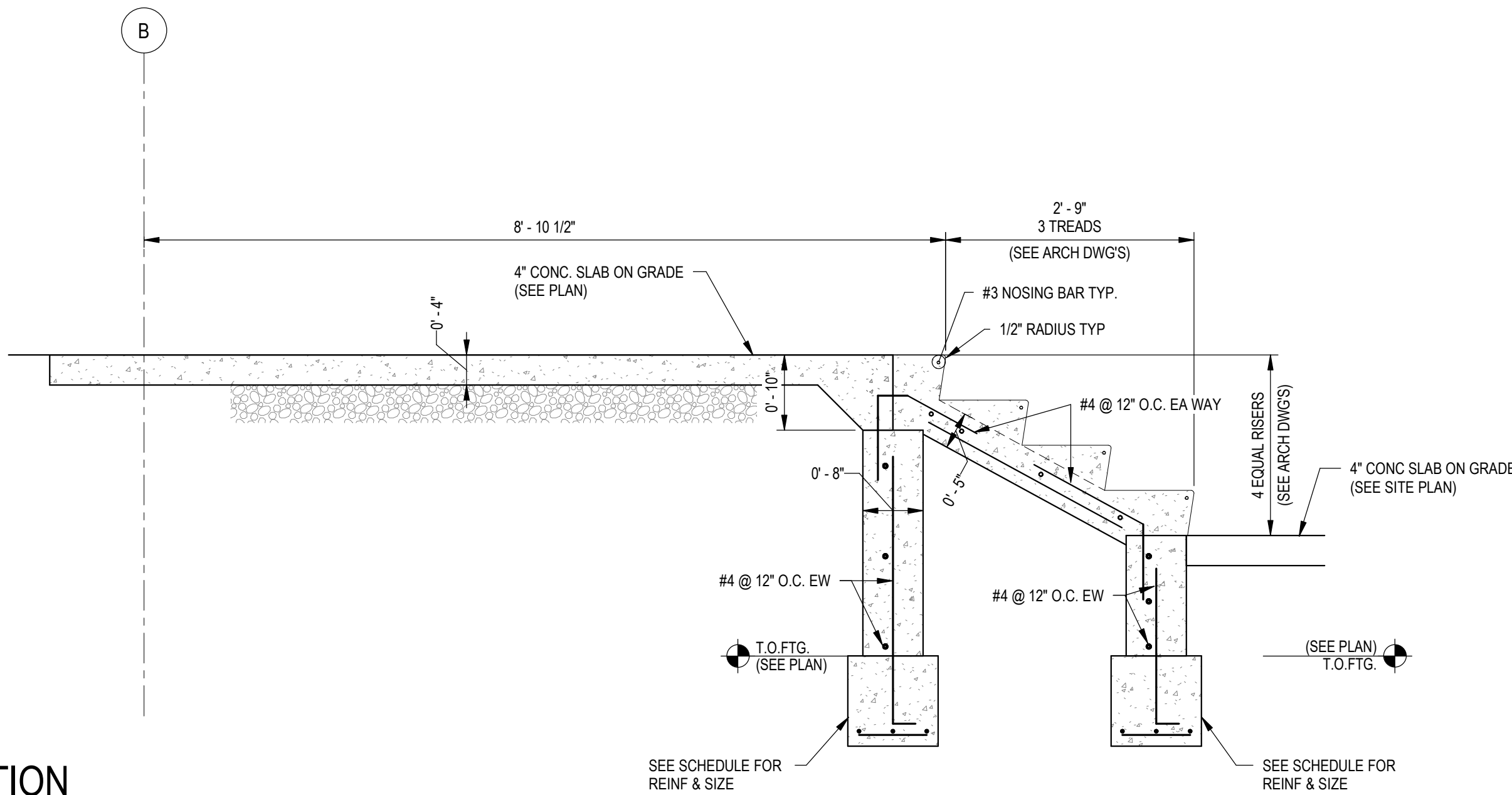
2 INTERIOR COLUMN FOOTING DETAIL



3 SLAB ON GRADE TRENCH DRAIN



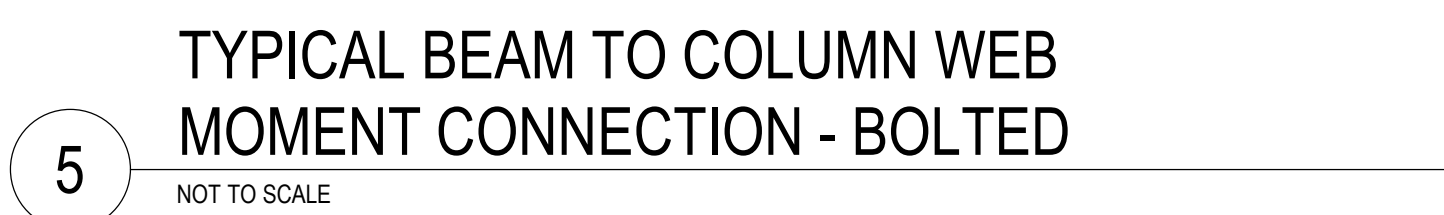
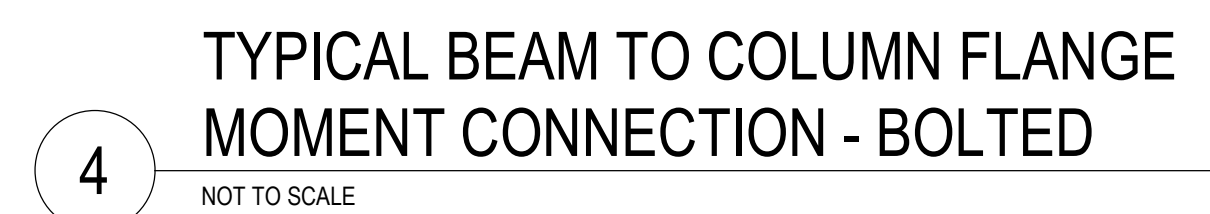
4 SECTION

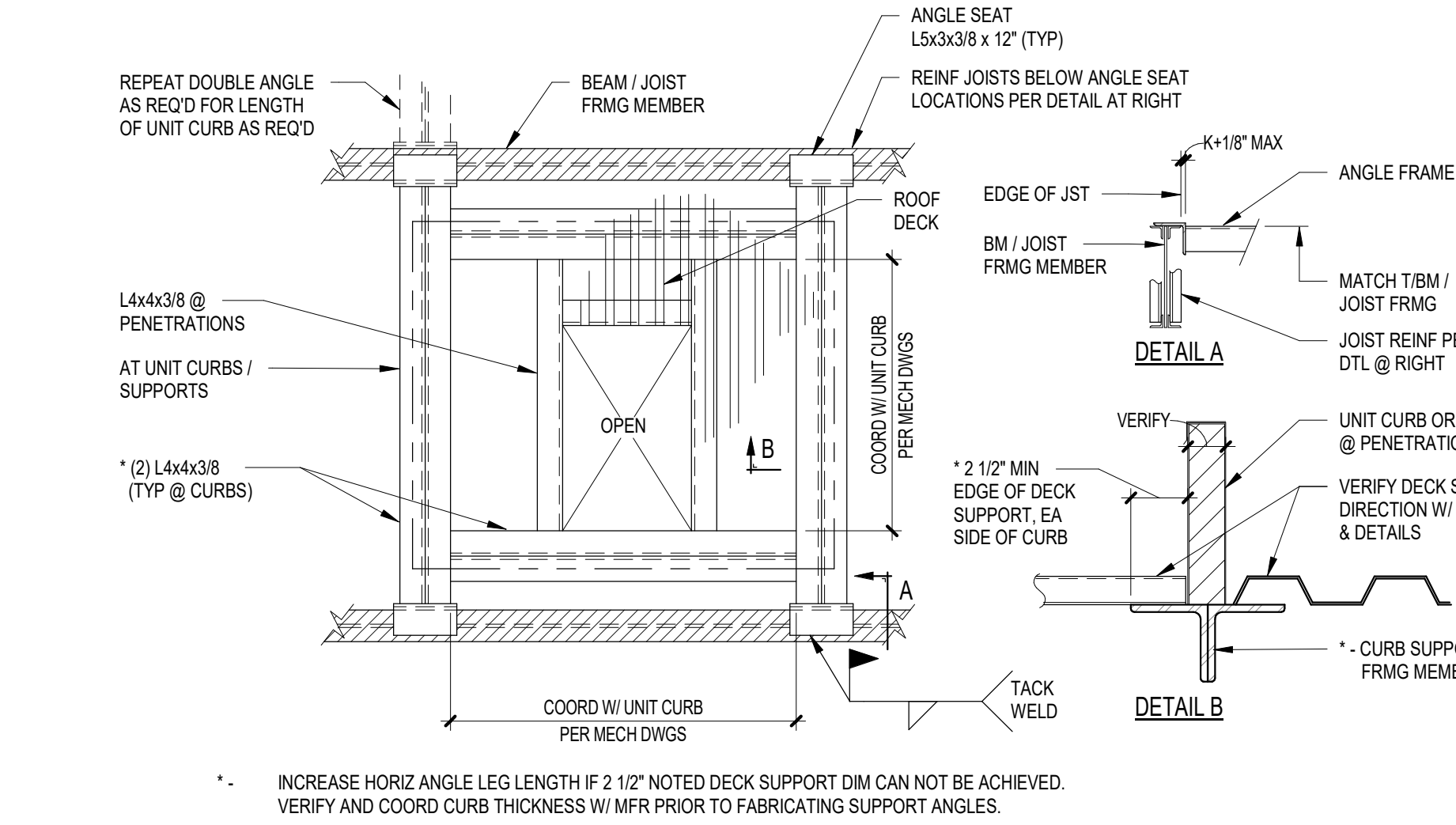


5 SECTION



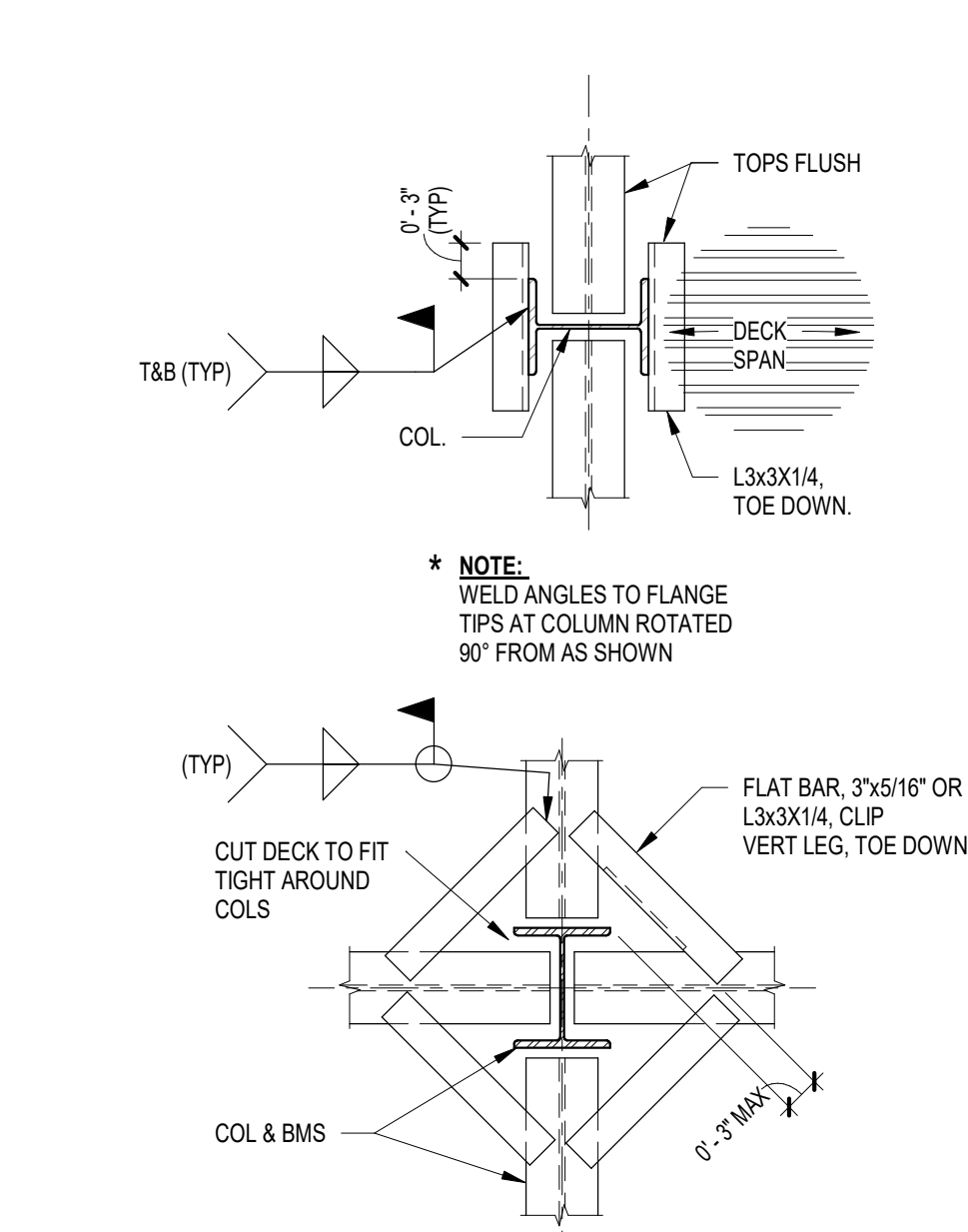


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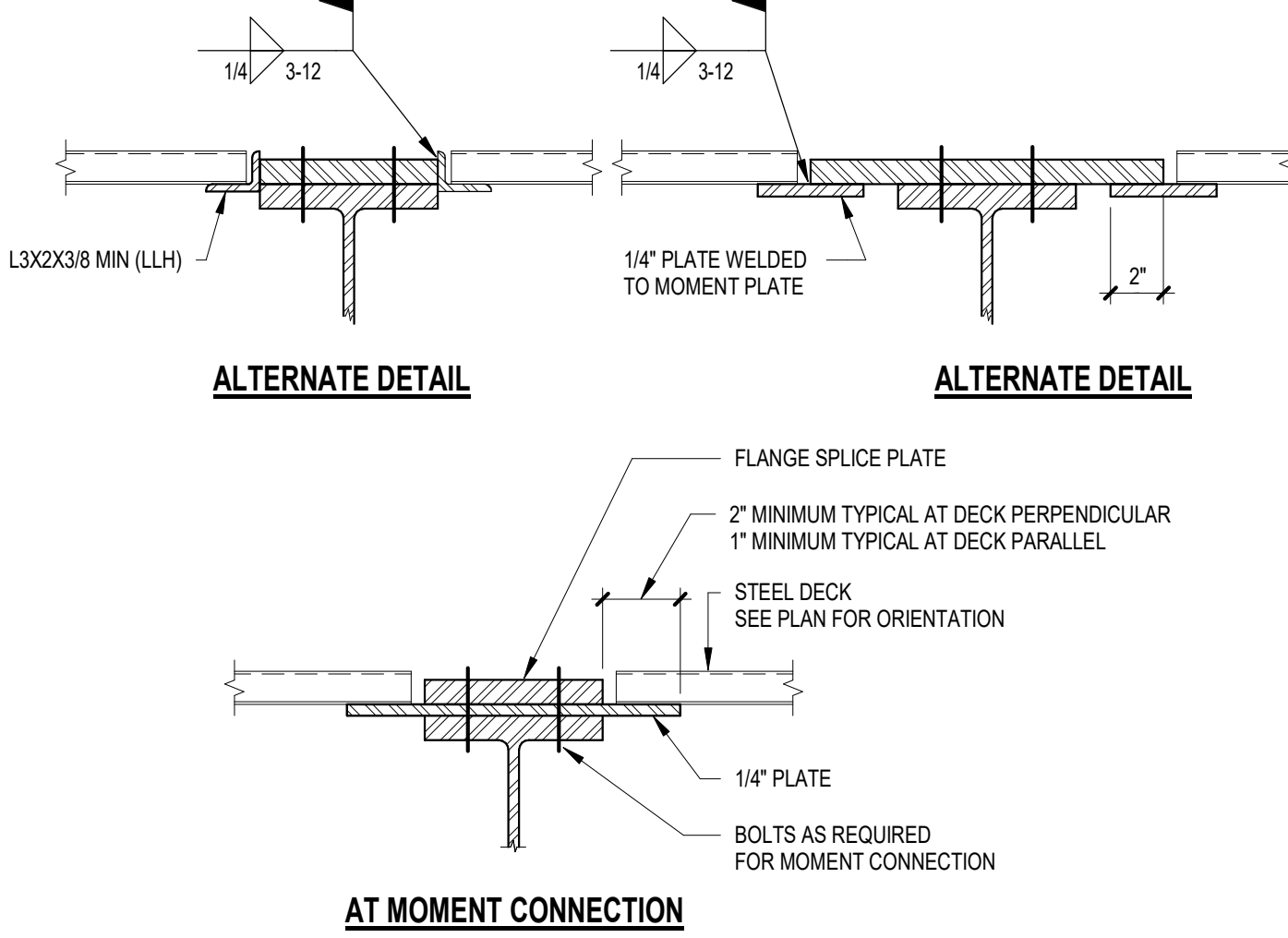


NOTE:
IF WEIGHT OF PURCHASED ROOF TOP EQUIPMENT EXCEEDS THE WEIGHT SHOWN ON THE MECHANICAL DRAWINGS BY 5% OR LOCATION OF UNIT IS DIFFERENT THAN PLAN, G.C. MUST CONTACT THE ENGINEER OF RECORD TO VERIFY ROOF CAPACITY.

1 ROOF DECK SUPPORT DETAIL
NOT TO SCALE

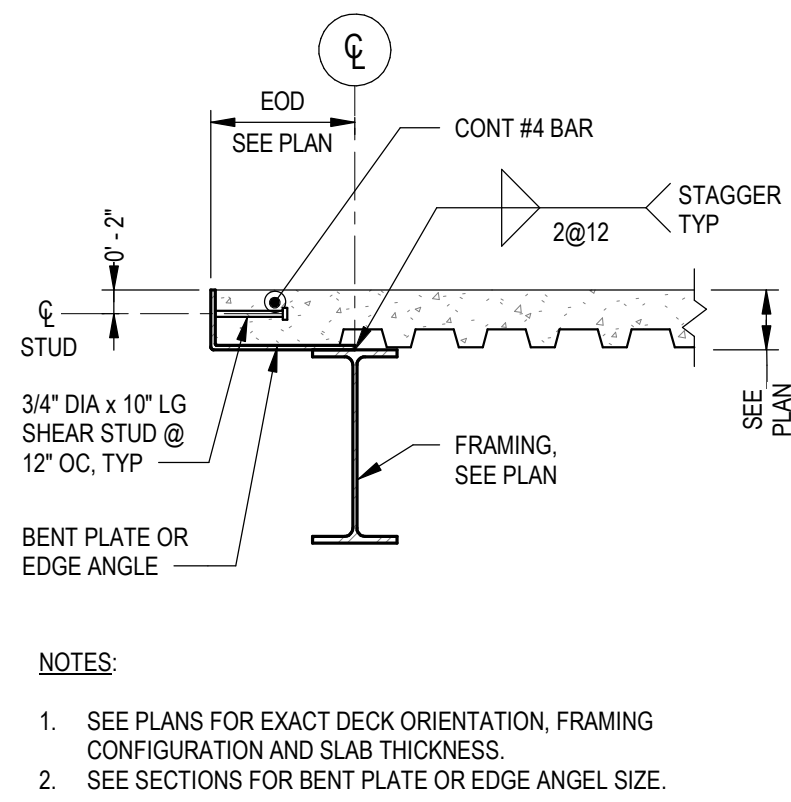


2 TYPICAL DECK SUPPORT AT MOMENT CONNECTION
NOT TO SCALE

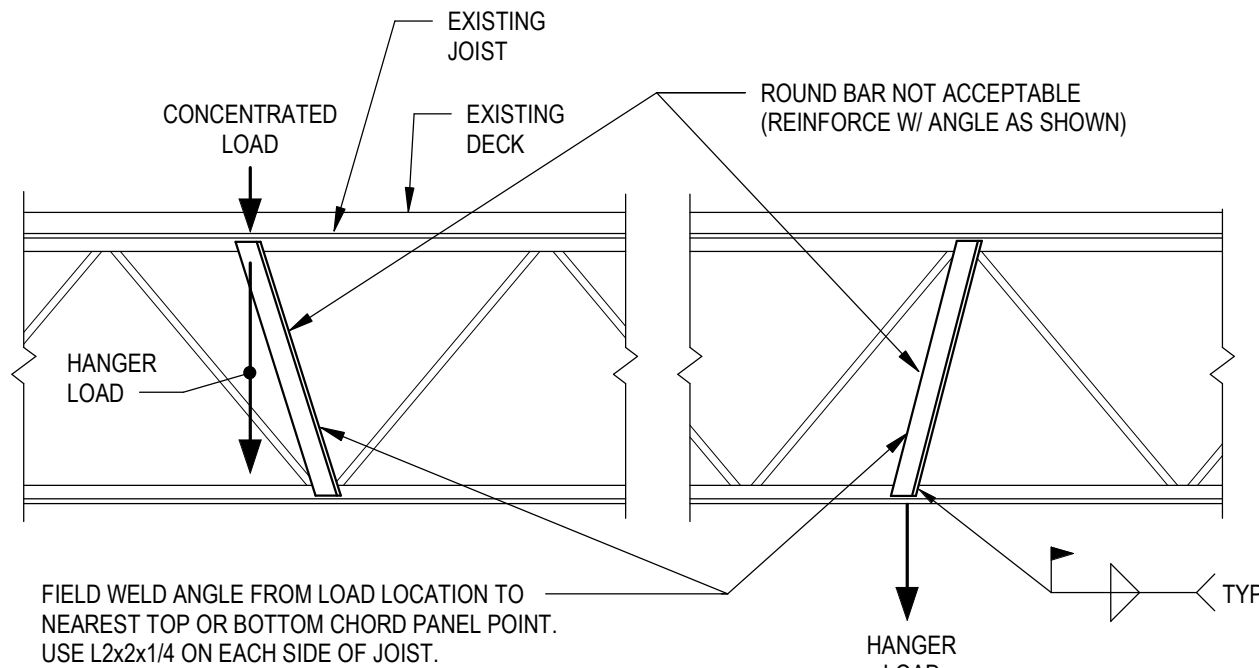


NOTES:
1. MAXIMUM CIRCULAR OPENING SIZE = 1'-4" DIA
2. MAXIMUM RECTANGULAR OPENING SIZE = 2'-0"
3. CUT DECK AFTER CONC REACHES $P_c = 3000$
4. COORDINATE EXACT SIZE AND LOCATION OF OPENINGS W/ ARCH AND MEP REQUIREMENTS

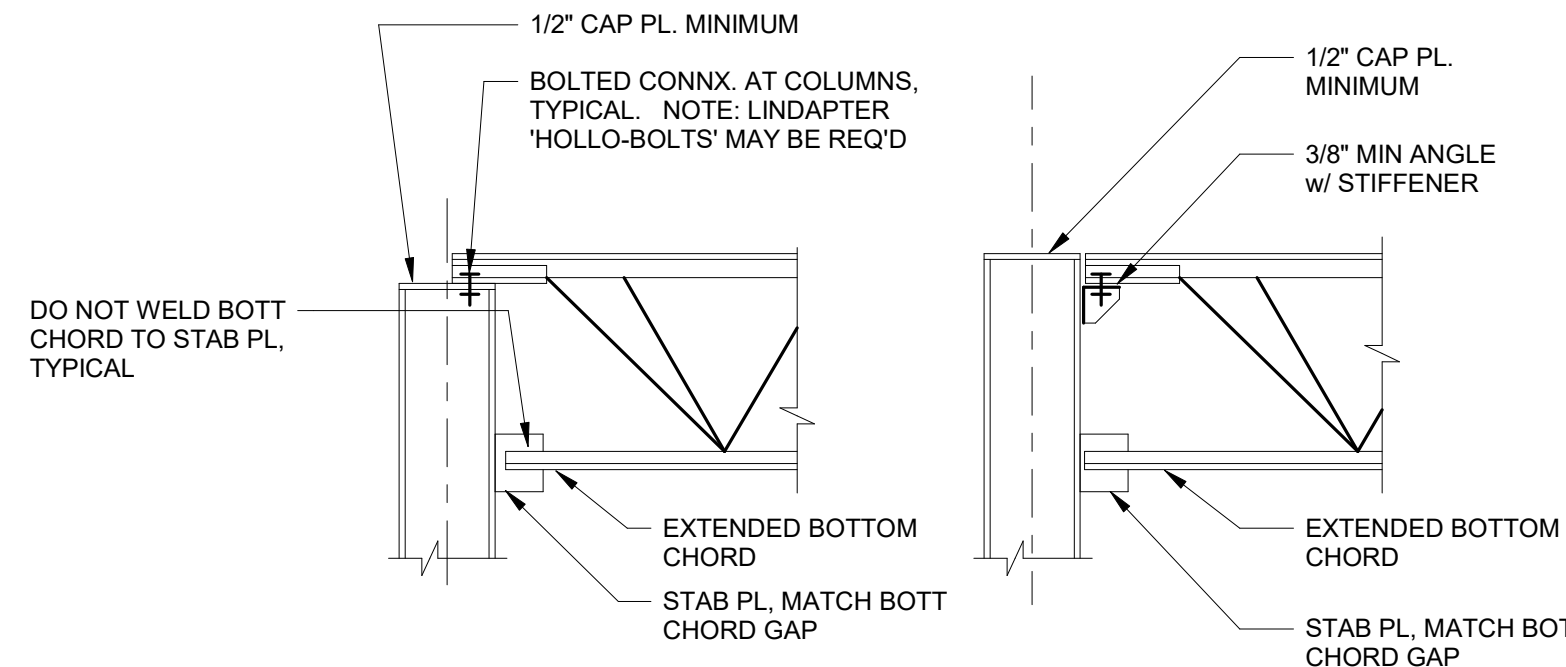
3 REINFORCED OPENINGS IN SLAB ON METAL DECK
NOT TO SCALE



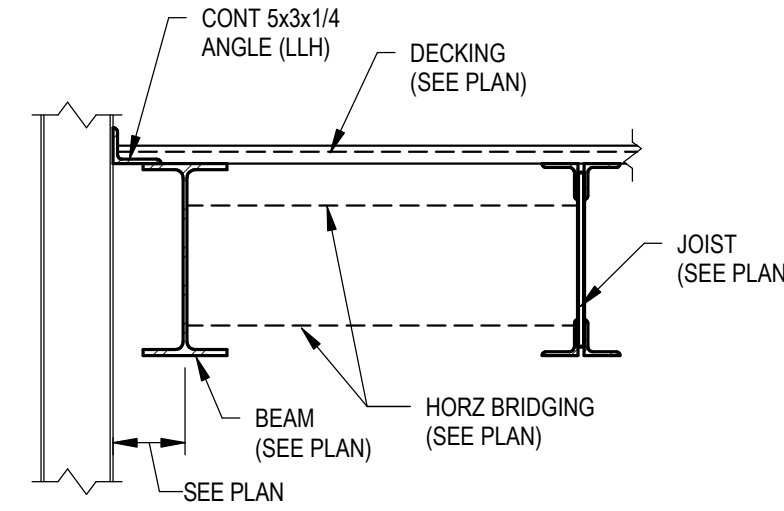
4 TYPICAL SLAB EDGE ON METAL DECK DETAIL
NOT TO SCALE



5 JOIST REINFORCING DETAIL FOR POINT LOADS
NOT TO SCALE



6 JOIST TO COLUMN DETAILS
NOT TO SCALE



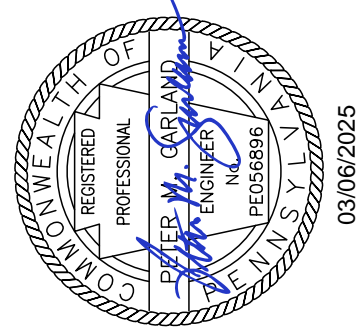
7 HORIZONTAL BRIDGING DETAIL
NOT TO SCALE

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TYPICAL SECTIONS AND DETAILS

JOB NO. 91242
DATE 03/06/2025
SHEET NO.

S5.03



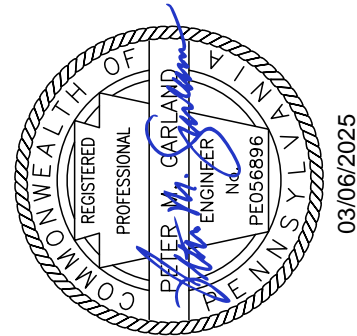
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SHEET NO.	

S5.03

KCBA Architects
Eight East Broad Street
Philadelphia, PA 19102-3407
P 215.388.5806
kcb-architects.com



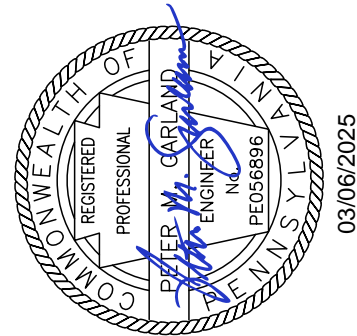
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S5.03

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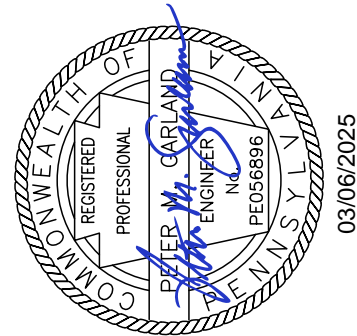
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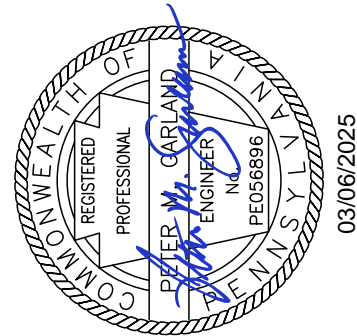
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REVISIONS	DATE	DESCRIPTION

JOB NO.	91242
DATE	03/06/2025
SHEET NO.	

S5.03

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Philadelphia, PA 19102-3407
P 215.388.5806
kcb-architects.com



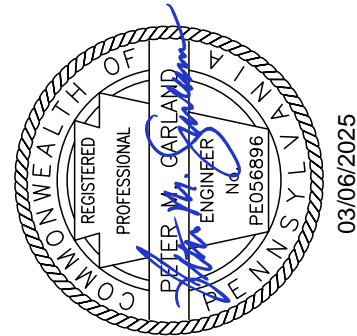
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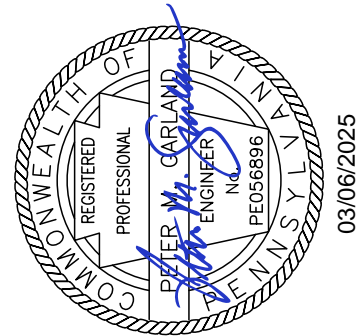
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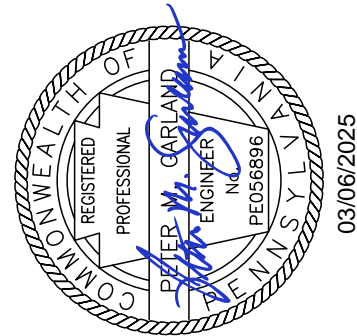
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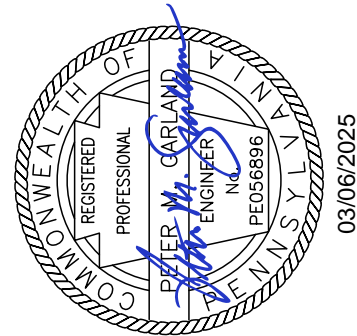
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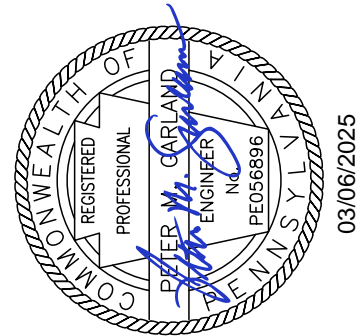
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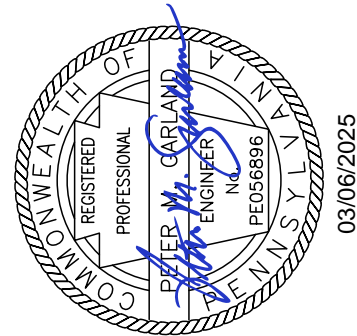
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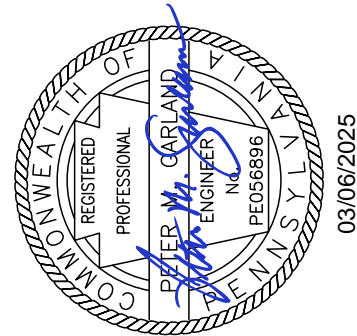
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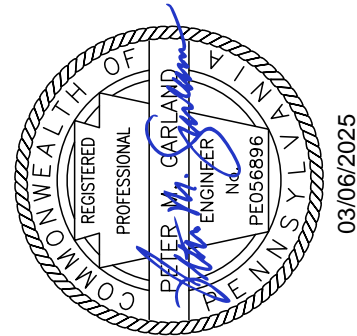
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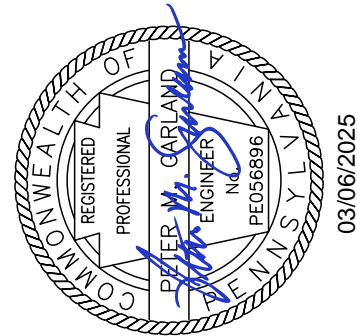
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