

FOLCROFT BOROUGH NEW BOROUGH ADMINISTRATION, POLICE & COMMUNITY CENTER

ASHLAND AVE. FOLCROFT, PA 19032

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LINN ARCHITECTS

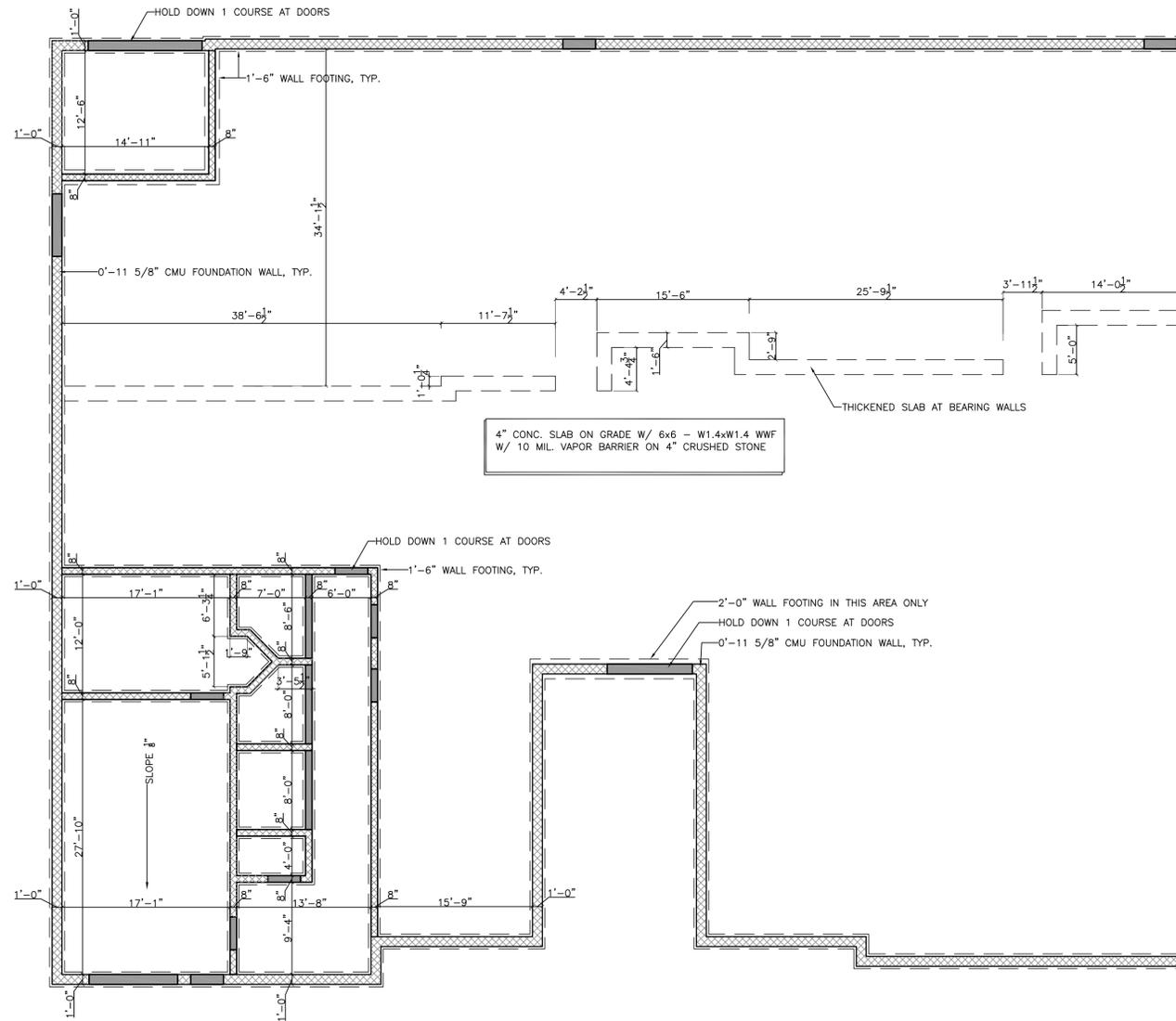
ARCHITECTURE
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INTERIOR DESIGN

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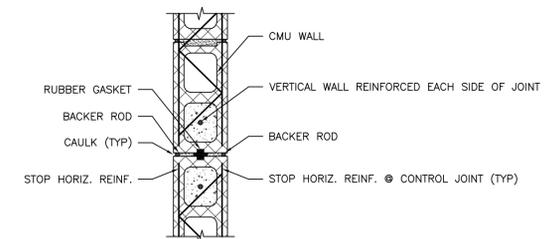
TITLE SHEET

FOLCROFT MUNICIPAL COMPLEX
BOROUGH OF FOLCROFT
ASHLAND AVE.
FOLCROFT, PA 19032

DATE: 6/25/20	NO.	DESCRIPTION	DATE	
	1	ISSUED FOR BID	2/28/20	
SCALE	DRAWN BY	CHECKED BY	PROJ. NO.	
			17086	
SHEET NO.				SHEET OF
T-1				



1 FOUNDATION PLAN
A-1 BOROUGH BUILDING SCALE: 1/8"=1'-0"



2 TYP. CONTROL JOINT DTL.
A-1 BOROUGH BUILDING SCALE: 1"=1'-0"



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FOUNDATION PLAN
NEW MUNICIPAL BUILDINGS - BOROUGH BUILDING
BOROUGH OF FOLCROFT
ASHLAND AVE.
FOLCROFT, PA 19032

DATE	REVISIONS NO.	DESCRIPTION	DATE
6/25/20	1	ISSUED FOR BID	2/28/20
6/25/20	2	ISSUE FOR PERMIT	6/25/20

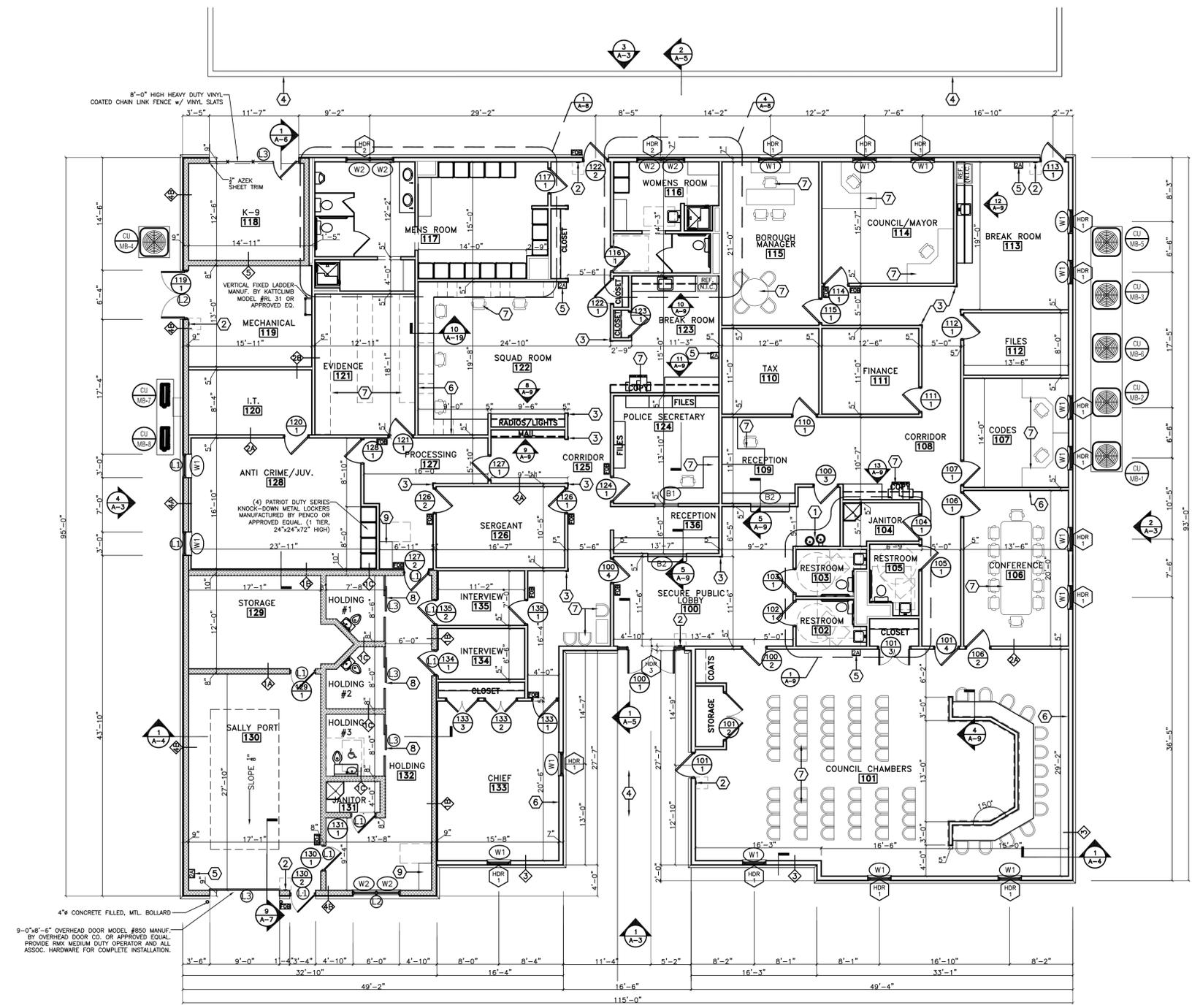
SHEET NO. **A-1**

PROJ. NO. 17086
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CMU WALL REINFORCING SCHEDULE	
VERTICAL	HORIZONTAL
#5 BARS @ 48" O.C.	(2) #4 BARS IN FULLY GROUTED BOND BEAM AT TOP OF WALL AND BEARING LOCATIONS. STANDARD WEIGHT (NO. 9) LADDER OR WIRE REINFORCEMENT @ 16" O.C.

LINTEL SCHEDULE: 8" WALLS							
TAG	W	LINTEL DEPTH	STIRRUPS	"A" BARS	"B" BARS	"C" BARS	# CELLS TO GROUT
(1)		8"	-	(1) #5	(1) #5	(1) #5	ONE
(2)		>2'-8" <6'-0"	#3 @ 16"	(2) #5	(1) #5	(2) #5	TWO
(3)		>8'-0" <12'-0"	#3 @ 8"	(2) #5	(2) #5	(2) #5	TWO

NOTES:
 1) USE BAR QUANTITIES AND SIZES GIVEN IN LINTEL SCHEDULE UNLESS OTHERWISE NOTED ON THE DRAWINGS.
 2) EXTEND "C" BARS 2'-8" MIN BEYOND TOP AND BOTTOM OF OPENING, EXCEPT THAT WHEN "H" OR "W" EXCEEDS 2'-0", "C" BARS SHALL EXTEND FULL HEIGHT.
 3) "A" AND "B" BARS SHALL EXTEND 2'-8" EACH SIDE OF THE OPENING.
 4) GROUT ALL CELLS OVER OPENING TO LINTEL DEPTH INDICATION IN SCHEDULE.
 5) CELLS SHOWN TO BE GROUTED ARE MINIMUM. GROUT ALL CELLS AS SHOWN IN TYPICAL CMU WALL DETAIL.
 6) DO NOT SOLID GROUT WALLS UNLESS SPECIFICALLY INDICATED.

HEADER SCHEDULE			
TAG	SIZE	GRADE	REMARKS
HDR 1	(2)2x8	DF-L NO. 1/NO. 2	SPANS LESS THAN 3'-6"
HDR 2	(3)2x12	DF-L NO. 1/NO. 2	SPANS BETWEEN 3'-6" TO 6'-0"
HDR 3	(2)2x12	DF-L NO. 2	PRESSURE/PRESERVATIVE TREATED AT EXPOSED DOOR LOCATIONS

- WALL LEGEND**
BOROUGH BUILDING
- INTERIOR WALL - 8" REINFORCED CMU
 - INTERIOR WALL - 8" REINFORCED CMU W/ 1X3 WD. FURRING AND 3/8" GYP. BD.
 - INTERIOR WALL - 8" REINFORCED CMU, WALLS GROUTED SOLID
 - INTERIOR WALL - 2x4 WD. STUDS @ 16" O.C. W/ 3-1/2" (R-13) BATT INSULATION, 3/8" GYP. BD. @ BOTH SIDES. PROVIDE M.R. GYP. BD. @ ALL RESTROOM/KITCHEN WET WALLS. RUN WALLS MIN 6" ABOVE SCHEDULED CEILING FINISH.
 - INTERIOR WALL - 2x4 WD. STUDS @ 16" O.C. W/ 3-1/2" (R-13) BATT INSULATION, 3/8" GYP. BD. @ BOTH SIDES. PROVIDE M.R. GYP. BD. @ ALL RESTROOM/KITCHEN WET WALLS. RUN WALL TIGHT TO BTM. OF TRUSS.
 - EXTERIOR WALL - 2x6 WD. STUDS @ 16" O.C. W/ 5-1/2" (R-21) BATT INSULATION, 5/8" GYP. BD. @ INTERIOR. 1/2" EXTERIOR SHEATHING, TYVEK FLUID APPLIED WEATHER BARRIER, AND VINYL SIDING AT EXTERIOR. BRICK WATERTABLE. SEE 2/A-7 FOR WATERTABLE DETAIL.
 - EXTERIOR WALL - 8" REINFORCED CMU W/ 1X3 WD. FURRING @ 16" O.C., TYVEK FLUID APPLIED WEATHER BARRIER, VINYL SIDING AND BRICK WATERTABLE @ EXTERIOR. SEE 1/A-7 FOR WATERTABLE DETAIL. 2x6 WD. STUDS @ 16" O.C. W/ 5-1/2" (R-19) BATT INSULATION, 5/8" GYP. BD. @ INTERIOR.
 - EXTERIOR WALL - PAINTED 8" REINFORCED CMU W/ 1X3 WD. FURRING @ 16" O.C., TYVEK FLUID APPLIED WEATHER BARRIER, VINYL SIDING AND BRICK WATERTABLE @ EXTERIOR. SEE 1/A-7 FOR WATERTABLE DETAIL.
 - 8" REINFORCED CMU W/ INSULATED CORE

- KEYNOTES**
BOROUGH BUILDING
- 1 ADA ACCESSIBLE HIGH/LOW WATER FOUNTAIN W/ SPOUT MTD. 36" MAX. A.F.F. SEE PLUMBING PLANS FOR FIXTURE INFORMATION.
 - 2 TACTILE EXIT SIGN MOUNTED ON STRIKE SIDE OF DOOR @ 48"-60" A.F.F. TO BRAILLE.
 - 3 CLEAR LEXAN POLYCARBONATE CORNER GUARD - 48" IN LENGTH WITH 2" WINGS.
 - 4 4" CONCRETE WALK ON 4" CRUSHED STONE W/ W1.4XW1.4 6x6 WWF. PROVIDE CONTROL JOINTS @ 5'-0" O.C. & EXPANSION JOINT @ 20'-0" O.C. SEE CIVIL PLANS FOR LOCATIONS.
 - 5 2A FIRE EXTINGUISHER MTD. 48" TO TOP
 - 6 PROVIDE SOLID BLOCKING IN WALL FOR TVS OR SHELVING AS REQ. VERIFY FINAL LOCATION WITH OWNER.
 - 7 FURNITURE SUPPLIED & INSTALLED BY OWNER.
 - 8 SLIDING CELL DOOR AND BARS TO BE FOLGER ADAM SLIDING GRILLE 102-30 TRACK AND GRILLE. HARDWARE TO BE FOLGER ADAM 320 MECHANICAL LOCKS. DOOR TO BE GRILLE MOUNTED. GRILLES TO BE MOUNTED TO MASONRY WALLS WITH 3/8" CONCRETE ANCHORS @ 12" O.C. MASONRY WALLS AT HOLDING CELLS TO BE GROUTED SOLID.
 - 9 ADA ACCESSIBLE HOPE BENCH 1'-8"x3'-6" WITH STAINLESS STEEL LEGS MOUNTED 18" A.F.F. TO TOP. PROVIDE CLEAR FLOOR SPACE PER. 305.3.

1 PROPOSED FLOOR PLAN
 A-2 BOROUGH BUILDING SCALE: 1/8"=1'-0"

PROPOSED FLOOR PLAN
 NEW MUNICIPAL BUILDINGS - BOROUGH BUILDING
 BOROUGH OF FOLCROFT
 ASHLAND AVE.
 FOLCROFT, PA 19032

DATE	REVISIONS	NO.	DESCRIPTION	DATE
6/25/20	1	ISSUED FOR BID	2/28/20	
1/8=1'-0"	2	ISSUE FOR PERMIT	6/25/20	

SHEET NO. **A-2** OF SHEET

PROJ. NO. 17086



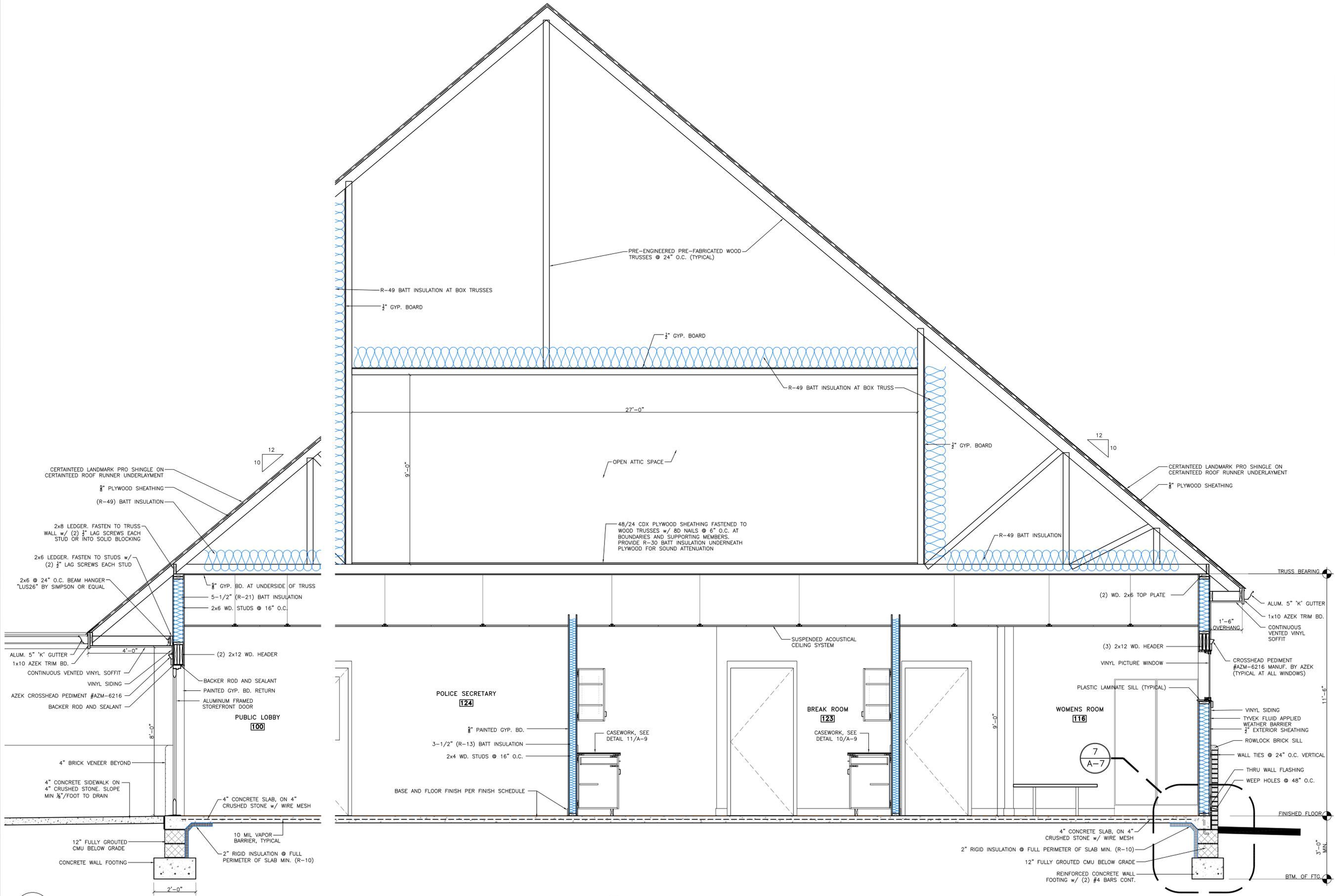
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BUILDING SECTIONS
 NEW MUNICIPAL BUILDINGS - BOROUGH BUILDING
 BOROUGH OF FOLCROFT
 ASHLAND AVE.
 FOLCROFT, PA 19032

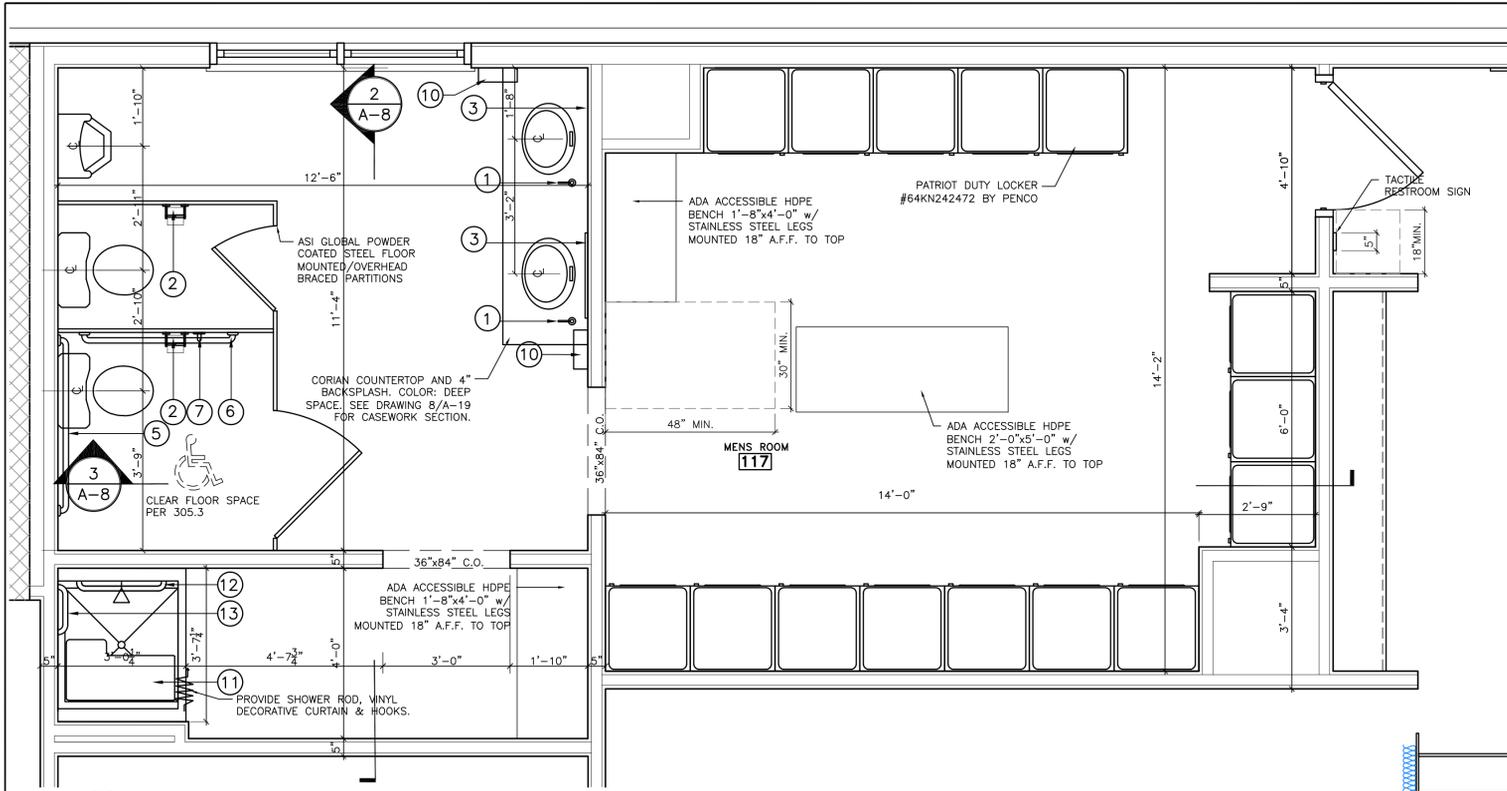
NO.	DATE	DESCRIPTION	REVISIONS
1	2/28/20	ISSUED FOR BID	
2	6/25/20	ISSUE FOR PERMIT	

DATE: 6/25/20
 SCALE: 1/2"=1'-0"
 DRAWN BY: []
 CHECKED BY: []
 PROJ. NO.: 17086
SHEET NO. A-5
 OF [] SHEET



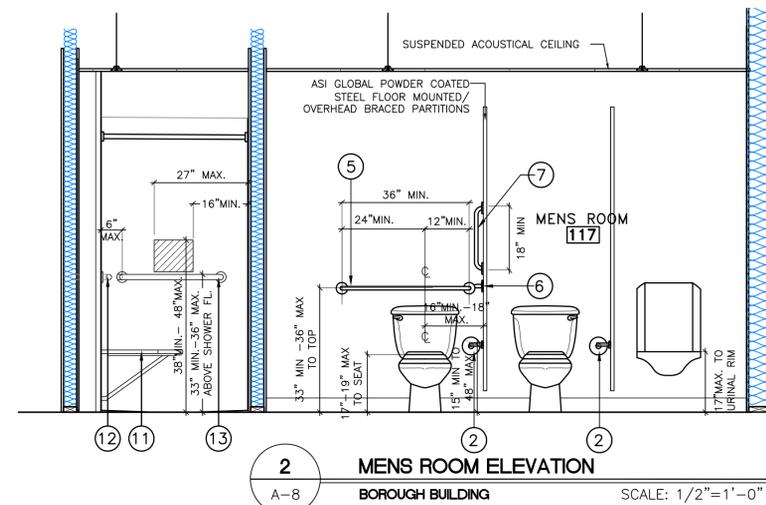
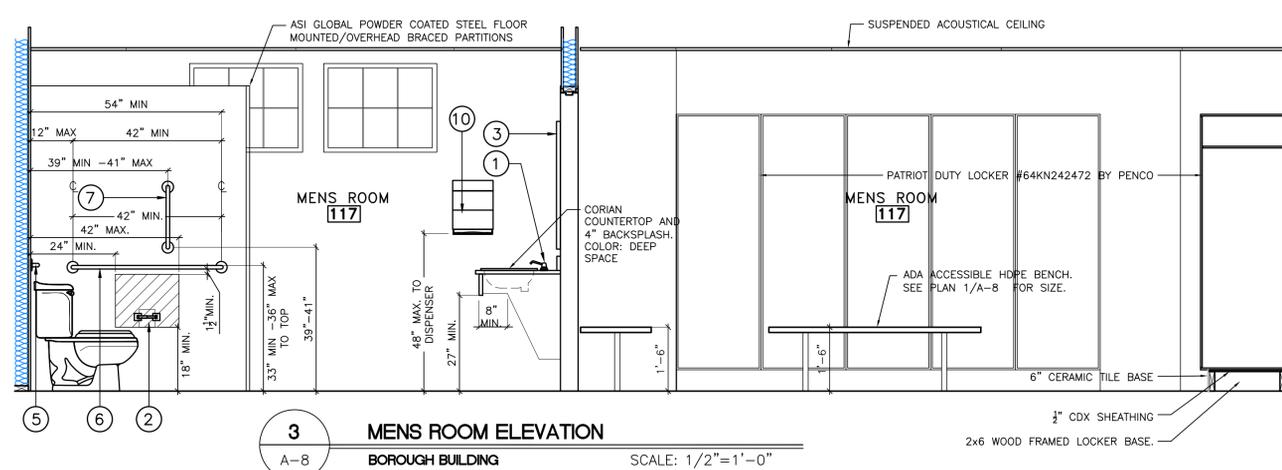
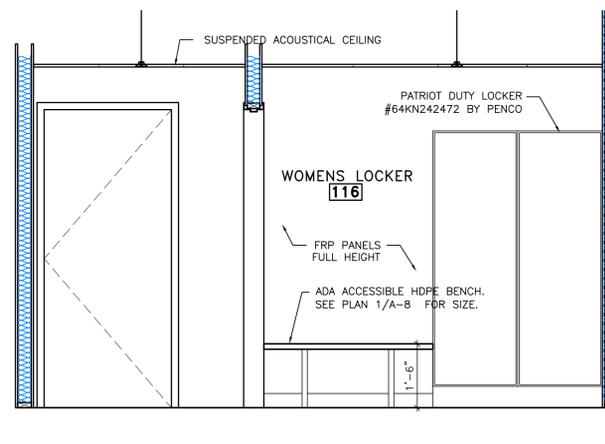
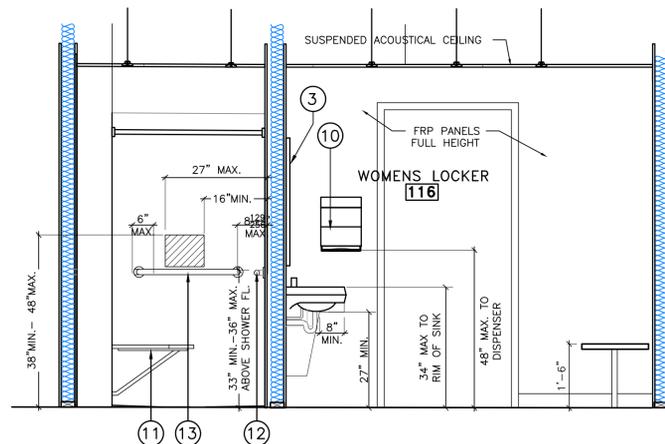
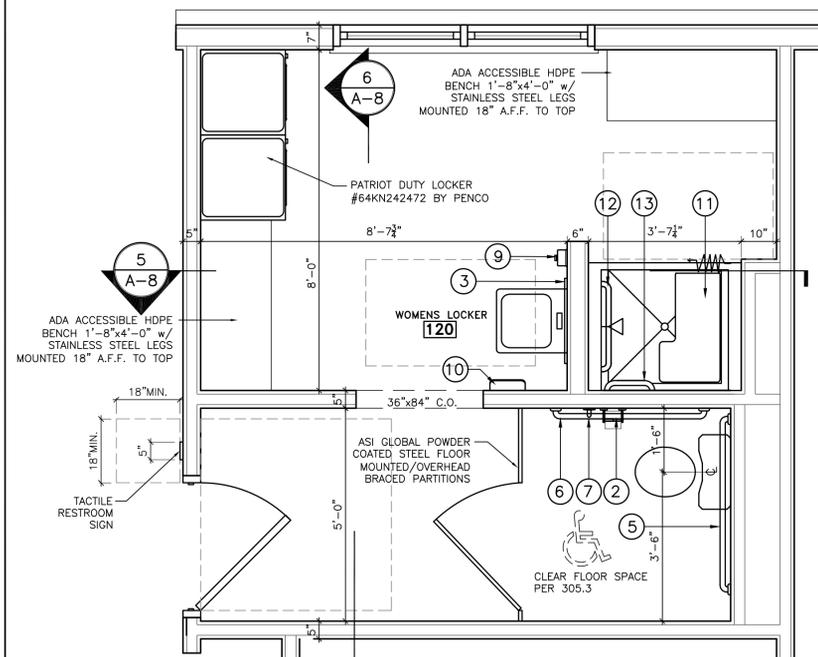
1 BUILDING SECTION
 A-5 BOROUGH BUILDING SCALE: 1/2"=1'-0"

2 BUILDING SECTION
 A-5 BOROUGH BUILDING SCALE: 1/2"=1'-0"



TOILET RM. ACCESSORY SCHEDULE - BOROUGH BLDG.							
DESIG	MANUF.	CAT. #	ITEM	MTG.	FINISH	MTG. HEIGHT	TOTAL QTY.
1	ASI	0332-D	COUNTER MOUNTED SOAP DISP.	SURFACE	STAIN. STL.	34" A.F.F.	2
2	ASI	7035-S	TOIL. PAPER DISPENSER	SEMI-RECESSED	STAIN. STL.	SEE MOUNTING LOCATION	6
3	ASI	0600-2436	24x36 MIRROR	SURFACE	STAIN. STL.	40" MAX TO BOTTOM OF REFLECTIVE SURFACE	6
4	XLERATOR	XL-SB-208-277	AIR HAND DRYER + RECESS KIT	RECESSED	STAIN. STL.	48" MAX TO CONTROLS	3
5	ASI	3800 x 36	REAR GRAB BAR	SURFACE	STAIN. STL.	33" MIN TO 36" MAX A.F.F. TO TOP	2
6	ASI	3800 x 42	SIDE GRAB BAR	SURFACE	STAIN. STL.	33" MIN TO 36" MAX A.F.F. TO TOP	5
7	ASI	3800 x 18	VERT GRAB BAR	SURFACE	STAIN. STL.	39" MIN TO 41" MAX A.F.F. TO TOP	3
8	ASI	3800 x 24	REAR GRAB BAR	SURFACE	STAIN. STL.	33" MIN TO 36" MAX A.F.F. TO TOP	3
9	ASI	0347	SOAP DISPENSER	SURFACE	STAIN. STL.	48" MAX TO CONTROLS (REACH RANGE VARIES)	4
10	ASI	0210	PAPER TOWEL DISPENSER	SURFACE	STAIN. STL.	48" MAX. TO DISPENSER	3
11	ASI	8203	FOLDING SHOWER BENCH	SURFACE	STAIN. STL.	17" TO 19" A.F.F. TO TOP OF SEAT	2
12	ASI	3800 x 48	BACK WALL SHOWER GRAB BAR	SURFACE	STAIN. STL.	33" MIN TO 36" MAX A.F.F. TO TOP	2
13	ASI	3800 x 24	SIDE WALL SHOWER GRAB BAR	SURFACE	STAIN. STL.	33" MIN TO 36" MAX A.F.F. TO TOP	2

- NOTES:
 1. PROVIDE WOOD BLOCKING AS REQUIRED FOR ALL ACCESSORIES
 2. PROVIDE ADA APPROVED INSULATION ON ALL EXPOSED PLUMBING



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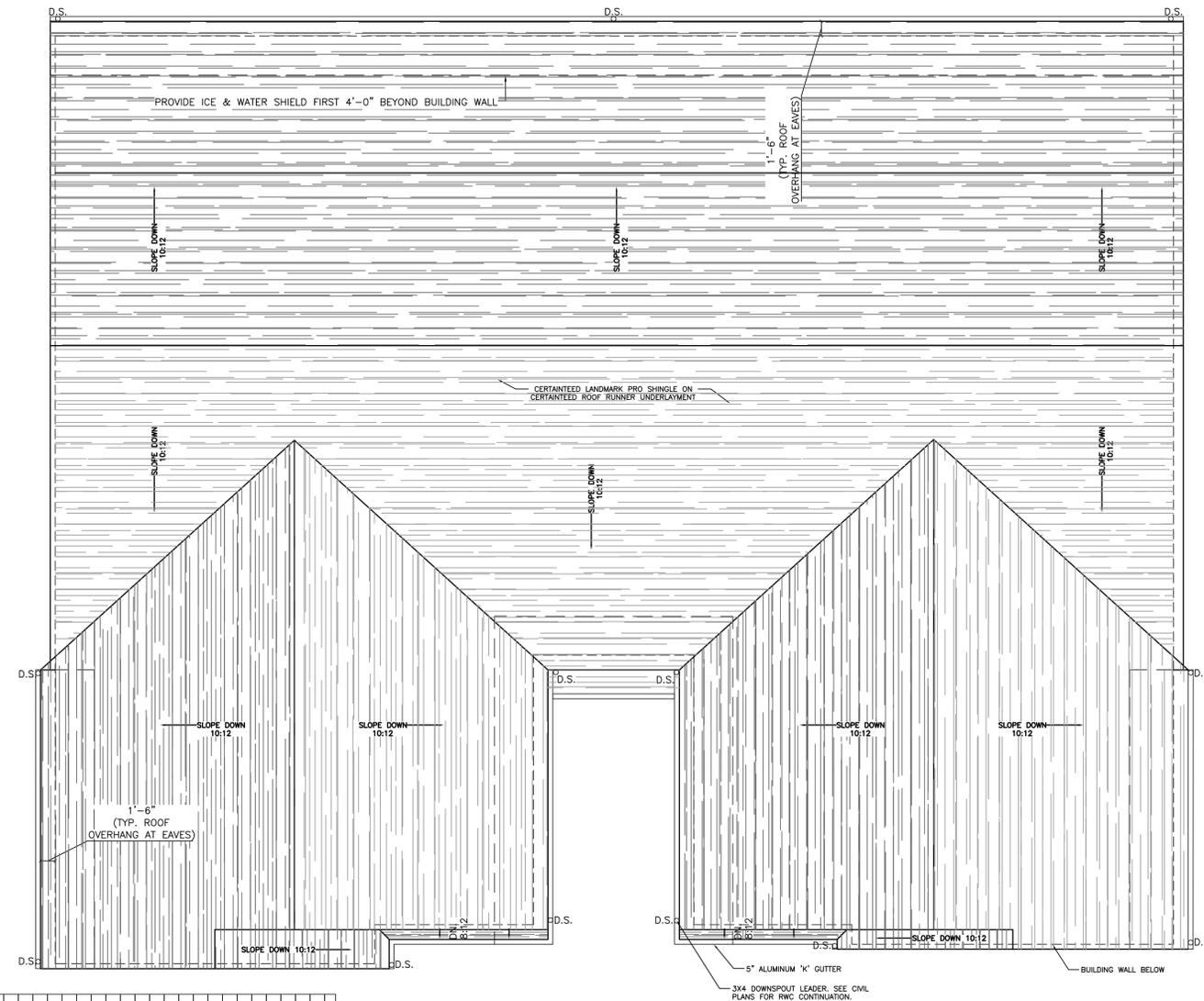
RESTROOM PLANS AND ELEVATIONS
 NEW MUNICIPAL BUILDINGS - BOROUGH BUILDING
 BOROUGH OF FOLCROFT
 ASHLAND AVE.
 FOLCROFT, PA 19032

DATE	REVISIONS NO.	DESCRIPTION	DATE
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6/25/20 <td>2 <td>ISSUE FOR PERMIT <td>6/25/20</td> </td></td>	2 <td>ISSUE FOR PERMIT <td>6/25/20</td> </td>	ISSUE FOR PERMIT <td>6/25/20</td>	6/25/20

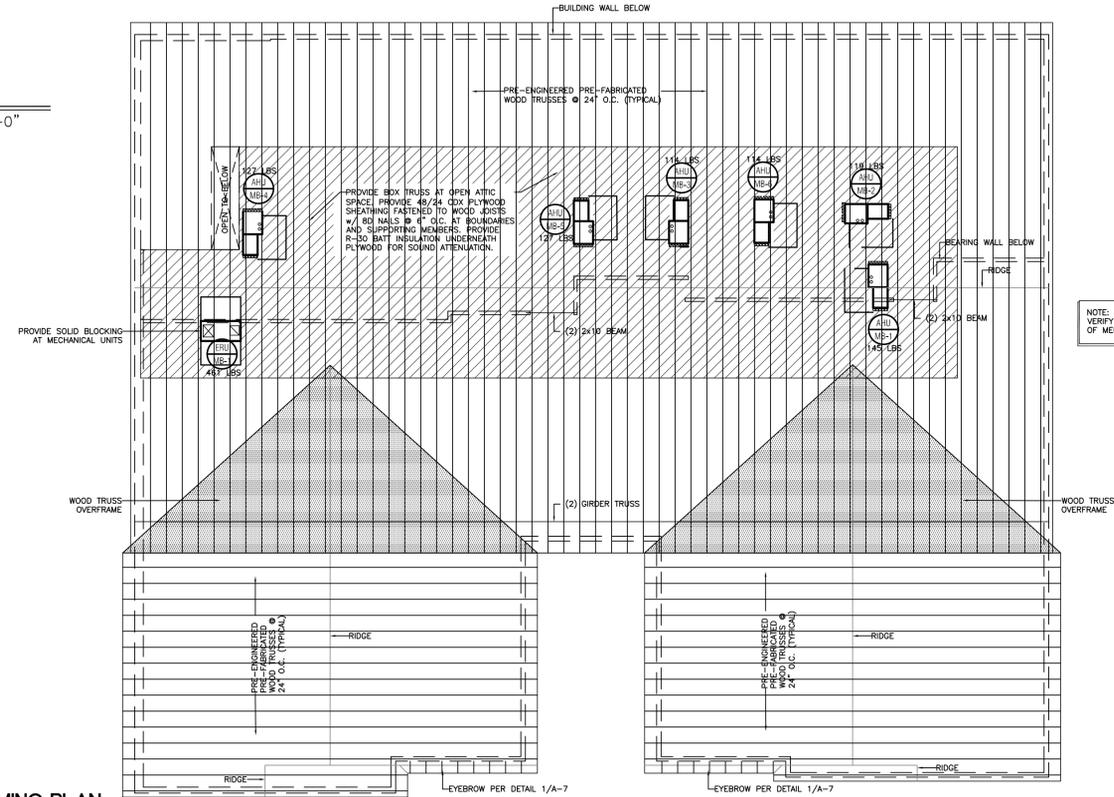
SHEET NO. **A-8** OF



1 REFLECTED CEILING PLAN
BOROUGH BUILDING SCALE: 1/8"=1'-0"



2 ROOF PLAN
BOROUGH BUILDING SCALE: 1/8"=1'-0"



3 FRAMING PLAN
BOROUGH BUILDING SCALE: 3/32"=1'-0"



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RCP, ROOF PLAN AND FRAMING PLAN
NEW MUNICIPAL BUILDINGS - BOROUGH BUILDING
BOROUGH OF FOLCROFT
ASHLAND AVE.
FOLCROFT, PA 19032

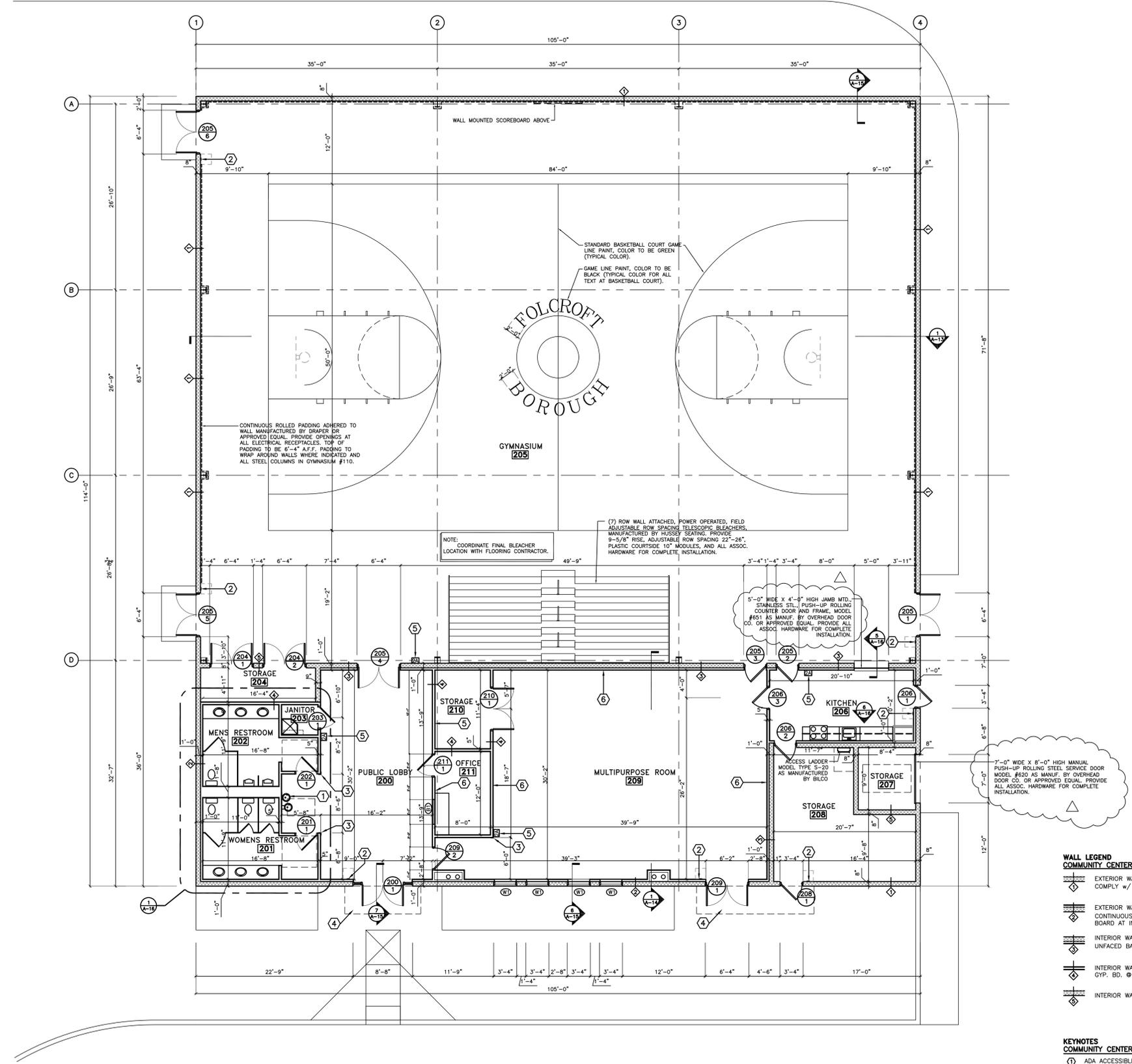
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SHEET NO. **A-10** OF SHEET 17086



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1 PROPOSED FLOOR PLAN
 A-11 COMMUNITY CENTER SCALE: 1/8" = 1'-0"

PROPOSED FLOOR PLAN
 NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
 BOROUGH OF FOLCROFT
 ASHLAND AVE.
 FOLCROFT, PA 19032

WALL LEGEND
COMMUNITY CENTER

	EXTERIOR WALL - 8" REINFORCED CMU w/ GROUND FACE @ EXTERIOR. CMU TO HAVE INSULATED CORES TO COMPLY w/ 2015 IBC R-9.5 CONTINUOUS.
	EXTERIOR WALL - 8" REINFORCED GROUND FACE CMU w/ INSULATED CORES TO COMPLY w/ 2015 IBC R-9.5 CONTINUOUS. 3-1/2" 20 GA. MTL. STUDS @ 16" O.C. w/ 3-1/2" (R-13) FACED BATT INSULATION AND 1/2" GYP. BOARD AT INTERIOR. RUN STUDS TIGHT TO DECK ABOVE.
	INTERIOR WALL - 8" REINFORCED PAINTED CMU w/ 3-1/2" 20 GA. MTL. STUDS @ 16" O.C. w/ 3-1/2" (R-13) UNFACED BATT INSULATION AND 1/2" GYP. BD.
	INTERIOR WALL - 3-1/2" 20 GA. MTL. STUDS @ 16" O.C. w/ 3-1/2" (R-13) UNFACED BATT INSULATION w/ 1/2" GYP. BD. @ BOTH SIDES.
	INTERIOR WALL - 8" CMU

KEYNOTES
COMMUNITY CENTER

- ADA ACCESSIBLE HIGH/LOW WATER FOUNTAIN w/ SPOUT MTD. 36" MAX. A.F.F. SEE PLUMBING PLANS FOR FIXTURE INFORMATION.
- TACTILE EXIT SIGN MOUNTED ON STRIKE SIDE OF DOOR @ 48"-60" A.F.F. TO BRAILLE.
- CLEAR LEXAN POLYCARBONATE CORNER GUARD - 48" IN LENGTH WITH 2" WINGS.
- 4" CONCRETE WALK ON 4" CRUSHED STONE w/ W1.4xW1.4 6x6 WWF. PROVIDE CONTROL JOINTS @ 5'-0" O.C. & EXPANSION JOINT @ 20'-0" O.C. SEE CIVIL PLANS FOR LOCATIONS.
- 2A FIRE EXTINGUISHER MTD. 48" TO TOP FURNITURE SUPPLIED & INSTALLED BY OWNER.
- PROVIDE SOLID BLOCKING IN WALL FOR TVs OR SHELVING AS REQ. VERIFY FINAL LOCATION WITH OWNER.

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6/25/20	1	ISSUED FOR BID	2/28/20
1/8" = 1'-0"	2	ISSUE FOR PERMIT	6/25/20

NO.	NO.	NO.
1	2	3
DRAWN BY	CHECKED BY	PROJ. NO.
		17056

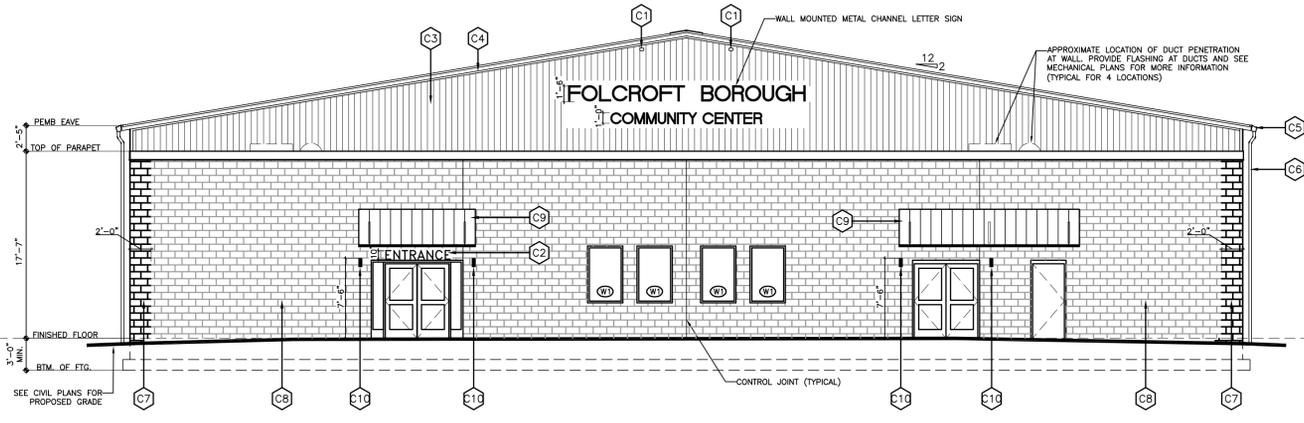
SHEET NO. A-11 OF SHEET



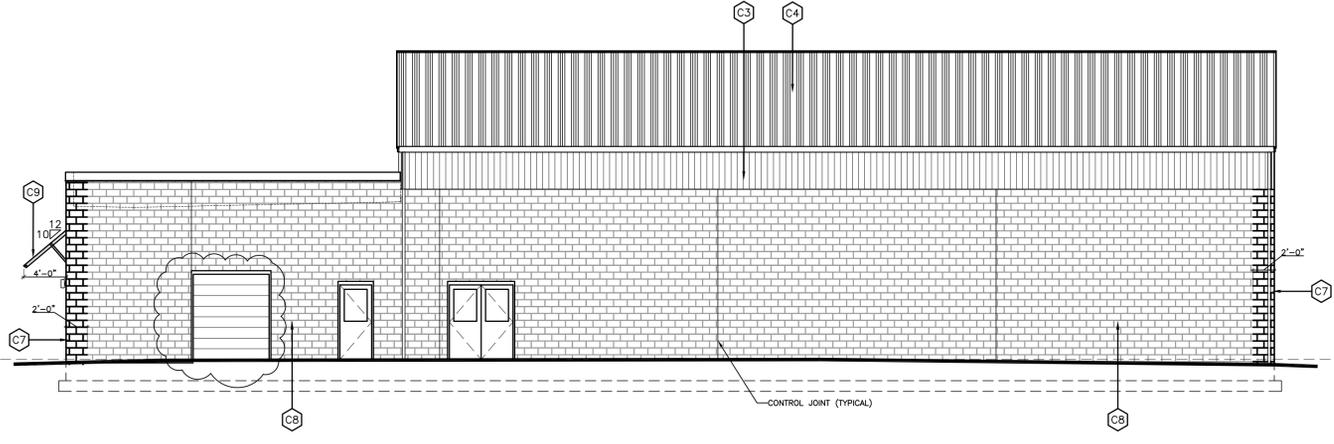
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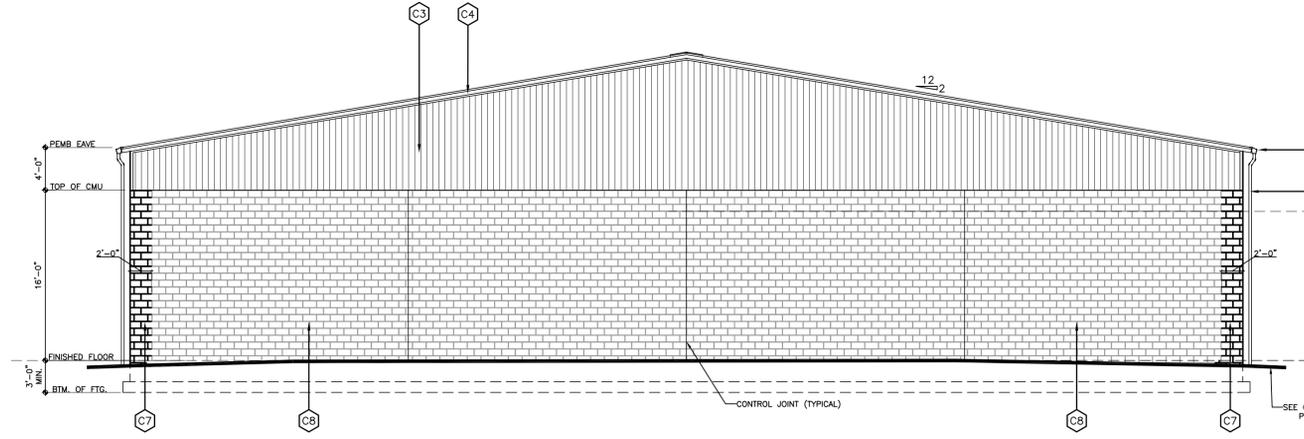
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 INTERIOR DESIGN



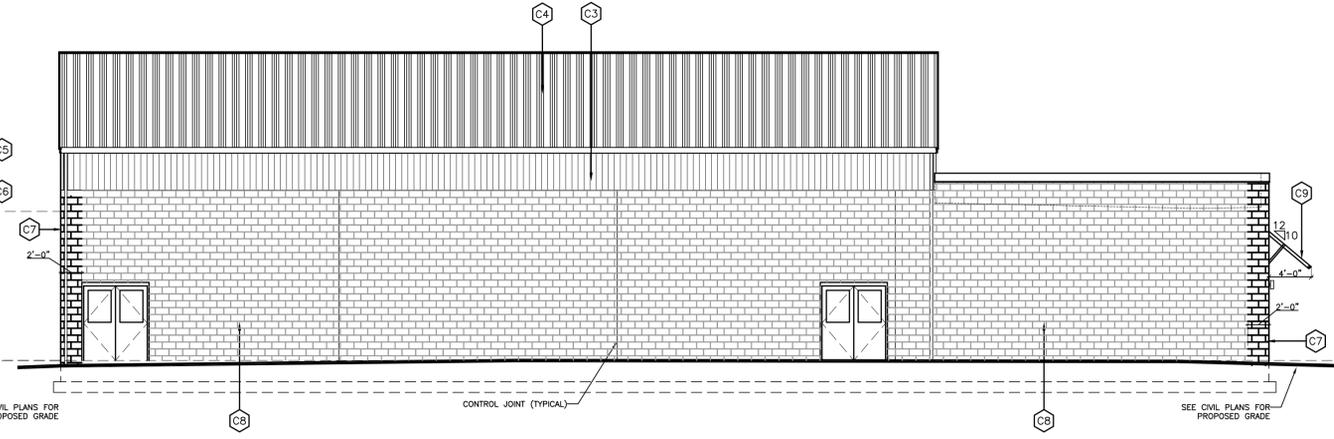
1 PROPOSED FRONT ELEVATION
 A-12 COMMUNITY CENTER SCALE: 1/8"=1'-0"



2 PROPOSED RIGHT SIDE ELEVATION
 A-12 COMMUNITY CENTER SCALE: 1/8"=1'-0"



3 PROPOSED REAR ELEVATION
 A-12 COMMUNITY CENTER SCALE: 1/8"=1'-0"



4 PROPOSED LEFT SIDE ELEVATION
 A-12 COMMUNITY CENTER SCALE: 1/8"=1'-0"

EXTERIOR FINISH SCHEDULE - COMMUNITY CENTER			
TAG	MATERIAL/ MANUF.	STYLE/COLOR	REMARKS
C1	WALL MOUNTED LED	COLOR SILVER	SEE ELECTRICAL PLANS FOR MORE INFO.
C2	ALUMINUM PIN MTD. LETTERS	COLOR BLACK	SEE ELEVATION FOR LETTER HEIGHT
C3	METAL WALL PANELS BY BUILDING MANUF.	COLOR WHITE	
C4	METAL ROOF PANELS BY BUILDING MANUF.	COLOR GREY	
C5	ALUMINUM GUTTER	COLOR WHITE	6" BOX
C6	.050 ALUM. RWC	COLOR WHITE	3"x4"
C7	GROUND FACE CMU/FIZZANO BROS	F-524	AT BUILDING CORNERS ONLY
C8	GROUND FACE CMU/FIZZANO BROS	F-515	
C9	STANDING SEAM METAL ROOF SYSTEM/ATAS OR APPROVED EQ.	DUTCH SEAM/MRD150/15" KYNAR 500 FINISH/BLACK	
E10	WALL MOUNTED UP/DOWN LED	COLOR WHITE	SEE ELECTRICAL PLANS FOR MORE INFO.

NOTES:
 1.) CONTRACTOR TO SUBMIT SAMPLES FOR APPROVAL
 2.) REFER TO SPECIFICATIONS FOR MORE INFORMATION

PROPOSED ELEVATIONS
 NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
 BOROUGH OF FOLCROFT
 ASHLAND AVE.
 FOLCROFT, PA 19032

REVISIONS	DATE	DESCRIPTION
NO. 1	2/28/20	ISSUED FOR BID
NO. 2	6/25/20	ISSUE FOR PERMIT

DATE: 6/25/20
 SCALE: 1/8"=1'-0"
 DRAWN BY: [blank]
 CHECKED BY: [blank]
 PROJ. NO.: 17086
 SHEET NO. **A-12** OF [blank]



LINN ARCHITECTS
 ARCHITECTURE
 ENGINEERING
 SITE PLANNING
 INTERIOR DESIGN

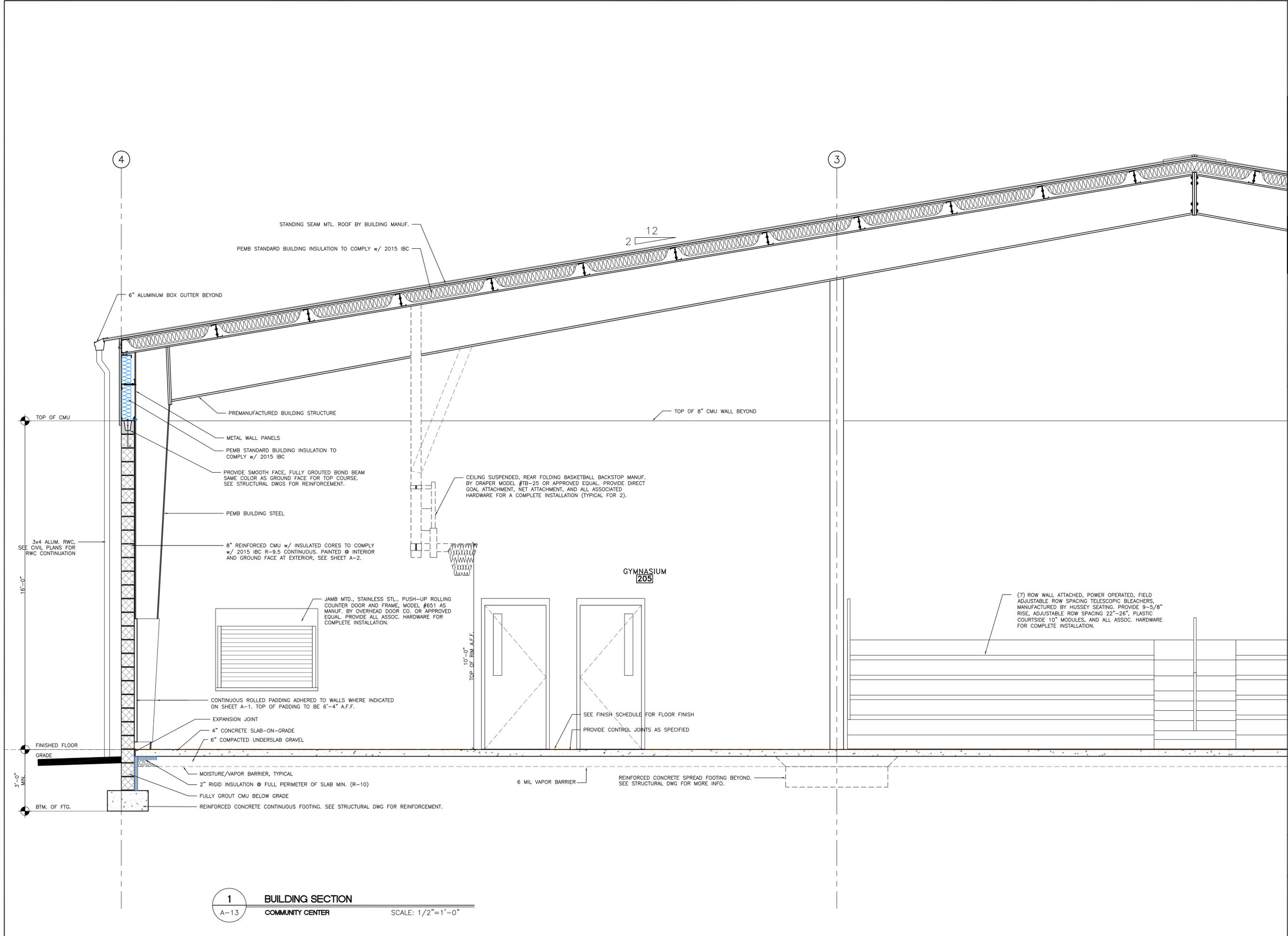
1140 N. PROVIDENCE ROAD
 MEDIA, PENNSYLVANIA 19063
 TEL: 610-566-7044
 FAX: 610-566-3258

LINN ARCHITECTS

BUILDING SECTION
 NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
 BOROUGH OF FOLCROFT
 ASHLAND AVE.
 FOLCROFT, PA 19032

NO.	DESCRIPTION	DATE
1	ISSUED FOR BID	2/28/20
2	ISSUE FOR PERMIT	6/25/20

DATE: 6/25/20
 SCALE: 1/8"=1'-0"
 DRAWN BY:
 CHECKED BY:
 PROJ. NO. 17086
A-13 OF SHEET



1
 A-13 **BUILDING SECTION**
 COMMUNITY CENTER SCALE: 1/2"=1'-0"



LINN ARCHITECTS
 140 N. PROVIDENCE ROAD
 MEDIA, PENNSYLVANIA 19063
 TEL: 610-566-7044
 FAX: 610-566-3258

LINN ARCHITECTS
 ARCHITECTURE
 ENGINEERING
 SITE PLANNING
 INTERIOR DESIGN

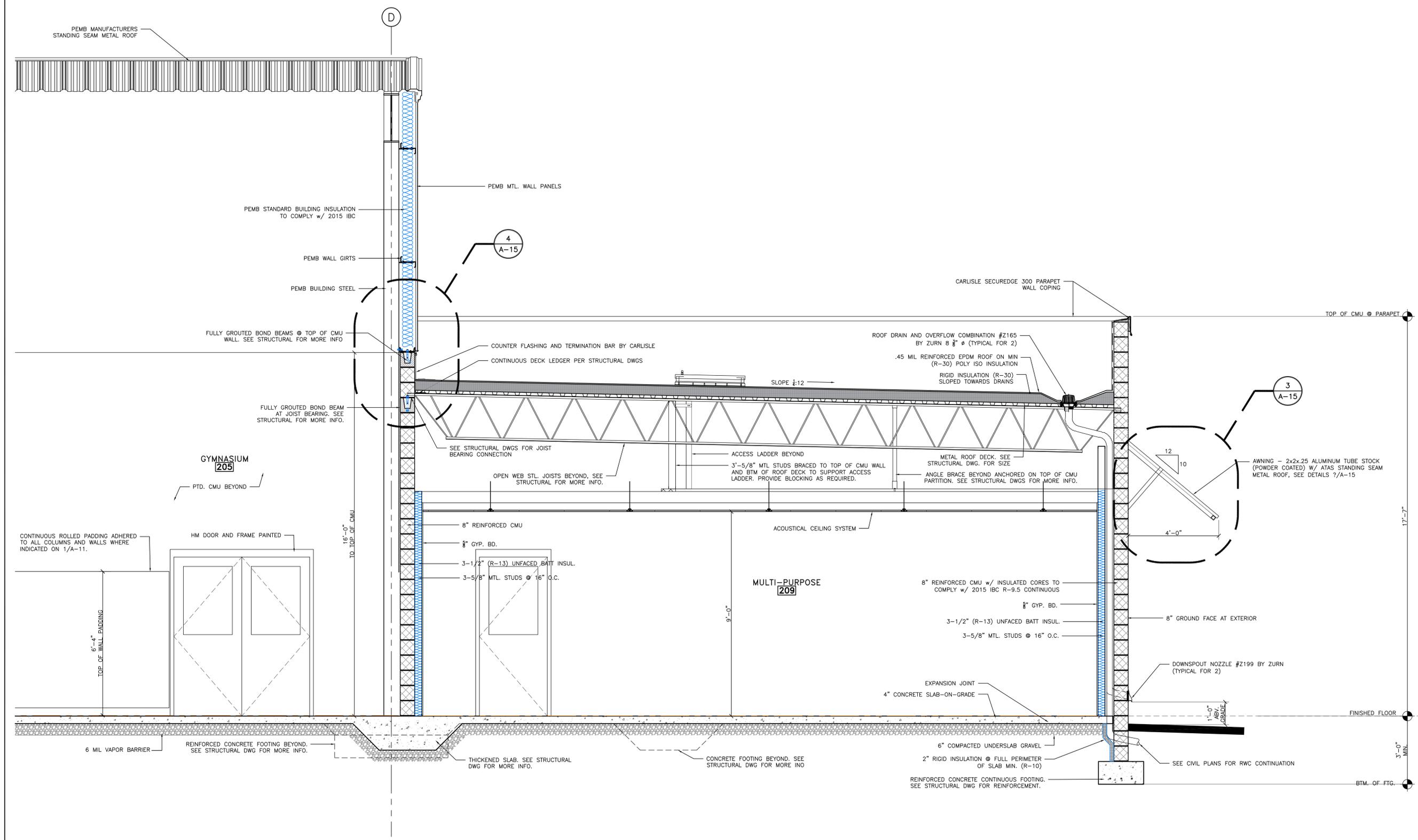
BUILDING SECTION
 NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
 BOROUGH OF FOLCROFT
 ASHLAND AVE.
 FOLCROFT, PA 19032

REVISIONS

NO.	DESCRIPTION	DATE
1	ISSUED FOR BID	2/28/20
2	ISSUE FOR PERMIT	6/25/20

DATE: 6/25/20
 SCALE: 1/8"=1'-0"
 DRAWN BY:
 CHECKED BY:
 PROJ. NO.: 17086

SHEET NO. **A-14** OF SHEET



1 BUILDING SECTION
 A-14 COMMUNITY CENTER SCALE: 1/2"=1'-0"



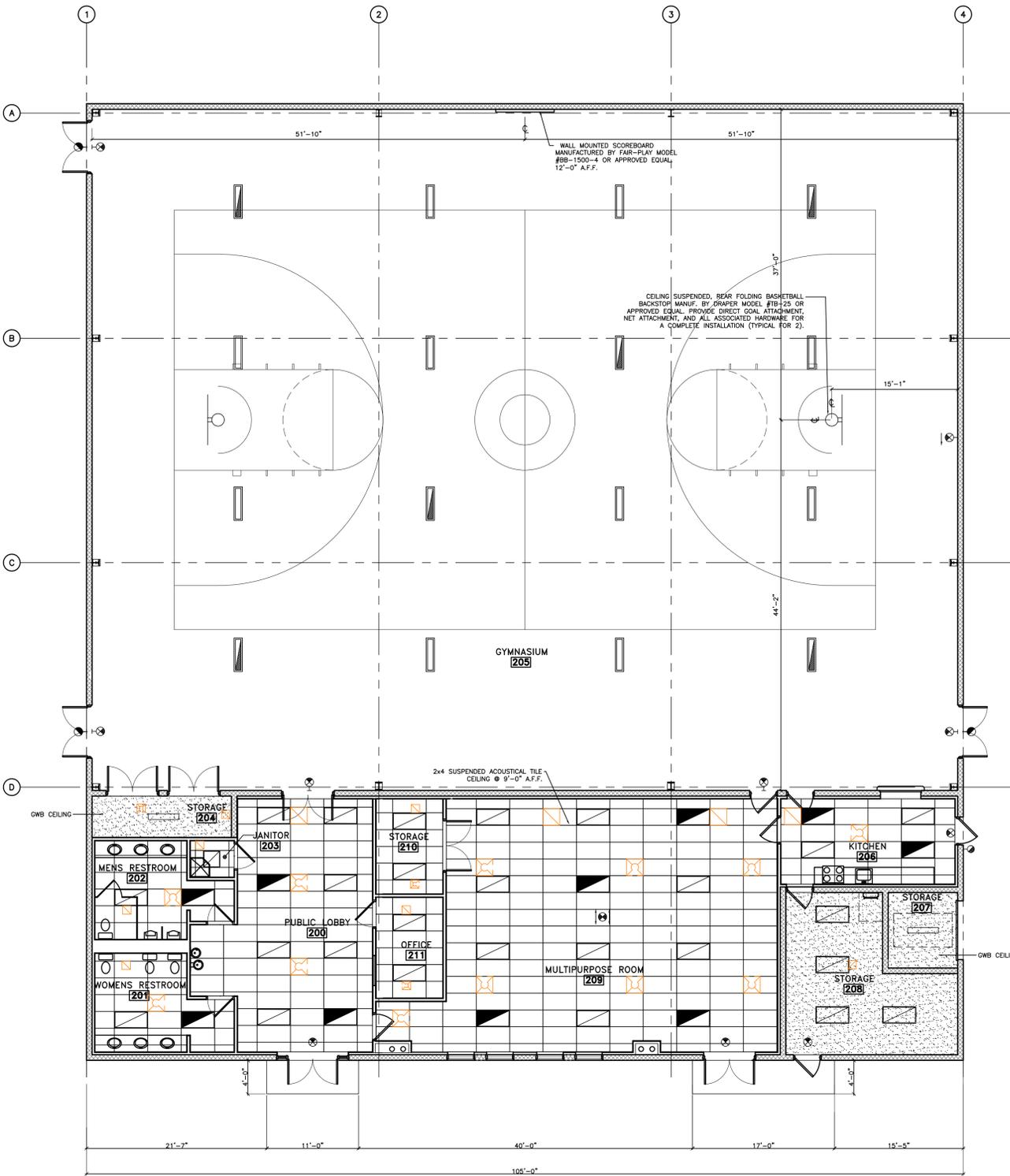
LINN ARCHITECTS
 ARCHITECTURE
 ENGINEERING
 SITE PLANNING
 INTERIOR DESIGN

1140 N. PROVIDENCE ROAD
 MEDIA, PENNSYLVANIA 19063
 TEL: 610-566-7044
 FAX: 610-566-3258

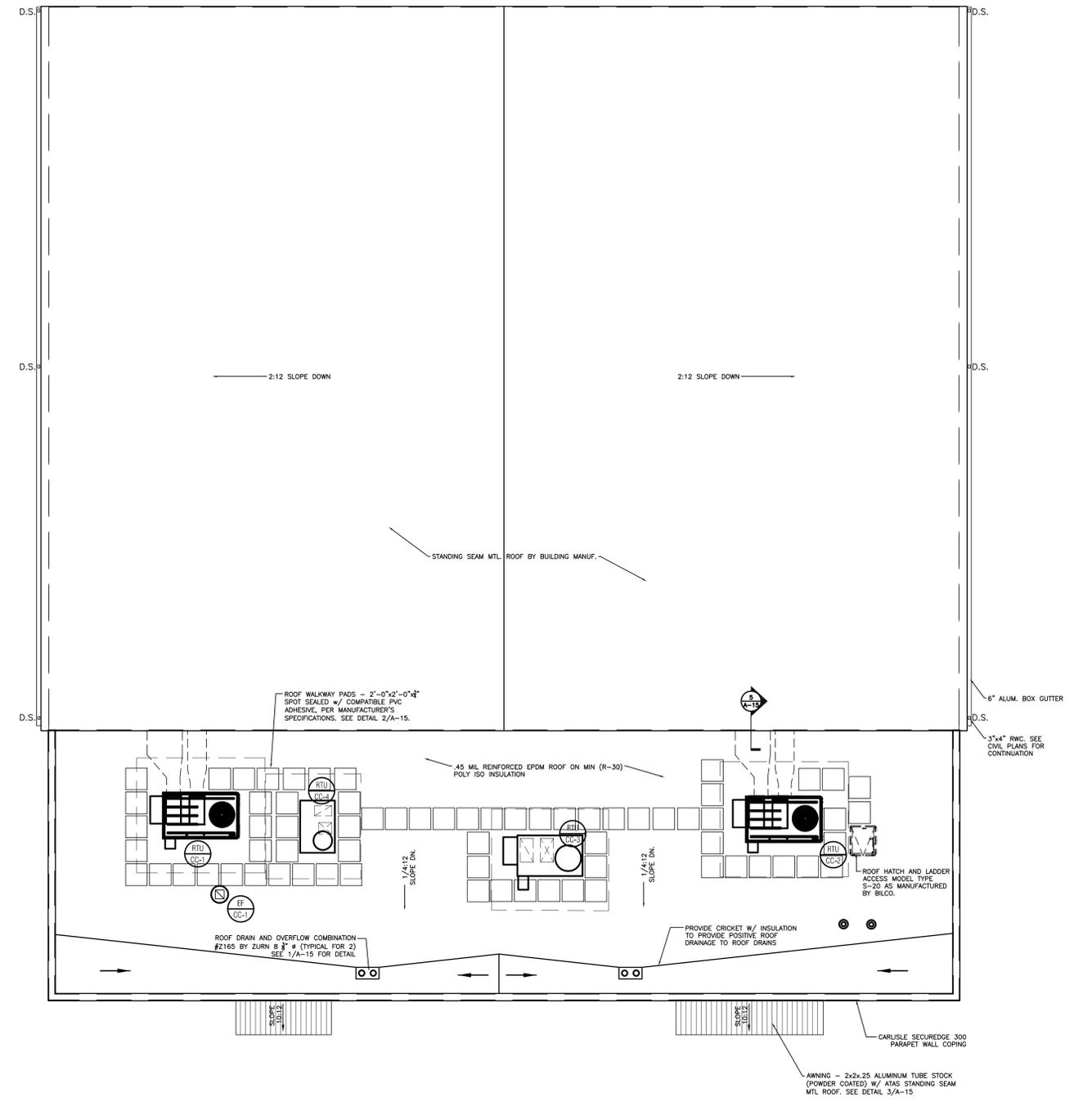
PROPOSED RCP AND ROOF PLAN
 NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
 BOROUGH OF FOLCROFT
 ASHLAND AVE.
 FOLCROFT, PA 19032

DATE	REVISIONS	DESCRIPTION	NO.
6/25/20 <td>1 <td>ISSUED FOR BID <td>1</td> </td></td>	1 <td>ISSUED FOR BID <td>1</td> </td>	ISSUED FOR BID <td>1</td>	1
6/25/20 <td>2 <td>ISSUE FOR PERMIT <td>2</td> </td></td>	2 <td>ISSUE FOR PERMIT <td>2</td> </td>	ISSUE FOR PERMIT <td>2</td>	2

DATE: 6/25/20
 SCALE: 1/8"=1'-0"
 DRAWN BY:
 CHECKED BY:
 PROJ. NO. 17096
 SHEET NO. **A-17** OF SHEET



1 PROPOSED REFLECTED CEILING PLAN
 A-17 COMMUNITY CENTER SCALE: 1/8"=1'-0"

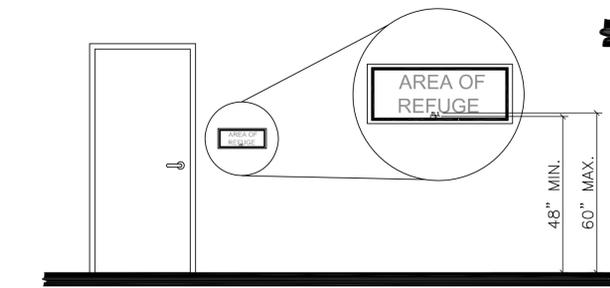


2 PROPOSED ROOF PLAN
 A-17 COMMUNITY CENTER SCALE: 1/8"=1'-0"

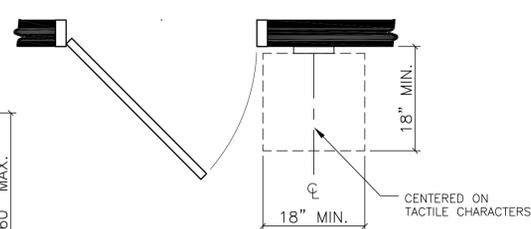
HARDWARE ITEM SCHEDULE			
ITEM NUMBER	DESCRIPTION	FINISH	MANUFACTURER
1A	PIVOT SET #147	626	RIXSON
1B	INTERMEDIATE PIVOT #M19	626	RIXSON
1C	HINGE, #1A714 FT	US32D	MCKINNEY
1D	ELECTROLYNX HARNESS, #QC OPTION	-	MCKINNEY
1E	HINGE (HEAVY WEIGHT), #TA386 FT x NRP	US32D	MCKINNEY
1F	HINGE (HEAVY WEIGHT), #TA386 FT x NRP x QC12	US32D	MCKINNEY
1G	HINGE, #1A714 FT x NRP	US32D	MCKINNEY
1H	HINGE, #1A786 FT x NRP	US32D	MCKINNEY
1I	HINGE, #1A786 FT x QC12	US32D	MCKINNEY
1J	HINGE, #1A714 FT x QC12	US32D	MCKINNEY
1K	HINGE, #1A714 FT x NRP	US32D	MCKINNEY
1L	HINGE, #1A714 FT x NRP	US32D	MCKINNEY
1M	HINGE, #1A786 FT	US32D	MCKINNEY
2A	STOREROOM/CLOSET LOCK, #DG3 8204 CRNJ MK	US26D	SARGENT
2B	OFFICE/ENTRY LOCK, #DG3 8205 CRNJ x LB THUMB TURN x MK	US26D	SARGENT
2C	CLASSROOM LOCK, #DG3 8237 CRNJ MK	US26D	SARGENT
2D	PRIVACY LOCK, #49 8265 CRNJ x LB THUMB TURN	US26D	SARGENT
2E	CLASSROOM DEADLOCK, #DG3 4877 MK	US26D	SARGENT
2F	PASSAGE LATCH, #8215 CRNJ	US26D	SARGENT
2G	SINGLE DUMM TRIM, #8293 CRNJ	US26D	SARGENT
2H	ROLLER LATCH, #592	US26D	ROCKWOOD
3A	CONCEALED VERTICAL ROD EXIT, EXIT ONLY, #DG3 16 43 AD8410 ED MK	US32D	SARGENT
3B	CONCEALED VERTICAL ROD EXIT, NIGHTLATCH, #DG3 16 43 AD8410 x 106 x LESS PULL x MK	US32D	SARGENT
3C	EXIT DEVICE (CARD READER), #DG3 43 M1-8876-(B/F) IPS ETNJ MK	US32D	SARGENT
3D	RIM EXIT DEVICE, STOREROOM, #DG3 43 8804 LESS PULL MK	US32D	SARGENT
3E	EXIT DEVICE (CARD READER), #DG3 43 M1-82271(B/F) IPS CRNJ MK	US26D	SARGENT
3F	SURFACE VERTICAL ROD EXIT, CLASSROOM, #DG3 43 NB8713 ETNJ MK	US32D	SARGENT
4A	SURFACE CLOSER, #MC 281 02/OZA x (MTG. PLATE AS APPLICABLE)	EN	SARGENT
4B	SURFACE CLOSER, #MC 281 P3	EN	SARGENT
4C	SURFACE CLOSER, #MC 281 O/P9	EN	SARGENT
4D	SURFACE CLOSER, #MC 281 CPS	EN	SARGENT
4E	SURFACE CLOSER, #MC 281 P10	EN	SARGENT
5A	PULL, #RM2240-48 MTG-TYPE 12XHD	US32D	ROCKWOOD
5B	DOOR PULL, #RM3301-24	US32D	ROCKWOOD
5C	PUSH PLATE, #RM1030H	US32D	ROCKWOOD
6A	OVERHEAD STOP, #1-x36 (HEAVY DUTY CONCEALED)	630	RIXSON
6B	WALL STOP, #RMB61-RMB55	US32D	ROCKWOOD
6C	SILENCER, #608-RKW	-	ROCKWOOD
6D	SURFACE OVERHEAD STOP, #9-x36	652	RIXSON
6E	SURFACE OVERHEAD STOP, #10-x36	652	RIXSON
6F	CONCEALED OVERHEAD STOP, #2-x36	652	RIXSON
7A	KICK PLATE, #K1050 10" HIGH x CSK x BEV	US32D	ROCKWOOD
8A	FLUSH BOLT, #555	US26D	ROCKWOOD
8B	DUST PROOF STRIKE, #570	US26D	ROCKWOOD
9A	THRESHOLD, #273x3AFG x MSES25SS	-	PEMCO
10A	WEATHER STRIP (INTEGRAL WITHIN CONSTRUCTION OF ALUMINUM DOOR AND FRAME ASSEMBLY)	-	-
10B	ASTRAGAL, #293242CNB x TKSP	-	PEMCO
10C	WEATHER STRIP, #2891APK x TKSP	-	PEMCO
11A	DOOR BOTTOM, #217AV x TKSP	-	PEMCO
12A	POSITION SWITCH, #DPS-M-BK	-	SECURITRON
13A	POWER SUPPLY, #AQD4	-	SECURITRON
14A	POWER DISTRIBUTION BOARD, #PDB-8C1R2	-	SECURITRON
15A	ELECTRIC POWER TRANSFER, #EL-SEPT	-	SECURITRON

* X = HANDING

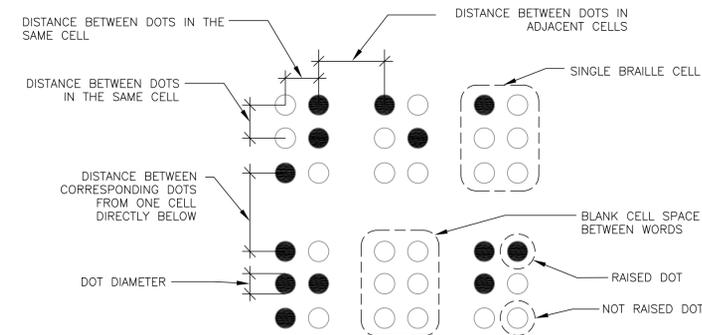
DOOR AND FRAME SCHEDULE																																					
NO.	DOOR				FRAME				HEAD & JAMB	HINGES	LOCKS	EXIT DEVICES	CLOSERS	PUSH/PULL	STOPS	KICK PLATE	BOLTS	THRESHOLD	WEATHER STRIPPING	DOOR BOTTOM	POWER	DOOR NO.	REMARKS														
	W	H	T	TYPE	MAT.	CORE	GLASS	FOB																TYPE	MAT.	GLASS	QT.	ITEM	QT.	ITEM	QT.	ITEM	QT.	ITEM	QT.	ITEM	QT.
100-1	(2)	3'-0"	7'-0"	1'-0"	D	ALUM.	INSUL.	TEMP.	-	6	ALUM.	2/2	1A/1B	-	-	1/1	3A/3B	1	4A	2	5A	2	6A	-	-	1	9A	-	2	11A	2	12A	100-1				
100-2		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	-	1	HM	3	1M	-	-	1	4B	1	5A	1/3	6A/6C	1	7A	-	-	-	-	-	-	-	-	100-2					
100-3		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	2/1/3	1H/1A/1D	-	-	1	3C	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	100-3	PROVIDE ONE-WAY GLASS				
100-4		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3/2/3	1C/1D/1K	-	-	1	3E	1	4C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	100-4	PROVIDE ONE-WAY GLASS				
101-1		3'-0"	7'-0"	1'-0"	D	ALUM.	INSUL.	TEMP.	●	2	ALUM.	1/1	1A/1B/1D	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	101-1					
101-2	(2)	3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	5	HM	6	1L	1/1/1	2E/2G/2H	-	-	-	-	2/2	6C/6E	2	7A	2/1	8A/8B	-	-	1	9A	1	10A	1	11A	1/1/1	13A/14A/15A	101-2	
101-3	(2)	3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	5	HM	6	1L	1/1/1	2E/2G/2H	-	-	-	-	2/2	6C/6E	2	7A	2/1	8A/8B	-	-	-	-	-	-	-	-	101-3			
101-4		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	-	1	HM	3	1C	1	2C	-	-	-	-	1	4C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	101-4				
102-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	1	HM	3	1C	1	2D	-	-	-	-	1	4D	3	6C	1	7A	-	-	-	-	-	-	-	102-1	COAT HOOK, RM802 QTY:1 US26D RO			
103-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	1	HM	3	1C	1	2D	-	-	-	-	1	4D	3	6C	1	7A	-	-	-	-	-	-	-	103-1				
104-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	1	HM	3	1C	1	2A	-	-	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	104-1			
105-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	1	HM	3	1C	1	2D	-	-	-	-	1	4D	3	6C	1	7A	-	-	-	-	-	-	-	105-1				
106-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3	1C	1	2C	-	-	-	-	1	4C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	106-1				
106-2		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3	1C	1	2C	-	-	-	-	1	4C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	106-2				
107-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3	1M	-	-	-	-	-	-	1	4C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	107-1				
110-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3	1C	1	2B	-	-	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	110-1			
111-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3	1C	1	2B	-	-	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	111-1			
112-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	1	HM	3	1C	1	2B	-	-	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	112-1			
113-1		3'-0"	7'-0"	1'-0"	C	HM	INSUL.	TEMP.	-	1	HM	3	1F	-	-	1	3D	1	4A	1	5A	1	6A	1	7A	-	-	-	-	-	-	-	113-1	PROVIDE ONE-WAY GLASS			
114-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3	1C	1	2B	-	-	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	114-1			
115-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3	1C	1	2B	-	-	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	115-1			
116-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	●	1	HM	3	1M	-	-	-	-	-	1	4C	1/1	5B/5C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	116-1	4C: DELAYED ACTION		
117-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	●	1	HM	3	1M	-	-	-	-	-	1	4A	1/1	5B/5C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	117-1	4C: DELAYED ACTION		
119-1	(2)	3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	4	HM	6	1C	1	2A	-	-	-	-	1	4A	2	6A	2	7A	2	8A	1	9A	2/110B/10C	2	11A	2	12A	119-1		
120-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	1	HM	3	1C	1	2A	-	-	-	-	3/1	6C/6E	1	7A	-	-	-	-	-	-	-	-	-	-	-	120-1		
121-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	●	1	HM	3/2/3	1C/1D/1K	-	-	1	3E	1	4C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	-	121-1		
122-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	1	HM	3	1C	1	2B	-	-	-	-	3/1	6C/6E	1	7A	-	-	-	-	-	-	-	-	-	-	-	122-1		
122-2		3'-0"	7'-0"	1'-0"	B	HM	INSUL.	TEMP.	●	2	HM	3/2/1	1D/1E/1F	-	-	1	3C	1	4A	1	6A	1	7A	-	-	-	-	-	-	-	-	-	-	-	122-2	PROVIDE ONE-WAY GLASS	
123-1		3'-0"	7'-0"	1'-0"	A	WD	SC	TEMP.	-	1	HM	3	1C	1	2B	-	-	-	-	3/1	6C/6E	1	7A	-	-	-	-	-	-	-	-	-	-	-	123-1		
124-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3/2/3	1C/1D/1K	-	-	1	3E	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	-	124-1		
126-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3/2/3	1C/1D/1K	-	-	1	3E	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	-	126-1		
126-2		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3/2/3	1C/1D/1K	-	-	1	3E	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	-	126-2		
127-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3/2/3	1C/1D/1K	-	-	1	3E	1	4C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	-	127-1		
127-2		3'-0"	7'-0"	1'-0"	B	HM	INSUL.	TEMP.	●	2	HM	3/2/3	1C/1D/1K	-	-	1	3E	1	4C	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	-	127-2		
128-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3/2/3	1C/1D/1K	-	-	1	3E	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	-	128-1		
129-1		3'-0"	7'-0"	1'-0"	A	WD	INSUL.	TEMP.	-	2	HM	3	1C	1	2A	-	-	-	-	1/3	6B/6C	1	7A	-	-	-	-	-	-	-	-	-	-	-	129-1		
130-1		3'-0"	7'-0"	1'-0"	B	HM	INSUL.	TEMP.	●	2	HM	2/2/1	1C/1D/1K	-	-	1	3E	1	4D	3	6C	1	7A	-	-	-	-	-	-	-	-	-	-	-	130-1		
130-2		3'-0"	7'-0"	1'-0"	C	HM	INSUL.	TEMP.	-	2	HM	2/2/1	1D/1E/1F	-	-	1	3C	1	4A	1	6A	1	7A	-	-	-	-	-	-	-	-	-	-	-	130-2		
131-1		3'-0"	7'-0"	1'-0"	A	HM	INSUL.	TEMP.	●	2	HM	3	1C	1	2A	-	-	-	-	2/1	6C/6E	1	7A	-	-	-	-	-	-	-	-	-	-	-	131-1		
133-1		3'-0"	7'-0"	1'-0"	B	WD	SC	TEMP.	●	1	HM	3/2/3	1C/1D/1K	-																							



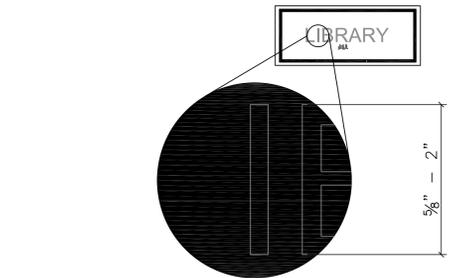
1 TYPICAL HEIGHT OF TACTILE CHARACTERS
A-19 SCALE: N.T.S.



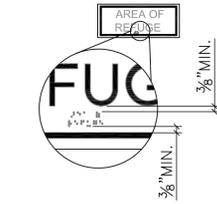
2 TYPICAL LOCATION OF TACTILE SIGNS
A-19 • DOORS SCALE: N.T.S.



4 TYPICAL BRAILLE MEASUREMENT
A-19 SCALE: N.T.S.



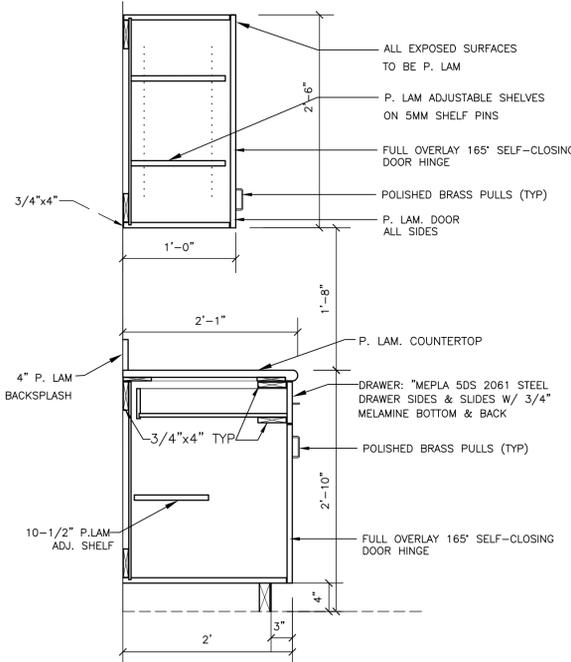
5 TYPICAL CHARACTER HEIGHT
A-19 SCALE: N.T.S.



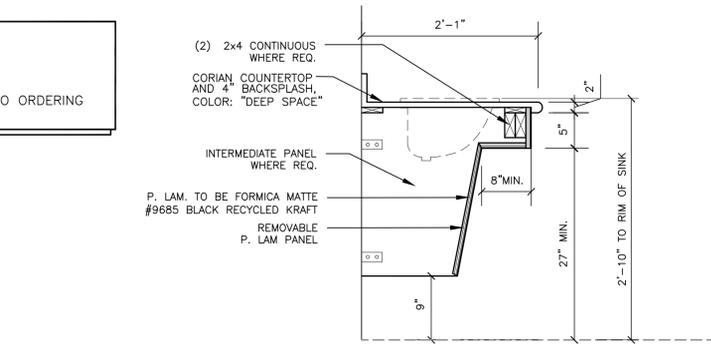
6 TYPICAL POSITION OF BRAILLE
A-19 SCALE: N.T.S.

NOTE: BRAILLE SHALL BE 48" MINIMUM & 60" MAXIMUM ABOVE FINISH FLOOR, MEASURED TO THE BASELINE OF THE BRAILLE CELLS.

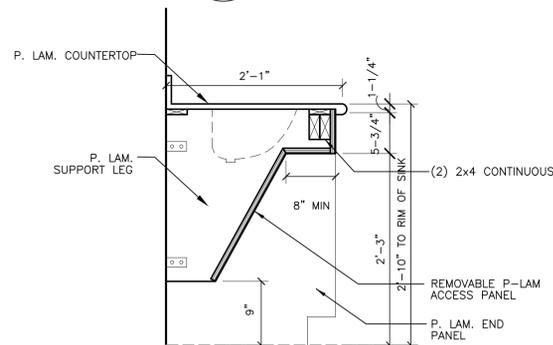
- CASEWORK NOTES:**
1. ALL INTERIOR SURFACES TO BE PLASTIC LAMINATE
 2. ALL COUNTERTOPS TO RECEIVE 4" BACKSPASH
 3. ALL CABINETS TO INCLUDE SHELVES
 4. CONTRACTOR TO CONFIRM ALL APPLIANCES WITH OWNER PRIOR TO ORDERING
 5. PROVIDE ADA APPROVED INSULATION ON ALL EXPOSED PIPING



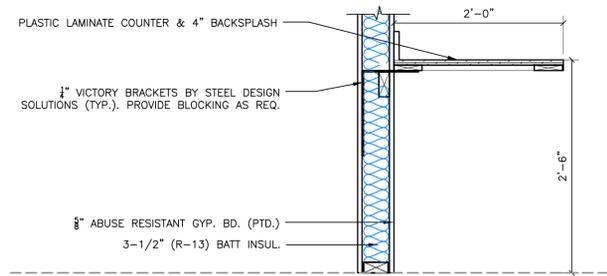
7 TYPICAL CASEWORK DETAIL
A-19 COMMUNITY CENTER SCALE: 1"=1'-0"



8 TYPICAL ADA SINK DETAIL
A-19 SCALE: 1"=1'-0"



9 TYPICAL ADA SINK DETAIL
A-19 SCALE: 1"=1'-0"



10 TYPICAL DESK DETAIL
A-19 SCALE: 1"=1'-0"

INTERIOR ROOM FINISH SCHEDULE

#	ROOM NAME	FLOORS MATERIALS	BASES MATERIALS	WALLS MATERIALS	CEILING		REMARKS
					MTG.	MTG.	
100	SECURE PUBLIC LOBBY	VCT 1	4" VINYL VENTED VINYL COVE	FRP	2x4 ACT 1		
101	COUNCIL CHAMBERS	VCT 2	CERAMIC BASE TILE 1	FRP	GYP. BD.		
102	RESTROOM	VCT 3	CERAMIC BASE TILE 2	FRP	GYP. BD.		
103	RESTROOM	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
104	JANITOR	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
105	RESTROOM	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
106	CONFERENCE	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
107	CODES	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
108	CORRIDOR	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
109	RECEPTION	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
110	TAX	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
111	FINANCE	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
112	FILES	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
113	BREAK ROOM	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
114	COUNCIL / MAYOR	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
115	BOROUGH MANAGER	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
116	WOMEN'S LOCKER	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
117	MEN'S LOCKER	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
118	K-9	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		VENTED VINYL AT 9'-0" A.F.F.
119	MECHANICAL	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
120	I.T.	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
121	EVIDENCE	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
122	SQUAD ROOM	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
123	BREAK ROOM	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
124	POLICE SECRETARY	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
125	CORRIDOR	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
126	SERGEANT	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
127	PROCESSING	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
128	ANTI CRIME / JUV.	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
129	STORAGE	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
130	SALLY PORT	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
131	JANITOR	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
132	HOLDING	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
133	CHIEF	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
134	INTERVIEW	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
135	INTERVIEW	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
136	RECEPTION	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
200	PUBLIC LOBBY	VCT 1	4" VINYL VENTED VINYL COVE	FRP	2x4 ACT 1		
201	WOMEN'S RESTROOM	VCT 2	CERAMIC BASE TILE 1	FRP	GYP. BD.		
202	MEN'S RESTROOM	VCT 3	CERAMIC BASE TILE 2	FRP	GYP. BD.		
203	JANITOR	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
204	STORAGE	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
205	GYMNASIUM	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
206	KITCHEN	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
207	STORAGE	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		
208	STORAGE	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
209	MULTIPURPOSE ROOM	CARPET TILE 3	CERAMIC BASE TILE 1	FRP	GYP. BD.		
210	STORAGE	CARPET TILE 2	CERAMIC BASE TILE 2	FRP	GYP. BD.		
211	OFFICE	CARPET TILE 1	CERAMIC BASE TILE 1	FRP	GYP. BD.		

MATERIAL SPECIFICATIONS				
ITEM	MANUFACTURER	STYLE OR CATALOG #	COLOR	REMARKS
FLOORS				
VCT 1	TARKETT	12"x12"x1/8"	TO BE SELECTED	
VCT 2	TARKETT	12"x12"x1/8"	TO BE SELECTED	
VCT 3	TARKETT	12"x12"x1/8"	TO BE SELECTED	
CARPET TILE 1	PATCRAFT	24"x24"	TO BE SELECTED	CONTACT TODD GOLDEN @ 610-312-8677
CARPET TILE 2	PATCRAFT	24"x24"	TO BE SELECTED	CONTACT TODD GOLDEN @ 610-312-8677
CARPET TILE 3	PATCRAFT	PASEO MODULAR 24"x24"/BEYOND THE DOOR COLLECTION	TO BE SELECTED	CONTACT TODD GOLDEN @ 610-312-8677
CERAMIC FLOOR TILE 1	DALTILE	2x2 MOSAIC	TO BE SELECTED	
CERAMIC FLOOR TILE 2	DALTILE	2x2 MOSAIC	TO BE SELECTED	
SPORTS VINYL	TARKETT SPORTS INDOOR	OMNISPORTS 9.4 MM	TO BE SELECTED	
BASE				
VINYL	TARKETT	4" COVE BASE	TO BE SELECTED	
VENTED VINYL	TARKETT	4" VENTED COVE BASE	TO BE SELECTED	
CERAMIC BASE TILE 1	DALTILE	4" COVE BASE	TO BE SELECTED	
BASE BOARD	TARKETT	REVEAL, 4-1/4"	TO BE SELECTED	
CERAMIC BASE TILE 2	DALTILE	6" COVE BASE	TO BE SELECTED	
WALLS				
CERAMIC WALL TILE 1	DALTILE	4x4 WALL TILE	TO BE SELECTED	
FRP	CRANE COMPOSITES	GLASBORD	TO BE SELECTED	
CHAIR RAIL	TARKETT	4" CHAIR RAIL	TO BE SELECTED	
CONTINUOUS ROLLED PADDING	DRAPER	6'-0" HIGH, ECO VISION	TO BE SELECTED	
PAINT WALLS	SHERWIN WILLIAMS	EGG SHELL FINISH	TO BE SELECTED	
PAINT DOOR/WINDOW FRAMES	SHERWIN WILLIAMS	SEMI GLOSS FINISH	TO BE SELECTED	
CEILING				
2x4 ACOUSTICAL TILE 1	ARMSTRONG	ULTIMA TEGULAR TILE	#1901 WHITE	w/ PRELUDE XL 15/16 GRID
PAINT CEILING	SHERWIN WILLIAMS	FLAT FINISH	TO BE SELECTED	

FINISH NOTES:

1. ALL FINAL FINISH SELECTIONS (COLORS, MATERIALS, ETC.) TO BE REVIEWED AND APPROVED BY ARCHITECT/OWNER.
2. ALL FINISHES AND BASE TO TERMINATE AT INSIDE CORNERS. G.C. TO REVIEW FINISH LOCATIONS WITH ARCHITECT PRIOR TO INSTALLATION.
3. PROVIDE ADA ACCEPTABLE TRANSITION STRIPS AT ALL INTERSECTIONS OF DISSIMILAR FLOORING TYPES.
4. PROVIDE MOISTURE RESISTANT GYP. BOARD AT ALL WALLS ADJACENT TO PLUMBING FIXTURES (SINKS, TOILETS, SHOWERS).
5. ALL ALUMINUM AND HOLLOW METAL DOORS AND WINDOW FRAMES TO BE PAINTED. FRAMES SHALL RECEIVE PRIMER COAT AND (2) FINISH COATS OF SEMI-GLOSS ENAMEL PAINT.
6. ALL NEW WALLS TO BE PAINTED WITH (1) COAT OF PRIMER/SEALER AND (2) COATS OF PAINT. EXPOSED BLOCK SHALL RECEIVE BLOCK FILLER AND (1) COAT OF PAINT.



LINN ARCHITECTS
ARCHITECTURE
ENGINEERING
SITE PLANNING
INTERIOR DESIGN
140 N. PROVIDENCE ROAD
MEDIA, PENNSYLVANIA 19063
TEL: 610-566-7044
FAX: 610-566-3258

INT. FINISH SCHEDULE, ADA REQ.S. & CASEWORK

NEW MUNICIPAL BUILDINGS - BOROUGH BUILDING

BOROUGH OF FOLCROFT

ASHLAND AVE.

FOLCROFT, PA 19032

REVISIONS	DATE	DESCRIPTION
NO. 1	6/25/20	ISSUED FOR BID
NO. 2	6/25/20	ISSUE FOR PERMIT

DATE: 6/25/20
SCALE: 1"=1'-0"
DRAWN BY: []
CHECKED BY: []
PROJ. NO.: 17086
SHEET NO. **A-19** OF []

GENERAL NOTES

CODES & STANDARDS

- INTERNATIONAL BUILDING CODE - 2015 IBC
- AMERICAN SOCIETY OF CIVIL ENGINEERS - ASCE 7-10
- AMERICAN CONCRETE INSTITUTE - ACI 318-14
- AMERICAN INSTITUTE OF STEEL CONSTRUCTION - AISC 360-10
- SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS - AISC 341-10
- AMERICAN WELDING SOCIETY - AWS D1.4/D1.4M-2011
- INTERNATIONAL MASONRY INSTITUTE TMS 402-13/ACI 530-11/ASCE 5-13, TMS 602-13/ACI 530.1-13/ASCE 6-13
- ASTM STANDARDS FOR THE MATERIALS SPECIFIED

DESIGN & STRUCTURAL CRITERIA

- PROJECT LOCATION/LOCAL JURISDICTION: FOLCROFT, PA 19032
- RISK CATEGORY: CATEGORY II - FOR DETERMINATION OF LOADING, IMPORTANCE & OTHER STRUCTURAL ENGINEERING DESIGN FACTORS.
- SOIL DESIGN CRITERIA
 - FROST DEPTH: 36 INCHES
 - ALLOWABLE BEARING PRESSURE: 1500 PSF (ASSUMED)
 - COEFFICIENT OF FRICTION: .35 (ASSUMED)
 - ACTIVE EARTH PRESSURE: 30 PSF/FT (ASSUMED)
 - AT-REST EARTH PRESSURE: 60 PSF/FT (ASSUMED)
 - PASSIVE EARTH PRESSURE: 250 PSF/FT (ASSUMED)
- DEAD LOADS
 - ROOF DEAD LOAD: PEMB = 10 PSF
ROOF DEAD LOAD: LOW ROOF = 20 PSF
- LIVE LOADS
 - ROOF LIVE LOAD: 20 PSF
- WIND DESIGN CRITERIA
 - BASIC WIND SPEED: $V_{ult} = 115$ MPH
 - EXPOSURE B
 - INTERNAL PRESSURE COEFFICIENT: +/- 0.18
 - COMPONENT & CLADDING WIND PRESSURE: SEE CHART
- SEISMIC DESIGN CRITERIA
 - $S_s = 0.201g$, $S_1 = 0.06g$
 - SITE SOIL CLASSIFICATION: D
 - $SDS = 0.215g$, $SD1 = 0.096g$
 - SEISMIC DESIGN CATEGORY B
 - SEISMIC DESIGN RESPONSE COEFFICIENT: $C_s = (0.11$ LOW MASONRY BLDG) | MAIN BUILDING BY PEMB MANUFACTURER
 - ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE AND/OR BY PEMB LATERAL FORCE RESISTING SYSTEM = ORDINARY REINFORCED MASONRY SHEAR WALLS (SIDE MASONRY BLDG R = 2) | MAIN BUILDING BY PEMB MANUFACTURER
- SNOW DESIGN CRITERIA
 - GROUND SNOW LOAD (Pg): 20 PSF
 - DESIGN SNOW LOAD (Pf): 20 PSF
 - EXPOSURE FACTOR, C_e : BY OTHERS
 - THERMAL FACTOR, C_t : BY OTHERS
 - IMPORTANCE FACTOR, I_s : BY OTHERS
 - ROOF SLOPE FACTOR, C_s : BY OTHERS
 - EXPOSURE BY OTHERS
 - DRIFTING: BY OTHERS
 - UNBALANCED: BY OTHERS
- DEFLECTION
 - ROOF TOTAL LOAD: L/240
 - ROOF LIVE LOAD: L/360
 - LATERAL SYSTEMS: L/180

MISCELLANEOUS

- REFERENCE CIVIL DRAWINGS FOR EQUIPMENT LOCATION AND ORIENTATION ON THE SITE. THE CONTRACTOR AND SUB-TRADES SHALL FURNISH ALL REQUIRED MATERIAL, LABOR, EQUIPMENT AND PERFORM ALL WORK AS NECESSARY, AS INDICATED ON THE PROJECT DOCUMENTS, OR AS REASONABLY INFERRED TO EXECUTE THE SCOPE OF WORK FOR A PROPERLY FINISHED, COMPLETE JOB.
- THE QUALITY OF WORKMANSHIP SHOULD BE SET AND SUPERVISED BY THE CONTRACTOR TO PASS BUILDING DEPT. OR ENGINEER INSPECTION FOR ROUGH CONSTRUCTION. THE LEVEL OF QUALITY AND TOLERANCE SHOULD BE APPROPRIATE FOR THE INSTALLED ELEMENT TO RECEIVE THE NEXT IN-LINE FINISH ASPECT OF CONSTRUCTION.
- THE PURPOSE OF PROJECT DRAWINGS IS TO DEPICT THE OVERALL SCOPE OF THE PROJECT. THE PROJECT DRAWINGS HAVE BEEN DEVELOPED TO SHOW A LEVEL OF DETAIL WITH THE OBJECTIVE OF PLAN CHECK APPROVAL AND ISSUANCE OF A BUILDING PERMIT. THIS MODERATE LEVEL OF DETAIL SHOULD ALLOW FOR A VARIETY OF STANDARD CONSTRUCTION METHODS AND SEQUENCES. THE PROJECT DRAWINGS ARE INTENDED TO COMPLY WITH THE ORDINANCES, RULES AND REGULATIONS OF THE JURISDICTION IN WHICH THE BUILDING IS LOCATED.
- THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNOLOGIES, SEQUENCES AND PROCEDURES.
- CONSTRUCTION MATERIAL SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS OR ROOF. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT.
- WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDUM.
- OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. THEY SHALL BE RESPONSIBLE FOR ALL CHANGES NECESSARY IF THEY CHOOSE AN OPTION AND THEY SHALL COORDINATE ALL DETAILS.
- NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT.
- TYPICAL DETAILS ARE NOT CUT ON DRAWINGS, BUT APPLY UNLESS NOTED OTHERWISE.
- IN THE CASE OF DISCREPANCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS, PLANS/DETAILS OR REFERENCE STANDARDS, THE ARCHITECT/ENGINEER SHALL DETERMINE WHICH SHALL GOVERN. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. SHOULD ANY DISCREPANCY BE FOUND IN THE CONTRACT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK, UNLESS PRIOR TO THE SUBMISSION OF THE PRICE, THE CONTRACTOR ASKS FOR A DECISION FROM THE ARCHITECT AS TO WHICH SHALL GOVERN. ACCORDINGLY, ANY CONFLICT IN OR BETWEEN THE CONTRACT DOCUMENTS SHALL NOT BE A BASIS FOR ADJUSTMENT IN THE CONTRACT PRICE.
- VISITS TO THE JOBSITE BY THE ENGINEER TO OBSERVE CONSTRUCTION DO NOT IN ANY WAY MEAN THAT THEY ARE THE GUARANTORS OF THE CONTRACTOR'S WORK, NOR SUPERVISION, NOR SAFETY AT THE JOBSITE.
- REVIEW OF SHOP DRAWINGS BY THE ENGINEER IS FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. REVIEW OF SUCH SHOP DRAWINGS BY THE ENGINEER SHALL NOT RELIEVE THE CONTRACTOR FROM RESPONSIBILITY FOR CORRECTNESS OF DIMENSIONS, FABRICATION DETAILS, SPACE REQUIREMENTS, AND ERRORS IN THE SHOP DRAWINGS, OR FOR DEVIATIONS FROM THE CONTRACT DRAWINGS OR SPECIFICATIONS UNLESS THE CONTRACTOR HAS SPECIFICALLY CALLED ATTENTION TO SUCH DEVIATIONS IN WRITING BY A LETTER ACCOMPANYING THE SHOP DRAWINGS AND THE ENGINEER APPROVES SUCH CHANGE OR DEVIATION IN WRITING.
- THE CONTRACTOR IS RESPONSIBLE FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK THAT CONFORMS TO THE REGULATIONS OF THE OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) SAFETY AND HEALTH STANDARDS FOR THE CONSTRUCTION INDUSTRY.
- ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING WITH APPROPRIATE TRADES, DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION. DO NOT PENETRATE ANY STRUCTURAL ELEMENTS (BEAMS, COLUMNS, WALLS, SLABS, STEEL DECK, ETC.) WITHOUT PRIOR WRITTEN APPROVAL OF STRUCTURAL ENGINEER THROUGH ARCHITECT.
- ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF A CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.
- CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS SHOWN ON STRUCTURAL DRAWINGS WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS. DO NOT SCALE DRAWINGS, USE FIGURED DIMENSIONS ONLY.

MATERIALS

STRUCTURAL STEEL	W & WT CHANNEL & ANGLE PLATES HSS SQ OR RECT HSS ROUND	ASTM A992, Fy = 50 KSI ASTM A36, Fy = 36 KSI ASTM A36, Fy = 36 KSI ASTM A500, GR B, Fy = 46 KSI ASTM A500, GR B, Fy = 42 KSI
CONCRETE	ALL (U.N.O.)	PORTLAND CEMENT ASTM C150 TYPE II W/C RATIO = 0.45 MAXIMUM 28 DAY Fc = 4,000 PSI SLUMP RANGE = 3-5 INCHES AIR CONTENT = 4.5 - 7.5% 3/4" MAXIMUM NORMAL WEIGHT AGGREGATE
REINFORCING BARS		ASTM A615, GRADE 60 (NON-WELDABLE) ASTM A706, GRADE 60 (WELDABLE) ASTM A-1064 (WELDED WIRE REINF)
ANCHOR RODS		ASTM F1554, GRADE 36 MIN
WELD METAL		E70XX ELECTRODE
ADHESIVE ANCHORS	MASONRY	HILTI HIT HY 270 OR SIMPSON STRONG-TIE SET ANCHORING ADHESIVE
	CONCRETE	HILTI HIT HY-200
	ALL	ASTM A36 ALL-THREAD WITH CHISEL POINT
COLD FORMED STEEL		STEEL STUDS, ASTM A653, 18 GA. MIN Fy = 33 KSI U.N.O. MIN
MASONRY		CMU BLOCK: NORMAL WEIGHT, Fc = 1900 PSI MORTAR: PORTLAND CEMENT/LIME, TYPE M OR S GROUT: Fc = 2000 PSI (28 DAYS) WALL COMPRESSIVE STRENGTH = Fm = 1500 PSI (28 DAYS)

CONCRETE

- CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301, "STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE" AND ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE". ALL REINFORCING SHALL CONFORM TO THE CRSI SPECIFICATIONS & HANDBOOK. CONCRETE PLACEMENT SHALL MEET ALL COLD WEATHER AND HOT WEATHER REQUIREMENTS OUTLINED IN ACI 306 & 305 RESPECTIVELY.
- ADDITION OF WATER TO THE BATCH FOR MATERIAL WITH INSUFFICIENT SLUMP WILL NOT BE PERMITTED, UNLESS THE SUPPLIER HAS SPECIFICALLY WITHHELD WATER FROM THE BATCH AT THE PLANT. IN SUCH CASE THE MIX DESIGN AND TRUCK TICKET MUST CLEARLY STATE THE MAXIMUM AMOUNT OF WATER THAT CAN BE ADDED TO THE BATCH ON SITE. IN NO CASE SHALL THE DESIGN WATER TO CEMENTITIOUS MATERIAL RATIO BE EXCEEDED.
- CONCRETE CONTAINING SUPERPLASTICIZING ADMIXTURE SHALL HAVE A SLUMP OF 4" +/-- 1", TO BE FIELD VERIFIED, PRIOR TO ADDING ADMIXTURE, AND NOT EXCEEDING 8" AT PLACEMENT.
- MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED, INCLUDING SLABS ON GRADE AT 2'-0" O.C. AROUND UNDER-FLOOR DUCTS AND SLAB EDGES, REINFORCING, KEYS, ETC. MECHANICALLY VIBRATE ONLY THE TOP 5 FEET OF CAISSON CONCRETE.
- IF CONCRETE IS PLACED BY THE PUMP METHOD, SUPPORTS SHALL BE PRODUCED FOR THE HOSE. THE HOSE SHALL NOT BE ALLOWED TO CONTACT THE REBAR OR TENDONS. THIS REQUIREMENT IS MANDATORY. DISCHARGE SHALL BE DIRECTED SO AS TO PREVENT DISPLACEMENT OF REBAR, TENDONS, OR ACCESSORIES.
- REINFORCING SHALL BE CONTINUOUS AROUND ALL CORNERS AND THROUGH CONSTRUCTION JOINTS UNLESS SHOWN OTHERWISE.
- ALL HOOKS ON ALL BARS SHALL BE STANDARD 90 DEGREE HOOKS UNLESS SHOWN OTHERWISE.
- REINFORCING STEEL SHALL NOT BE BENT OR STRAIGHTENED IN A MANNER INJURIOUS TO THE CONCRETE OR STEEL.
- ALL REINFORCING TO BE WELDED SHALL BE WELDED IN ACCORDANCE WITH AWS D1.4. NO TACK WELDING OF REINFORCING BARS IS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE BY STRUCTURAL ENGINEER.
- ALL CONDUITS, GROUND WIRES, DRAINS, ANCHOR BOLTS, OTHER EMBEDDED ITEMS, ETC. SHALL BE IN PLACE BEFORE CONCRETE PLACEMENT.
- REINFORCING LAP SPLICES IN CONCRETE SHALL BE PER TYPICAL DETAIL UNLESS NOTED OTHERWISE. ALL SPLICE LOCATIONS ARE SUBJECT TO APPROVAL. PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT CORNERS AND INTERSECTIONS OF FOOTINGS AND WALLS.
- ALL FIELD BENDING OF REINFORCING SHALL BE STANDARD 90 DEGREE HOOKS AS DEFINED IN CURRENT ACI 318 UNLESS NOTED OR DETAILED OTHERWISE.
- WHEN TOTAL NUMBER OF REINFORCING BARS IS SHOWN ON DESIGN DRAWINGS AND SPACING IS NOT SPECIFIED, BARS SHALL BE EQUALLY SPACED.
- DETAILS OF REINFORCING NOT SHOWN IN THESE PLANS SHALL BE DONE IN ACCORDANCE WITH ACI 315 AND ACI 318.
- ALL SLABS-ON-GRADE SHALL HAVE CONTROL JOINTS CUT IN CONCRETE WITHIN 8 HOURS OF PLACEMENT AT A SPACING NO GREATER THAN 10' O.C.E.W. (U.N.O. ON PLANS).
- CONCRETE PROPERTIES (SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS):

CAST-IN-PLACE PROPERTIES		
	FOOTINGS/ FOUNDATION WALLS	SLABS ON GRADE/ SLABS ON METAL DECK
MINIMUM 28-DAY COMPRESSIVE STRENGTH	4000 PSI	4500 PSI
MAXIMUM WATER-CEMENT RATIO (BY WT)	0.42	0.43
MAXIMUM AGGREGATE SIZE	3/4"	3/4"
PERCENT AIR CONTENT	4.5% - 7.5%	4.5% - 7.5%
MAXIMUM SLUMP	4" **	3" **

- * AIR CONTENT OF SLABS-ON-GRADE MAY BE REDUCED TO 2% MIN IF THE SLAB WILL BE PROTECTED FROM FREEZE/THAW CYCLES DURING AND AFTER CONSTRUCTION.
- ** PROTECTED FROM FREEZE/THAW CYCLES DURING AND AFTER CONSTRUCTION. MAXIMUM SLUMP MAY BE INCREASED TO 8" WITH THE USE OF WATER-REDUCING ADMIXTURES TO MAINTAIN THE SPECIFIED W/C RATIO.

ANCHOR ROD NOTES

- ANCHOR ROD LOCATIONS AND DIAMETERS ARE PER BUILDING MANUFACTURER.
- ALL ANCHOR RODS SHALL BE EITHER HEADED OR DOUBLE NUT WITH 1/4"x2"x2" STEEL WASHER.
- MINIMUM EMBEDMENT:
 - 3/4" Ø RODS = 12"
 - 1" Ø AND PEMB ANCHOR RODS = 22"
- ANCHOR RODS SHALL BE ASTM F1554 GR 36 MATERIAL.
- ANCHOR NUTS SHALL BE INSTALLED SNUG TIGHT.
- EPOXY AND EXPANSION ANCHORS SHALL BE HILTI (OR EQUAL) INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS.

FOUNDATION AND SOIL PREPARATION

SITE GRADING AND EXCAVATIONS

- A GEOTECHNICAL ENGINEERING EVALUATION REPORT HAS NOT BEEN CONDUCTED FOR THIS PROJECT. THE CONTRACTOR IS THEREFORE RESPONSIBLE THAT ALL BUILDING CODE REQUIREMENTS ARE MET AND IN COMPLIANCE WITH THE LOCAL JURISDICTION. THE FOLLOWING NOTES ARE TYPICAL AND SHALL NOT GOVERN SITE SPECIFIC REQUIREMENTS.
- CONFORM TO IBC CHAPTER 18 "SOILS AND FOUNDATIONS".
- ALL TOPSOIL AND ORGANIC MATERIAL, ASPHALT, CONCRETE AND RELATED CONSTRUCTION DEBRIS SHALL BE REMOVED FROM THE PROPOSED BUILDING AND PAVEMENT AREAS AND ANY AREAS TO RECEIVE SITE GRADING FILL. FOR PLANNING PURPOSES, A MINIMUM STRIPPING THICKNESS OF 6 INCHES IS RECOMMENDED. THICKER STRIPPING DEPTHS MAY BE WARRANTED TO REMOVE ALL DETRIMENTAL ORGANICS AS DETERMINED ONCE ACTUAL STRIPPING OPERATIONS ARE PERFORMED.
- ALL FILL AND BACKFILL SHALL BE NON-EXPANSIVE, FREE OF ORGANICS AND DEBRIS AND SHALL BE APPROVED BY THE PROJECT GEOTECHNICAL ENGINEER. ALL FILL SHALL BE PLACED IN UNIFORM LIFTS NOT EXCEEDING 8 INCHES IN THICKNESS FOR FINE-GRAINED SOILS AND NOT EXCEEDING 12 INCHES FOR GRANULAR SOILS. ALL FILL AND BACKFILL SHALL BE COMPACTED TO THE FOLLOWING PERCENTAGES OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D698 OR EQUIVALENT (E.G. ASTM D4253-D4254).
 - BELOW FOUNDATIONS OR SPREAD FOOTINGS..... 95%
 - BELOW SLAB-ON-GRADE CONSTRUCTION..... 95%
 - BELOW STREETS, PARKING LOTS, OR OTHER PAVED AREAS..... 95%
 - GENERAL LANDSCAPING OR NONSTRUCTURAL AREAS..... 92%
 - UTILITY TRENCH BACKFILL, TO WITHIN 2 FEET OF SURFACE..... 95%
- IMPORTED STRUCTURAL FILL SHOULD BE NON-EXPANSIVE, FREE OF ORGANICS AND DEBRIS, AND SELECTED PER THE FOLLOWING GRADATION REQUIREMENTS:

SCREEN OR SIEVE SIZE	PERCENT PASSING BY WEIGHT
3-INCH	100
1 1/2-INCH	80-100
3/4-INCH	60-100
NO. 4	25-60
NO. 200	12 MAXIMUM

SPREAD FOOTING FOUNDATIONS

- BOTH INTERIOR AND EXTERIOR FOOTINGS SHALL BEAR ON THE PROPERLY COMPACTED NATIVE SOILS. AN ALLOWABLE SOIL BEARING PRESSURE OF 1,500 PSF WAS USED FOR ALL FOOTINGS,
- SOILS DISTURBED BELOW THE PLANNED DEPTHS OF FOOTING EXCAVATIONS SHALL EITHER BE RECOMPACTED OR BE REPLACED WITH SUITABLE COMPACTED BACKFILL APPROVED BY THE GEOTECHNICAL ENGINEER.
- THE BOTTOM OF THE FOOTING EXCAVATIONS SHALL BE FREE OF COBBLES AND BOULDERS TO AVOID STRESS CONCENTRATIONS ACTING ON THE BASE OF THE FOOTINGS.
- A REPRESENTATIVE OF THE PROJECT GEOTECHNICAL ENGINEER SHALL OBSERVE ALL FOOTING EXCAVATIONS AND BACKFILL PHASES PRIOR TO THE PLACEMENT OF CONCRETE FORMWORK.

FLOOR SLABS AND EXTERIOR FLATWORK

- FOR NORMALLY LOADED, SLAB-ON-GRADE CONSTRUCTION, A MINIMUM 6-INCH CUSHION COURSE CONSISTING OF FREE-DRAINING, CRUSHED GRAVEL SHOULD BE PLACED BENEATH THE SLABS AND COMPACTED TO A MINIMUM OF 95 PERCENT DENSITY PER ASTM D698 (OR EQUIVALENT PER ASTM D4253-D4254). THIS MATERIAL SHOULD CONFORM TO SECTION 02235 OF MPWSS AND INCORPORATE A MAXIMUM PARTICLES SIZE OF 3/4-INCH. PRIOR TO PLACING THE CUSHION COURSE, THE UPPER SIX INCHES OF SUBGRADE SHOULD BE COMPACTED TO 95 PERCENT OF MAXIMUM DENSITY PER ASTM D698.

MASONRY

- MASONRY WORK SHALL CONFORM TO ALL REQUIREMENTS OF TMS 402/602, "BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTURES".
- HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, GRADE N, TYPE I.
- Fm FOR DESIGN IS 1500 PSI.
- ALL UNITS SHALL BE LAID IN RUNNING BOND UNLESS NOTED OTHERWISE.
- VERTICAL REINFORCING (UNLESS NOTED OTHERWISE):
 - PLACE (2) #5 (8" WALL) BAR IN CENTER OF GROUT AT CENTER OF WALL, CONTINUOUS FULL HEIGHT OF WALL, WITH ONE BAR AT ALL CORNERS, INTERSECTIONS, WALL ENDS, BEAM BEARING, JAMBS AND EACH SIDE OF CONTROL JOINTS AND AT INTERVALS NOT TO EXCEED 48" O.C.. TIE AT 8'-0" VERTICALLY, WITH SINGLE WIRE LOOP TIE BY AMERICAN WIRE TIE PRODUCTS COMPANY (OR EQUAL). DOWEL ALL VERTICAL REINFORCING TO FOUNDATION WITH STANDARD 90 DEGREE HOOKED DOWELS TO MATCH VERTICAL REINFORCING.
 - HORIZONTAL REINFORCING (UNLESS NOTED OTHERWISE):
 - PLACE (2) #5 (8" WALL) BARS IN MINIMUM 8" DEEP GROUDED CONTINUOUS BOND BEAM AT ROOF AND ELEVATED FLOOR LINES AND AT 4'-0" O.C. VERTICALLY. PLACE #5 (8" WALL) BAR IN MINIMUM 8" DEEP GROUDED CONTINUOUS BOND BEAM AT TOP OF PARAPET OR TOP OF FREE-STANDING WALL. PLACE THESE BARS CONTINUOUS THROUGH CONTROL JOINTS. WRAP MASTIC TAPE FOR 1'-6" EACH SIDE OF CONTROL JOINT. PROVIDE BENT BARS TO MATCH HORIZONTAL BOND BEAM REINFORCING, AT CORNERS AND WALL INTERSECTIONS TO MAINTAIN BOND BEAM CONTINUITY. STAGGER ALTERNATE SPLICES A MINIMUM OF 4'-0". PROVIDE STANDARD WEIGHT (NO. 9 GAUGE WIRE) DUR-O-WALL OR DUR-O-WIRE LADDER TYPE JOINT REINFORCING AT 16' O.C. IN MASONRY WALLS. LAP JOINT REINFORCING 6" MINIMUM.
 - MECHANICALLY VIBRATE GROUT IN VERTICAL SPACES IMMEDIATELY AFTER POURING. PROVIDE CLEANOUTS IF GROUT POUR EXCEEDS 5'-0" IN HEIGHT. IF CLEANOUTS ARE PROVIDED, GROUT POUR MAXIMUM HEIGHT = 12'-0", IN LIFTS NOT TO EXCEED 6'-0".
 - UNLESS NOTED OTHERWISE ON THE PLANS, PLACE CONTROL JOINTS IN MASONRY WALLS SUCH THAT NO STRAIGHT RUN OF WALL EXCEEDS 24'-0".
 - (2) #4 HORIZONTAL BARS SHALL BE PLACED AT THE BOTTOM OF ALL MASONRY OPENINGS GREATER THAN 9" WIDE IN FULLY GROUDED CELLS WITH HORIZONTAL LINTEL REINFORCING AT THE TOP OF ALL OPENINGS (AS INDICATED IN THE LINTEL SCHEDULE). REBAR SHALL EXTEND 24" MINIMUM BEYOND OPENINGS.
 - (1) #5 VERTICAL REBAR SHALL BE PLACED IN FULLY GROUDED CELLS ON EACH SIDE OF OPENINGS GREATER THAN 9" WIDE. REBAR AND GROUDED CELLS SHALL EXTEND THE FULL HEIGHT OF THE WALL FROM DIAPHRAGM TO DIAPHRAGM.
 - REBAR SPLICE LENGTHS SHALL BE 64 BAR DIAMETERS.

STRUCTURAL STEEL

- DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (AISC 360).
- IF IT IS NECESSARY TO SPLICE ANY MEMBER, SPLICE LOCATIONS ARE SUBJECT TO REVIEW BY STRUCTURAL ENGINEER. SPLICES SHALL BE FULL PENETRATION WELDED AND TESTED. INDICATE ALL SPLICE LOCATIONS AND WELDING PROCEDURES ON SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION.
- ALL BEAMS SHALL BE ERECTED WITH THE NATURAL CAMBER UPWARDS.
- ALL BOLTS, ANCHOR BOLTS, EXPANSION/ADHESIVE ANCHORS, ETC. SHALL BE INSTALLED WITH STEEL WASHERS.
- ALL WELDING BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES, CERTIFICATES SHALL BE THOSE ISSUED BY AN INDEPENDENT TESTING AGENCY.
- ALL WELDING PER AMERICAN WELDING SOCIETY STANDARDS. SHOP WELDS OR FIELD WELDS SHALL BE SHOWN ON SHOP DRAWINGS.
- SLAG SHALL BE REMOVED FROM ALL COMPLETED WELDS, AND THE WELD AND ADJACENT BASE METAL SHALL BE CLEANED BY BRUSHING OR OTHER SUITABLE MEANS. WELDED JOINTS SHALL NOT BE PAINTED UNTIL AFTER WELDING HAS BEEN COMPLETED AND THE WELD ACCEPTED. ALL COMPLETE PENETRATION WELDS SHALL BE TESTED.
- ALL FIELD WELDING OF STRUCTURAL ATTACHMENTS TO BEAMS OR OTHER MEMBERS SHALL BE PERFORMED WITH THE MEMBER POSITIONED TO ELIMINATE BENDING STRESSES IN THE AREA OF THE WELD.
- BEAMS HAVING FLANGE(S) COPED LONGER THAN LENGTH OF CONNECTION ANGLE(S) OR THE FLANGE AND WEB COPED TO A DEPTH GREATER THAN REQUIRED FOR NORMAL FLANGE THICKNESS PLUS FILLET SHALL HAVE THE WEB REINFORCED TO CARRY LOAD UNLESS CONNECTION DESIGN COMPUTATIONS PROVE REINFORCEMENT UNNECESSARY.
- STRUCTURAL STEEL MEMBERS SHALL BE IDENTIFIED WITH ERECTION MARKS THAT ARE CLEARLY DISCERNABLE THROUGH SHOP GALVANIZING OR PAINTING.
- ALL REFERENCE TO HEADED STUDS SHALL INDICATE AUTOMATIC WELDED HIGH STRENGTH HEADED STUDS (NELSON OR EQUIVALENT). STUDS SHALL CONFORM TO ALL REQUIREMENTS OF THE LATEST EDITION OF THE "RECOMMENDED PRACTICES FOR STUD WELDING" AND THE "STRUCTURAL WELDING CODE" PUBLISHED BY THE AMERICAN WELDING SOCIETY. CONFORMANCE SHALL INCLUDE, BUT NOT BE LIMITED TO, ALL QUALITY CONTROL TESTING PROVISIONS OF THE AFOREMENTIONED PUBLICATIONS.
- ALL WELDS AND WELDING PROCEDURES SHALL BE PERFORMED IN ACCORDANCE WITH APPLICABLE PROVISIONS OF AISC AND AWS WELDING PROCEDURES AND CODES AS OUTLINED IN THE SPECIFICATIONS. SPECIAL ATTENTION SHALL BE GIVEN TO PROPER HEAT TEMPERATURE REQUIREMENTS.
- ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS. USE E80 SERIES FOR ASTM A706 REINFORCING BARS.
- ALL STEEL BASE PLATES SHALL UTILIZE MAXIMUM RECOMMENDED HOLE SIZE AND STEEL PLATE WASHERS PER AISC CONSTRUCTION MANUAL TABLE 14-2 UNO. CONTRACTOR TO FIELD WELD PLATE WASHERS TO BASE PLATE WITH MINIMUM SIZE FILLET FULL WASHER PERIMETER AFTER INSTALLATION OF COLUMN.

CONNECTIONS

- ALL BOLTED FIELD CONNECTIONS SHALL BE MADE WITH HIGH STRENGTH BOLTS UNLESS NOTED OTHERWISE. ALL BOLTED CONNECTIONS SHALL BE MADE WITH A MINIMUM OF TWO BOLTS, U.N.O.
- ALL HIGH STRENGTH BOLTS SHALL BE 7/8" DIAMETER UNLESS NOTED OTHERWISE.
- BOLTS SHALL BE BEARING TYPE UNLESS NOTED OTHERWISE.
- HIGH STRENGTH BOLTS SHALL BE TIGHTENED ACCORDING TO AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".
- ALL SHOP CONNECTIONS SHALL BE WELDED.
- ALL WELDED CONNECTIONS SHALL BE SEAL WELDED WITH 1/8" FILLET WELDS FOR GALVANIZING.
- IF THE SIZE OF A FILLET WELD IS NOT SHOWN ON THE DRAWINGS, PROVIDE THE MINIMUM FILLET WELD AS SHOWN IN TABLE J2.4 OF THE AISC SPECIFICATION.
- BRACING CONNECTIONS SHALL HAVE TWO BOLT CONNECTIONS AT EACH END UNLESS NOTED OTHERWISE ON THE DRAWINGS. BRACING CONNECTIONS SHALL BE DESIGNED FOR THE SHEAR CAPACITY OF THE BOLTS WITH THE BRACING MEMBER IN TENSION. THE WORK POINT SHALL BE AT THE CENTERLINE OF THE BEAM AND COLUMN UNLESS NOTED OTHERWISE.
- ALL GUSSET PLATES SHALL BE 3/8" MINIMUM THICKNESS UNLESS NOTED OR DETAILED OTHERWISE.
- GUSSET PLATES FOR THE VERTICAL DIAGONAL BRACING SHALL CONNECT TO BOTH THE BEAM AND COLUMN UNLESS NOTED OR DETAILED OTHERWISE.

DRYPACK/FLOWABLE GROUT

- THE SPACE BENEATH ALL BASEPLATES AND BEARING PLATES SHALL BE THOROUGHLY CLEANED BEFORE DRYPACKING OR GROUTING. DRYPACK/GROUT SOLID BENEATH ALL BASEPLATES AND BEARING PLATES. NO VOIDS ARE PERMISSIBLE. USE OF DRYPACK OR FLOWABLE GROUT IS AT THE PLAN OR DETAILS, DRYPACK/GROUT PER THE FOLLOWING:
 - DRYPACK - PORTLAND CEMENT, ASTM C150, TYPE 1; AND CLEAN, NATURAL SAND, ASTM C404, SIZE NO. 2, MINIMUM COMPRESSIVE STRENGTH SHALL BE 5000 PSI AT 28 DAYS WHEN TESTED IN ACCORDANCE WITH ASTM C109.
 - FLOWABLE GROUT - PREMIXED, NONMETALLIC, NONCORROSIVE, NONSTAINING GROUT CONTAINING SELECTED SILICA SANDS, PORTLAND CEMENT, SHRINKAGE COMPENSATING AGENTS, PLASTICIZING AND WATER-REDUCING AGENTS, COMPLYING WITH ASTM C1107, OF CONSISTENCY SUITABLE FOR APPLICATION, AND A 30-MINUTE WORKING TIME. MINIMUM COMPRESSIVE STRENGTH SHALL BE 5000 PSI AT 28 DAYS WHEN TESTED IN ACCORDANCE WITH ASTM C1107.
- GROUTING IS THE RESPONSIBILITY OF THE CONCRETE CONTRACTOR. GROUTING SHOULD BE PERFORMED PROMPTLY ONCE STEEL COLUMNS ARE IN THEIR FINAL LOCATIONS. STEEL ERECTOR SHALL NOTIFY CONCRETE CONTRACTOR IMMEDIATELY UPON FINAL PLACEMENT OF COLUMNS.

Structural Sheet Index with Issues

Sheet Number	Sheet Name	ISSUE FOR BID 2/4/20
S0.00	GENERAL STRUCTURAL NOTES	X
S0.01	GENERAL STRUCTURAL NOTES	X
S0.02	GENERAL SCHEDULES AND NOTES	X
S0.03	SPECIAL INSPECTIONS	X
S1.00	FOUNDATION PLAN	X
S1.10	ROOF FRAMING PLAN	X
S2.00	BUILDING SECTIONS	X
S5.00	FOUNDATION DETAILS	X
S6.00	FRAMING DETAILS	X
S7.00	TYPICAL FOUNDATION DETAILS	X
S8.00	TYPICAL FRAMING DETAILS	X

PERMIT ISSUE



REV	DATE	DESCRIPTION
0	06/25/20	ISSUE FOR PERMIT
A	2/6/20	ISSUED FOR BID



DRAWN BY: BMS
 DESIGNED BY: MAD
 QUALITY CHECK: MAD
 DATE: 2/6/2020
 JOB NO: M19-064
 FIELDBOOK:

NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
 ASHLAND AVE.
 FOLCROFT, PA 19032
 GENERAL STRUCTURAL NOTES

PRE-ENGINEERED METAL BUILDINGS

- SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION.
- BUILDING SHALL BE DESIGNED FOR ALL APPLICABLE LOADING CONDITIONS REQUIRED BY THE 2015 INTERNATIONAL BUILDING CODE.
- CONTRACTOR SHALL VERIFY THAT THE GEOMETRIC SIZE OF ALL STEEL FRAME CONCRETE SUPPORTS ARE ADEQUATE TO ACCOMMODATE THE METAL BUILDING COLUMN BASE PLATES, WITH CONSIDERATION FOR MANUFACTURER REQUIRED CLEARANCES. ANY REQUIRED INCREASES IN SIZES OF FOUNDATION SHALL BE AT THE EXPENSE AND RESPONSIBILITIES OF THE CONTRACTOR. ANY CHANGES TO THE FOUNDATION SHALL BE APPROVED BY THE ENGINEER.
- CONTRACTOR TO COORDINATE ALL PENETRATIONS THROUGH THE WALLS OF THE METAL BUILDING WITH ARCHITECTURAL AND MECHANICAL.

DEFERRED SUBMITTALS

- DEFERRED SUBMITTALS ARE THOSE PORTIONS FOR THE DESIGN WHICH ARE NOT SUBMITTED AT THE TIME OF PERMIT APPLICATION AND WHICH ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD.
- SUBMITTAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD THROUGH THE ARCHITECT AND GENERAL CONTRACTOR WITHIN 6 WEEKS OF AWARD OF CONTRACT TO THE GENERAL CONTRACTOR. ONCE THE SUBMITTAL DOCUMENTS HAVE BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS, THE ENGINEER OF RECORD WILL FORWARD THEM TO THE ARCHITECT WITH A NOTATION INDICATING THAT THEY ARE IN GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING. THE ARCHITECT WILL FORWARD THE DEFERRED SUBMITTAL DOCUMENTS TO THE GENERAL CONTRACTOR WHO WILL MAINTAIN ONE SET ON SITE FOR REFERENCE BY THE CITY INSPECTOR. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.
- ITEMS THAT ARE SUBMITTED FOR CONSIDERATION AS DEFERRED SUBMITTALS ARE AS FOLLOWS:
 - PRE-ENGINEERED METAL BUILDING
 - STRUCTURAL FILL MATERIAL
 - CONCRETE MIX DESIGN
 - OPEN WEB STEEL JOISTS
 - REBAR LAYOUT FOR CONCRETE AND CMU
 - FOUNDATION & ANCHOR BOLT DRAWING
 - SLAB-ON-GRADE LAYOUT

STEEL ROOF DECKING

- STEEL ROOF DECK SHALL COMPLY WITH I.C.B.O. REPORT 2078 (VERCO MANUFACTURING CO.), I.C.B.O. REPORT 3415 (VULCRAFT), I.C.B.O. REPORT 2757 (ASC STEEL DECK) OR HAVE CURRENT I.C.B.O. APPROVAL.
- DECK PROPERTIES:

DEPTH _____	1 1/2"
WIDTH _____	36
GAUGE _____	20
MINIMUM YIELD STRESS _____	33,000 PSI (ASTM A653)
MINIMUM +S _____	.235 IN ³ PER FOOT OF WIDTH
MINIMUM T _____	.216 IN ⁴ PER FOOT OF WIDTH
SPAN MINIMUM _____	3 SPAN
MINIMUM DIAPHRAGM SHEAR CAPACITY _____	360 PLF
FINISH _____	GALVANIZED

- MINIMUM ECK ATTACHMENT:

SPACING	ATTACHMENT TO SUPPORTING MEMBERS
5 PER SHEET	3/4" DIAMETER (1/2" EFFECTIVE DIAMETER) OR 5/8"x1 1/4" (3/8"x1" EFFECTIVE SIZE) PUDDLE WELDS PER SHEET AT ENDS, END LAPS AND INTERMEDIATE SUPPORTS.

12" O.C. 3/4" DIAMETER (1/2" EFFECTIVE DIAMETER) OR 5/8"x1 1/4" (3/8"x1" EFFECTIVE SIZE) PUDDLE WELDS AT BEAMS AND OPENING EDGES RUNNING PARALLEL TO THE DECK.

24" O.C. #10 TEK SCREWS AT SIDE SEAM ATTACHMENTS.

- SHEETS SHALL HAVE LAPPED ENDS. MINIMUM LAP = 2".
- 2" MINIMUM BEARING AT ALL SUPPORTS.
- NO PRESENT SUSPENDED LOADS ARE TO BE SUPPORTED BY THE STEEL DECK.
- THE FIRST SHEET OF STEEL DECK ADJACENT AND PARALLEL TO WALLS, PERIMETER MEMBERS OR MEMBERS IDENTIFIED AS CHORD, COLLECTOR OR DRAG MEMBERS (ON ONE OR BOTH SIDES AS APPLICABLE) SHALL BE FULL PANEL WIDTH SHEETS.
- ALL WELDING SHALL BE PERFORMED BY WELDERS EXPERIENCED IN LIGHT GAUGE STEEL DECK WORK. ALL WELDING DONE BY E60XX (MINIMUM) SERIES RODS.

STEEL JOISTS

- ALL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STANDARD SPECIFICATIONS OF THE STEEL JOIST INSTITUTE AND OSHA REGULATIONS.
- JOIST SIZES ARE AS INDICATED ON PLAN. TYPICAL CALLOUTS ARE AS FOLLOWS:

TYPICAL STEEL JOIST - 24K# (SIZED BASED ON APPLICABLE LOADS)

DESIGN HOIST AND BRIDGING FOR NET WIND UPLIFT AS SHOWN ON THE FRAMING PLAN NOTES.

- ALL JOISTS AND JOIST GIRDERS SHALL BE CAMBERED PER SJI SPECIFICATIONS.
- CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND SEALED CALCULATIONS FOR REVIEW PRIOR TO MANUFACTURE. SUBMITTAL SHALL INCLUDE CALCULATIONS FOR ALL JOIST SHOES WHERE BEARING LENGTH IS LESS THAN 4" AT LH SERIES JOISTS OR 2 1/2" FOR K SERIES JOISTS. CALCULATIONS SHALL INCLUDE DEAD AND LIVE LOAD DEFLECTIONS, STRESS RATIOS FOR INDIVIDUAL MEMBERS IN THE CALCULATIONS SHALL NOT EXCEED 1.00 FOR COMBINED DEAD AND LIVE LOADS AND 1.33 FOR ANY LOAD COMBINATION INCLUDING WIND OR SEISMIC. SHOP DRAWINGS AND CALCULATIONS SHALL INCLUDE DETAILS OF ANY OPTIONAL FIELD SPLICES AND IF HIGH STRENGTH BOLTS OR COMPLETE PENETRATION WELDS ARE UTILIZED, CONTRACTOR SHALL RETAIN AN INDEPENDENT TESTING LABORATORY FOR TESTING AND CERTIFICATION.
- ALL JOISTS SHALL BE DESIGNED FOR AN ADDITIONAL 500 LB. POINT LOAD ANYWHERE ALONG THE SPAN APPLIED AT THE BOTTOM CHORD PANEL POINTS.
- LIVE LOAD DEFLECTIONS SHALL BE LIMITED TO SPAN/360 AT SIMPLE SPAN FLOOR MEMBERS AND SPAN/240 AT SIMPLE SPAN ROOF MEMBERS.
- ALL CONCENTRATED LOADS TO STEEL JOISTS SHALL OCCUR WITHIN 6" OF A PANEL POINT. TO ACCOUNT FOR ADDITIONAL CONCENTRATED LOADS, MANUFACTURER SHALL ADD ADDITIONAL WEB MEMBERS AS REQUIRED AND ADJUST CHORD AND WEB SIZES ACCORDINGLY, BUT SHALL NOT ALTER DEPTH OF JOISTS.
- WHERE CROSS BRIDGING INTERFERES WITH MECHANICAL INSTALLATIONS, REMOVE THIS CROSS BRIDGING AFTER ROOF/FLOOR DECK IS IN PLACE AND REPLACE WITH HORIZONTAL ANGLES 2"x2"x3/16" AT TOP AND BOTTOM CHORDS. COORDINATE WITH JOIST MANUFACTURER.

LEGEND AND ABBREVIATIONS

A.B.	ANCHOR BOLT	IBC	INTERNATIONAL BUILDING CODE
ACI	AMERICAN CONCRETE INSTITUTE	INT	INTERIOR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LG	STEEL ANGLE
ALT	ALTERNATE	LLV	LIGHT GAUGE
APPROX	APPROXIMATE	LONG	LONG LEG VERTICAL
ARCH	ARCHITECTURAL	MAX	LONGITUDINAL
ASTM	AMERICAN SOCIETY FOR TESTING MATERIAL	MCJ	MAXIMUM
@	AT	MECH	MASONRY CONTROL JOINT
BLDG	BUILDING	MANUF OR MFR	MECHANICAL
BLK'G	BLOCKING	MIN	MANUFACTURER
BM	BEAM	MISC	MINIMUM
B.O.C.	BOTTOM OF CONCRETE	NO. OR #	MISCELLANEOUS
B.O.F.	BOTTOM OF FOOTING	(N)	NUMBER
B.O.S.	BOTTOM OF STEEL/SLAB	NTS	NEW
BOT	BOTTOM OF	O.C.	NOT TO SCALE
BRG	BEARING	O.C.E.F.	ON CENTER
B.T.B.	BACK TO BACK	O.C.E.W.	ON CENTER EACH FACE
BTWN	BETWEEN	OPP	ON CENTER EACH WAY
C	STEEL CHANNEL	OWJ	OPPOSITE
CFS	COLD FORMED STEEL	PEMB	OPEN WEB JOIST
CIP	CAST IN PLACE	PLCS	PRE-ENGINEERED METAL BUILDING
CJ	CONTROL JOINT	PL	PLACES
CL OR CL	CENTER LINE	PLATE	PLATE
CMU	CONCRETE MASONRY UNIT	PREFAB	PREFABRICATED
COL	COLUMN	P.T.	PRESSURE TREATED
CONC	CONCRETE	REF	REFERENCE
CONN	CONNECTION	REINF	REINFORCEMENT
CONT	CONTINUOUS	REQ'D	REQUIRED
DEMO	DEMOLISH	REV	REVISION/REVISED
DET	DETAIL	SCH OR SCHED	SCHEDULE
D.F.	DIAMETER	SFE	SUBFLOOR ELEVATION
Ø OR DIA	DIAMETER	SHT	SHEET
DIM	DIMENSION	SIM	SIMILAR
D.J.	DOUBLE JOIST	SOG	SLAB-ON-GRADE
DWG	DRAWING	SPCS OR SPA	SPACE(S)
EA	EACH	SPEC	SPECIFICATION(S)
EA WAY OR E.W.	EACH WAY	SQ	SQUARE
E.F.	EACH FACE	STD	STANDARD
E.J.	EXPANSION JOINT	STRUCT	STRUCTURAL
EL OR ELEV	ELEVATION	SYM	SYMMETRICAL
EMBED	EMBEDMENT	T&B	TOP & BOTTOM
ENG	ENGINEER	THRU	THROUGH
E.O.R.	ENGINEER OF RECORD	T.O.B.	TOP OF BEAM
EQ	EQUAL	T.O.C.	TOP OF CONCRETE
EXIST OR (E)	EXISTING	T.O.F.	TOP OF FOOTING
EXP	EXPANSION	T.O.S.	TOP OF STEEL/SLAB
EXT	EXTERIOR	T.O.W.	TOP OF WALL
FDT OR FND	FOUNDATION	TRANS	TRANSVERSE
F.F.	FINISH FLOOR	TYP	TYPICAL
FLR	FLOOR	V.I.F.	VERIFY IN FIELD
FTG	FOOTING	VERT	VERTICAL
GA	GAUGE	U.N.O.	UNLESS NOTED OTHERWISE
GALV	GALVANIZED	W/	WITH
GEN	GENERAL	WF	WIDE FLANGE
GR	GRADE	W.P.	WORKING POINT
HORIZ	HORIZONTAL	WT	WEIGHT
H.S.A.	HEADED STUD ANCHOR		
H.S.S.	HOLLOW STRUCT STEEL		



CONCRETE



CONCRETE MASONRY UNIT



STEEL IN SECTION



SECTION OR DETAIL DESIGNATION SYMBOL



SHEET NUMBER WHERE SECTION OR DETAIL IS SHOWN



ELEVATION NOTED

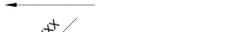


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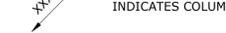


REVISION SPECIFIED

UP OR DOWN STAIR IDENTIFICATION



INDICATES COLUMN



INDICATES BASE PLATE, SEE SCHEDULE Sx.x



INDICATES COLUMN



INDICATES BASE PLATE, SEE SCHEDULE Sx.x



INDICATES COLUMN



INDICATES BASE PLATE, SEE SCHEDULE Sx.x



INDICATES COLUMN



INDICATES BASE PLATE, SEE SCHEDULE Sx.x



INDICATES COLUMN



INDICATES BASE PLATE, SEE SCHEDULE Sx.x



INDICATES COLUMN



INDICATES BASE PLATE, SEE SCHEDULE Sx.x



INDICATES COLUMN



INDICATES BASE PLATE, SEE SCHEDULE Sx.x



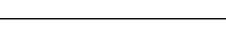
INDICATES COLUMN



INDICATES BASE PLATE, SEE SCHEDULE Sx.x



INDICATES COLUMN



INDICATES BASE PLATE, SEE SCHEDULE Sx.x

PERMIT ISSUE



REV	DATE	Description
0	06/25/20	ISSUE FOR PERMIT
A	2/6/20	ISSUED FOR BID



DRAWN BY:	BMS
DESIGNED BY:	MAD
QUALITY CHECK:	MAD
DATE:	2/6/2020
JOB NO:	M19-064
FIELDBOOK:	

NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
 ASHLAND AVE.
 FOLCROFT, PA 19032

GENERAL STRUCTURAL NOTES

CONCRETE	FINISH
FORMED SURFACES	
BEAM AND SLABS	POINT AND PATCH
EXTERIOR WALLS ABOVE GRADE	SACKED FINISH
EXTERIOR WALLS BELOW GRADE	ROUGH FORM FINISH
INTERIOR WALLS - EXPOSED	SACKED FINISH
INTERIOR WALLS - COVERED	POINT AND PATCH
UNFORMED SURFACES	
INTERIOR SLABS	TROWEL FINISH, HARDENED AND POLISHED
EXTERIOR SLABS ON GRADE	BROOM FINISH
EXTERIOR SLABS BELOW GRADE	FLOAT FINISH

CAST-IN-PLACE (NONPRESTRESSED) CONCRETE	CONCRETE COVER
CAST AGAINST & EXPOSED TO EARTH	3"
EXPOSED TO EARTH OR WEATHER No. 6 THROUGH No. 18 BARS No. 5 BAR, W31 OR D31 WIRE, AND SMALLER	2" 1-1/2"
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND SLABS, WALLS, JOISTS No. 14 AND No. 18 BARS No. 11 BAR AND SMALLER BEAMS, COLUMNS PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS SHELLS, FOLDED PLATE MEMBERS No. 6 BAR AND LARGER No. 5 BAR, W31 OR D31 WIRE, AND SMALLER	1-1/2" 3/4" 1-1/2" 3/4" 1/2"

BAR SIZE			CONCRETE	
IN-LB	SOFT METRIC	AREA (IN ²)	HORIZ & VERT	TOP
#3	#10	0.11	1'-7"	2'-1"
#4	#13	0.20	2'-1"	2'-9"
#5	#16	0.31	2'-7"	3'-5"
#6	#19	0.44	3'-1"	4'-1"
#7	#22	0.60	4'-6"	5'-11"
#8	#25	0.79	5'-2"	6'-9"
#9	#29	1.00	5'-10"	7'-7"
#10	#32	1.27	6'-7"	8'-6"

BAR SIZE		MASONRY		
IN-LB	AREA (IN ²)	MINIMUM MASONRY COVER		
		2"	3"	6"
#3	0.11	1'-3"	1'-0"	1'-0"
#4	0.20	2'-2"	1'-5"	1'-0"
#5	0.31	3'-4"	2'-3"	1'-2"
#6	0.44	6'-2"	4'-2"	2'-1"
#7	0.60	8'-5"	5'-7"	2'-10"
#8	0.79	12'-8"	8'-5"	4'-3"

- NOTES:
- BASED ON COMPRESSIVE STRENGTH OF MASONRY, f'm = 1500 PSI AND STEEL YIELD STRENGTH, Fy = 60 SKI.
 - TABLE APPLICABLE FOR BARS IN TENSION OR COMPRESSION.
 - CLEAR SPACING BETWEEN ADJACENT REINFORCEMENT SPLICES SHALL NOT BE LESS THAN MIN. MASONRY COVER OR 9d_b.

- NOTES:
- FOR REINFORCING WITH EPOXY COATING, MULTIPLY LAP LENGTH SHOWN BY 1.5.
 - CONCRETE LAP LENGTHS ARE CLASS "B" BASED ON F'c=4,000 PSI WITH COVER REQUIREMENTS INDICATED AND BAR SPACING AT LEAST TWO BAR DIAMETERS.
 - TOP BAR LAPS ARE HORIZONTAL LAPS WHERE MORE THAN 12" OF FRESH CONCRETE IS PLACED BELOW THE BARS.
 - TOP BAR LAP LENGTHS MAY BE USED AT ALL LOCATIONS IN CONCRETE AT THE CONTRACTOR'S DISCRETION.

1 CONCRETE FINISH SCHEDULE

DO NOT SCALE

2 REINFORCING CONCRETE COVER

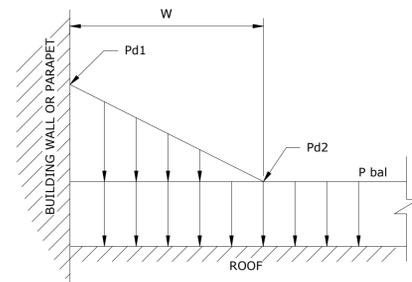
DO NOT SCALE

3 REINFORCING SPLICE LENGTHS

DO NOT SCALE

4 MASONRY REINFORCING BAR SPLICE LENGTHS

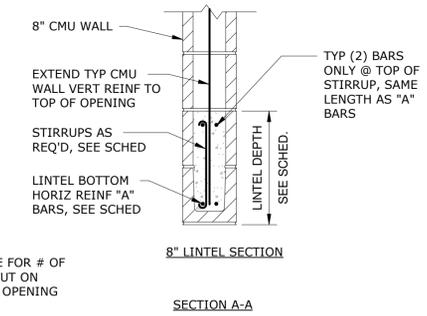
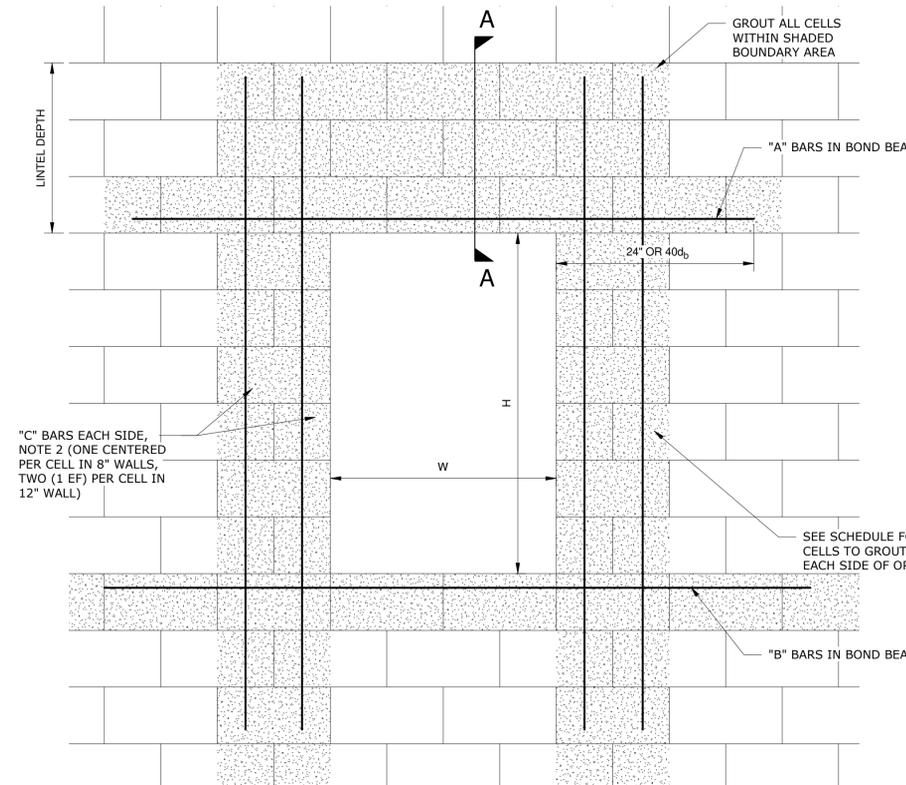
DO NOT SCALE



DRIFT	Pbal (PSF)	Pd1 (PSF)	Pd2 (PSF)	W
1	20	48	0	11'-4"
2	20	20	0	7'-3"
3	20	63	0	9'-3"

ZONE	EFFECTIVE WIND AREA (A _e)		
	10 ft ²	100 ft ²	500 ft ²
ROOF 1	+13.7/-21.8	+10.0/-19.8	+10.0/-19.8
ROOF 2	+13.7/-37.9	+10.0/-27.8	+10.0/-27.8
ROOF 3	+13.7/-56.0	+10.0/-44.0	+10.0/-44.0
ROOF OVERHANG 2	-44.4	-44.4	-44.4
ROOF OVERHANG 3	-74.6	-50.4	50.4
WALL 4	+23.8/-25.8	+20.2/-22.2	+17.7/-19.8
WALL 5	+23.8/-31.9	+20.2/-24.7	+17.7/-19.8

- NOTES:
- (+) AND (-) DENOTES PRESSURES TOWARDS AND AWAY FROM SURFACES, RESPECTIVELY.
 - LINEAR INTERPOLATION FOR OTHER EFFECTIVE AREAS ARE PERMITTED.
 - VALUES PER ASCE 7-10 CHAPTER 28, PART 2.
 - DIMENSION "a" = 8.95 FT.
 - TABLE INTENDED TO BE USED FOR PEMB COMPONENTS & CLADDING DESIGN.



8" WALLS							
MARK	W	LINTEL DEPTH	STIRRUPS	"A" BARS	"B" BARS	"C" BARS	# CELLS TO GROUT (NOTE 4)
L1	≤3'-4"	8"	-	(1) #5	(1) #5	(1) #6	ONE
L2	>3'-4" ≤7'-0"	16"	#3 @ 16"	(2) #5	(1) #5	(2) #6	TWO
L3	>7'-0"	24"	#3 @ 16"	(2) #5	(1) #5	(2) #6	TWO

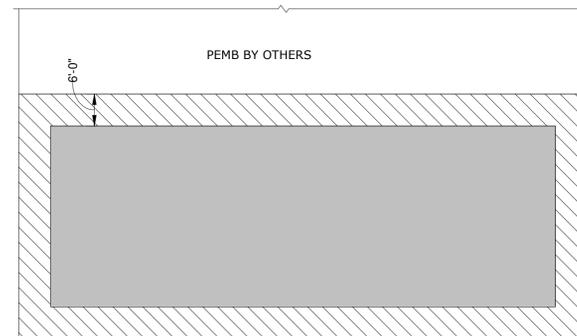
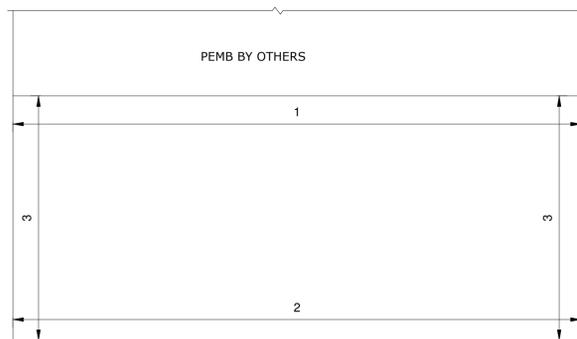
- NOTES:
- USE BAR QUANTITIES AND SIZES GIVEN IN LINTEL SCHEDULE UNLESS OTHERWISE NOTED ON THE DRAWINGS.
 - EXTEND "C" BARS 2'-8" MIN BEYOND TOP AND BOTTOM OF OPENING, EXCEPT THAT WHEN "H" OR "W" EXCEEDS 2'-0", "C" BARS SHALL EXTEND FULL HEIGHT.
 - "A" AND "B" BARS SHALL EXTEND 2'-8" EACH SIDE OF THE OPENINGS.
 - SEE SCHEDULE FOR NUMBER OF CELLS TO GROUT ON EACH SIDE OF THE OPENING.
 - GROUT ALL CELLS OVER OPENING TO LINTEL DEPTH INDICATED IN SCHEDULE.
 - CELLS SHOWN TO BE GROUTED ARE MINIMUM. GROUT ALL CELLS AS SHOWN IN TYPICAL CMU WALL DETAIL.
 - DO NOT SOLID GROUT WALLS UNLESS SPECIFICALLY INDICATED.

5 SNOW LOAD WITH DRIFT DETAIL

DO NOT SCALE

6 COMPONENTS & CLADDING WIND PRESSURES

12" = 1'-0"



- LEGEND:
- = -17.5 PSF
 - ▨ = -25.2 PSF

7 SNOW DRIFT PLAN

DO NOT SCALE

8 ROOF UPLIFT PLAN

DO NOT SCALE

9 CMU LINTEL SCHEDULE

DO NOT SCALE

PERMIT ISSUE



REV	DATE	DESCRIPTION
0	06/25/20	ISSUE FOR PERMIT
1	2/6/20	ISSUED FOR BID



DRAWN BY: BMS
DESIGNED BY: MAD
QUALITY CHECK: MAD
DATE: 2/6/2020
JOB NO: M19-064
FIELDBOOK:

NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
ASHLAND AVE.
FOLCROFT, PA 19032

GENERAL SCHEDULES AND NOTES

STATEMENT OF SPECIAL INSPECTIONS

- SPECIAL INSPECTION AND TESTING SHALL BE PROVIDED BY THE OWNER IN ACCORDANCE WITH CHAPTER 17 OF THE 2015 IBC.
- ALL SPECIAL INSPECTORS SHALL BE UNDER THE SUPERVISION OF A REGISTERED CIVIL OR STRUCTURAL ENGINEER LICENSED IN THE STATE IN WHICH THE WORK IS TO BE PERFORMED. ALL INSPECTIONS SHALL BE PERFORMED BY EXPERIENCED PERSONNEL MEETING THE REQUIREMENTS OF THE IBC AND AC291 "ACCREDITATION CRITERIA FOR SPECIAL INSPECTION AGENCIES" AND SHALL BE APPROVED BY THE LICENSED ENGINEER OF RECORD.
- SPECIAL INSPECTIONS ARE NOT REQUIRED FOR WORK OF A MINOR NATURE AS APPROVED BY THE BUILDING OFFICIAL, NOR ARE THEY REQUIRED FOR GROUP U OCCUPANCIES.
- EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN WIND OR SEISMIC FORCE RESISTING SYSTEM SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK AS OUTLINED IN 1704.4 OF THE IBC.
- IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE SPECIAL INSPECTOR OR SPECIAL INSPECTION AGENCY AT LEAST TWO WORKING DAYS PRIOR TO PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION. ALL WORK PERFORMED WITHOUT THE REQUIRED SPECIAL INSPECTION IS SUBJECT TO REMOVAL.
- SPECIAL INSPECTIONS SHALL BE REQUIRED FOR PROPOSED WORK THAT IS, IN THE OPINION OF THE BUILDING OFFICIAL, UNUSUAL IN ITS NATURE, SUCH AS, BUT NOT LIMITED TO THE FOLLOWING EXAMPLES: CONSTRUCTION MATERIALS AND SYSTEMS THAT ARE ALTERNATIVES TO MATERIALS AND SYSTEMS PRESCRIBED BY THE IBC, UNUSUAL DESIGN APPLICATIONS OF MATERIALS PRESCRIBED IN THE IBC, AND MATERIALS AND SYSTEMS REQUIRED TO BE INSTALLED IN ACCORDANCE WITH ADDITIONAL MANUFACTURER'S INSTRUCTIONS THAT PRESCRIBE REQUIREMENTS NOT CONTAINED IN THE IBC OR IN STANDARDS REFERENCED BY THE IBC.

SPECIAL INSPECTIONS FOR MASONRY

- MASONRY CONSTRUCTION SHALL BE INSPECTED AND VERIFIED IN ACCORDANCE WITH TMS 402/ACI 530/ASCE 5 AND TMS 602/ACI 530.1/ASCE 6 QUALITY ASSURANCE PROGRAM REQUIREMENTS.

SPECIAL INSPECTIONS FOR STEEL CONSTRUCTION

- SPECIAL INSPECTIONS OF THE STEEL FABRICATION PROCESS SHALL NOT BE REQUIRED WHERE THE FABRICATOR DOES NOT PERFORM ANY WELDING, THERMAL CUTTING OR HEATING OPERATION OF ANY KIND AS PART OF THE FABRICATION PROCESS. IN SUCH CASES, THE FABRICATOR SHALL BE REQUIRED TO SUBMIT A DETAILED PROCEDURE FOR MATERIAL CONTROL THAT DEMONSTRATES THE FABRICATOR'S ABILITY TO MAINTAIN SUITABLE RECORDS AND PROCEDURES SUCH THAT, AT ANY TIME DURING THE FABRICATION PROCESS, THE MATERIAL SPECIFICATION, AND GRADE FOR THE MAIN STRESS-CARRYING ELEMENTS ARE CAPABLE OF BEING DETERMINED. MILL TEST REPORTS SHALL BE IDENTIFIABLE TO THE MAIN STRESS-CARRYING ELEMENTS WHEN REQUIRED BY THE APPROVED CONSTRUCTION DOCUMENTS.
- STRUCTURAL STEEL - SPECIAL INSPECTION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360.
- STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL - SPECIAL INSPECTION FOR STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE TABLE SHOWN.
- COLD-FORMED STEEL - WELDING INSPECTION AND WELDING INSPECTOR QUALIFICATIONS FOR COLD-FORMED STEEL FLOOR AND ROOF DECKS SHALL BE IN ACCORDANCE WITH AWS D1.3.
- REINFORCING STEEL - WELDING INSPECTION AND WELDING INSPECTOR QUALIFICATIONS FOR REINFORCING STEEL SHALL BE IN ACCORDANCE WITH AWS D1.4 AND ACI 318.
- COLD-FORMED STEEL TRUSSES SPANNING 60 FEET OR GREATER - WHERE A COLD-FORMED STEEL TRUSS CLEAR SPAN IS 60 FEET OR GREATER, THE SPECIAL INSPECTOR SHALL VERIFY THAT THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING ARE INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE.

SPECIAL INSPECTIONS FOR STEEL			
APPLIES	INSPECTION TASK	FREQUENCY	
		CONT	PERIODIC
	1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:		
X	a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARD SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS		X
X	b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED INSPECTION OF ADHESIVE ANCHORS		
X	2. INSPECTION OF ADHESIVE ANCHORS		X
	3. INSPECTION OF HIGH-STRENGTH BOLTING (REF AISC 360, SEC. M2.5):		
X	a. SNUG-TIGHT JOINTS		X
X	b. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST-OFF BOLT, OR DIRECT TENSION INDICATOR.		X
X	c. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCHMARKING OR CALIBRATED WRENCH	X	
	4. MATERIAL VERIFICATION OF STRUCTURAL STEEL:		
X	a. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360.		X
X	b. MANUFACTURER'S CERTIFIED MILL TEST REPORTS		X
	5. MATERIAL VERIFICATION OF WELD FILLER MATERIAL:		
X	a. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS		X
X	b. MANUFACTURER'S CERTIFICATION OF COMPLIANCE REQUIRED		X
	6. INSPECTION OF WELDING:		
X	a. STRUCTURAL STEEL		
X	1) COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS	X	
X	2) MULTI-PASS FILLET WELDS	X	
X	3) SINGLE PASS FILLET WELDS > 5/16"	X	
X	4) PLUG AND SLOT WELDS	X	
X	5) SINGLE PASS FILLET WELDS < 5/16"		X
	7. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS:		
X	a. DETAILS SUCH AS BRACING AND STIFFENING		
X	b. MEMBER LOCATIONS		
X	c. APPLICATION OF JOINT DETAILS AT EACH CONNECTION		

SPECIAL INSPECTIONS FOR MASONRY					
VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.5 B.1.b.3 FOR SELF-CONSOLIDATING GROUT					
VERIFICATION OF f'm AND FAAC IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.4 B PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY THIS CODE					
APPLIES	INSPECTION TASK	FREQUENCY		FREQUENCY	
		CONT.	PERIODIC	TMS 402/ACI 530/ASCE 5	TMS 602/ACI 530.1/ASCE 6
X	1. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS		X		
	2. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
X	a. PROPORTIONS OF SITE-PREPARED MORTAR		X		Art 2.1, 2.6 A
X	b. CONSTRUCTION OF MORTAR JOINTS		X		Art 3.3 B
	c. GRADE AND SIZE OF PRE-STRESSING TENDONS AND ANCHORAGES		X		Art 2.4 B, 2.4 H
X	d. LOCATION OF REINFORCEMENT, CONNECTORS AND PRE-STRESSING TENDONS AND ANCHORAGES		X		Art 3.4, 3.6 A
	e. PRE-STRESSING TECHNIQUE		X		Art 3.6 B
	f. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	X (b)	X (c)		Art 2.1 C
	3. PRIOR TO GROUTING, VERIFY THE FOLLOWING ARE IN COMPLIANCE:				
X	a. GROUT SPACE		X		Art 3.2 D, 3.2 F
X	b. GRADE, TYPE AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRE-STRESSING TENDONS AND ANCHORAGES		X	SEC. 1.16	Art 2.4, 3.4
X	c. PLACEMENT OF REINFORCEMENT, CONNECTORS AND PRE-STRESSING TENDONS AND ANCHORAGES		X	SEC. 1.16	Art 3.2 E, 3.4, 3.6 A
X	d. PROPORTIONS OF SITE-PREPARED GROUT AND PRE-STRESSING GROUT FOR BONDED TENDONS		X		Art 2.6 B, 2.4, G.1.b
X	e. CONSTRUCTION OF MORTAR JOINTS		X		Art 3.3 B
	4. VERIFY DURING CONSTRUCTION:				
	a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS		X		Art 3.3 F
X	b. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION		X	SEC. 1.16.4.3 1.17.1	
	c. WELDING OF REINFORCEMENT		X	SEC. 2.1.7.7.2 3.3.3.4 8.3.3.4(b)	
X	d. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F (4.4°C)) OR HOT WEATHER (TEMPERATURE ABOVE 90°F (32.2°C))		X		Art 1.8 C, 1.8 D
	e. APPLICATION AND MEASUREMENT OF PRE-STRESSING FORCE		X		Art 3.6 B
	f. PLACEMENT OF GROUT AND PRE-STRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE		X		Art 3.5, 3.6 C
	g. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	X (b)	X (c)		Art 3.3 B.8
X	5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS		X		Art 1.4 B.2.a.3, 1.4 B.2.b.3, 1.4 B.2.b.3, 1.4 B.3.1.4 B.4

- FREQUENCY REFERS TO THE FREQUENCY OF INSPECTION, WHICH MAY BE CONTINUOUS DURING THE TASK LISTED OR PERIODICALLY DURING THE LISTED TASK, AS DEFINED IN THE TABLE
- REQUIRED FOR THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) IF AAC MASONRY
- REQUIRED FOR THE FIRST 5000 SQUARE FEET (465 SQUARE METERS) IF AAC MASONRY

TABLE 1705.3 REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION					
APPLIES	VERIFICATION AND INSPECTION	CONT	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
X	1. INSPECTION OF REINFORCING STEEL. INCLUDING PRE-STRESSING TENDONS, AND PLACEMENT		X	ACI 318: 3.5, 7.1-7.7	1910.4
	2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1705.2.2, ITEM 2b			AWS D1.4 ACI 318: 3.5.2	
X	3. INSPECTION OF ANCHORS CAST IN CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED		X	ACI 318: 8.1.3, 21.1.8	1908.5, 1909.1
X	4. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS		X	ACI 318: 3.8.6, 8.1.3, 21.1.8	1909.1
X	5. VERIFYING USE OF REQUIRED DESIGN MIX		X	ACI 318: CH.4, 5.2-5.4	1904.2, 1910.2, 1910.3
X	6. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE		X	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1910.10
	7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES		X	ACI 318: 5.9, 5.10	1910.6, 1910.7, 1910.8
	8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		X	ACI 318: 5.11-5.13	1910.9
	9a. INSPECTION OF PRE-STRESSED CONCRETE: APPLICATION OF PRE-STRESSED FORCES		X	ACI 318: 18.20 ACI 318: 18.18.4	
	9b. INSPECTION OF PRE-STRESSED CONCRETE: GROUTING OF BONDED PRE-STRESSING TENDONS IN THE SEISMIC FORCE-RESISTING SYSTEM		X	ACI 318: 18.20 ACI 318: 18.18.4	
	10. ERECTION OF PRECAST CONCRETE MEMBERS		X	ACI 318: CH. 16	
	11. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS		X	ACI 318: 6.2	
X	12. INSPECTION FORM-WORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		X	ACI 318: 6.1.1	

TABLE 1705.2.2 REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL				
APPLIES	VERIFICATION AND INSPECTION	CONT	PERIODIC	REFERENCED STANDARD
	1. MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK:			
X	a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.		X	APPLICABLE ASTM MATERIAL STANDARDS
X	b. MANUFACTURER'S CERTIFIED TEST REPORTS.		X	
X	2. INSPECTION OF WELDING:			
	a. COLD-FORMED STEEL DECK:			
	1) FLOOR AND ROOF DECK WELDS.		X	AWS D1.3
	b. REINFORCING STEEL:			
	1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A706.		X	
	2) REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.		X	AWS D1.4 ACI 318: SECTION 3.5.2
	3) SHEAR REINFORCEMENT.		X	
	4) OTHER REINFORCING STEEL.		X	

TABLE 1705.6 REQUIRED VERIFICATION AND INSPECTION OF SOILS			
APPLIES	VERIFICATION AND INSPECTION TASK	CONT	PERIODIC
X	1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.		X
X	2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.		X
X	3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.		X
X	4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	
X	5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.		X

PERMIT ISSUE



REV	DATE	DESCRIPTION
0	06/25/20	ISSUE FOR PERMIT
A	2/6/20	ISSUED FOR BID



DRAWN BY: BMS
DESIGNED BY: MAD
QUALITY CHECK: MAD
DATE: 2/6/2020
JOB NO: M19-064
FIELDBOOK:

NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
ASHLAND AVE.
FOLCROFT, PA 19032

SPECIAL INSPECTIONS



PERMIT ISSUE

FOOTING (F) SCHEDULE				
MARK	DIMENSION			REINFORCEMENT
	WIDTH (W)	LENGTH (L)	DEPTH (D)	
F1	7'-0"	7'-0"	12"	(7) #5 BARS EA. WAY, TOP & BOT.
F2	4'-0"	4'-0"	12"	(4) #5 BARS EA. WAY, TOP & BOT.

TDH 11002

GENERAL NOTES

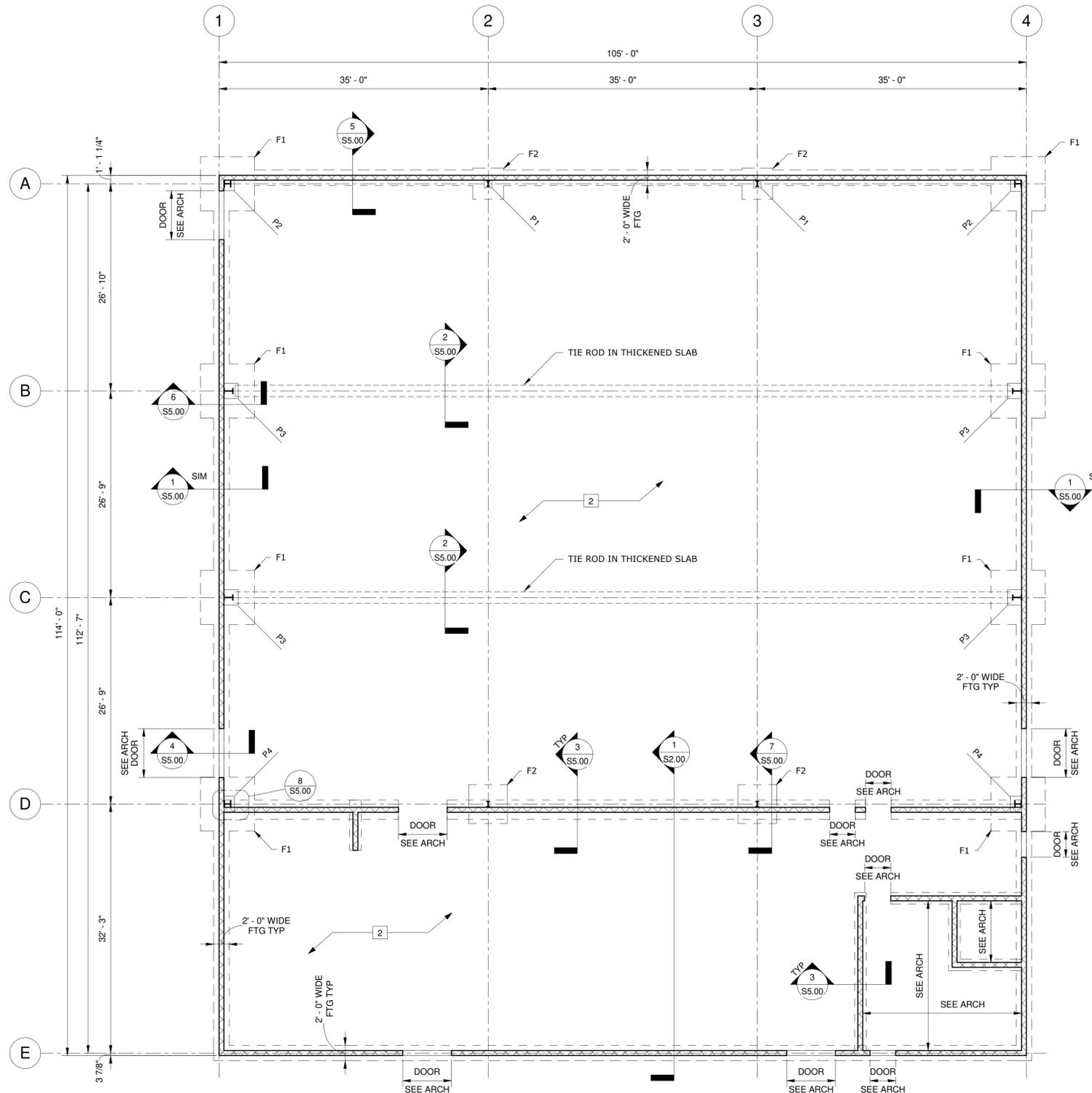
- ALL DIMENSIONS HAVE BEEN FURNISHED BY THE ARCHITECT. IF THERE ARE ANY DISCREPANCIES CONTACT ENGINEER THROUGH ARCHITECT.
- FOR GENERAL STRUCTURAL NOTES SEE S0.00 & S0.01.
- FOR TYPICAL FOUNDATION DETAILS SEE S7.00
- CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS.
- CONTRACTOR TO CHECK WITH FINAL PRE-ENGINEERED METAL BUILDING DRAWINGS AND ANCHOR PATTERN LAYOUT PRIOR TO CONSTRUCTION AND FOUNDATION POUR. ANY REQUIRED CHANGES ARE THE RESPONSIBILITY OF THE CONTRACTOR.
- CONCRETE PIERS ARE TO BE MINIMUM 24" SQUARE. THESE CAN BE POURED AGAINST WALL STRUCTURES AS LONG AS THEY ARE NOT SMALLER THAN 24" SQUARE, TYPICAL.
- FOOTINGS SHALL BE CENTERED ON THE STEEL BUILDING COLUMNS UNLESS NOTED OTHERWISE

FLAG NOTES

- CONCRETE CONTROL JOINT PER GENERAL NOTES
- 4" CONCRETE SLAB WITH WWF 6X6 - W2.1X2.1 CENTERED IN SLAB OVER 6" GRAVEL AND 10 MIL VAPOR BARRIER

FOUNDATION PLAN NOTES

- TOP OF SLAB ON GRADE ELEVATION = 100'-0". TOP OF SLAB ON GRADE REFERENCE ELEVATION = 0'-0" UNLESS OTHERWISE NOTED
- SLAB ON GRADE CONSTRUCTION CONSISTS OF 4" CONCRETE SLAB ON GRADE REINFORCED WITH 6 X 6 - W2.1X2.1 ON 6" OF COMPACTED AGGREGATE AND 10 MIL MIN VAPOR BARRIER. FOR ADDITIONAL INFORMATION SEE TYPICAL DETAILS.
- BOTTOM OF NEW FOOTING ELEVATION INDICATED THUS () IN PLAN
- TOP OF NEW PIER ELEVATIONS SHALL BE FLUSH WITH TOP OF SLAB ON GRADE ELEVATION
- FOR LOCATION AND SIZE OF ALL OPENINGS, CONCRETE PADS, SLAB DEPRESSIONS AND PITS, SEE ARCHITECTURAL DRAWINGS. REFER TO TYPICAL DETAIL DRAWINGS, FOR ADDITIONAL INFORMATION
- THE GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL OBTAIN CERTIFIED MANUFACTURER'S DRAWINGS FOR ALL EQUIPMENT RECESSES AND DIMENSIONS SHALL BE VERIFIED PRIOR TO THE COMMENCEMENT OF WORK.



1 STRUCTURAL FOUNDATION PLAN
1/8" = 1'-0"



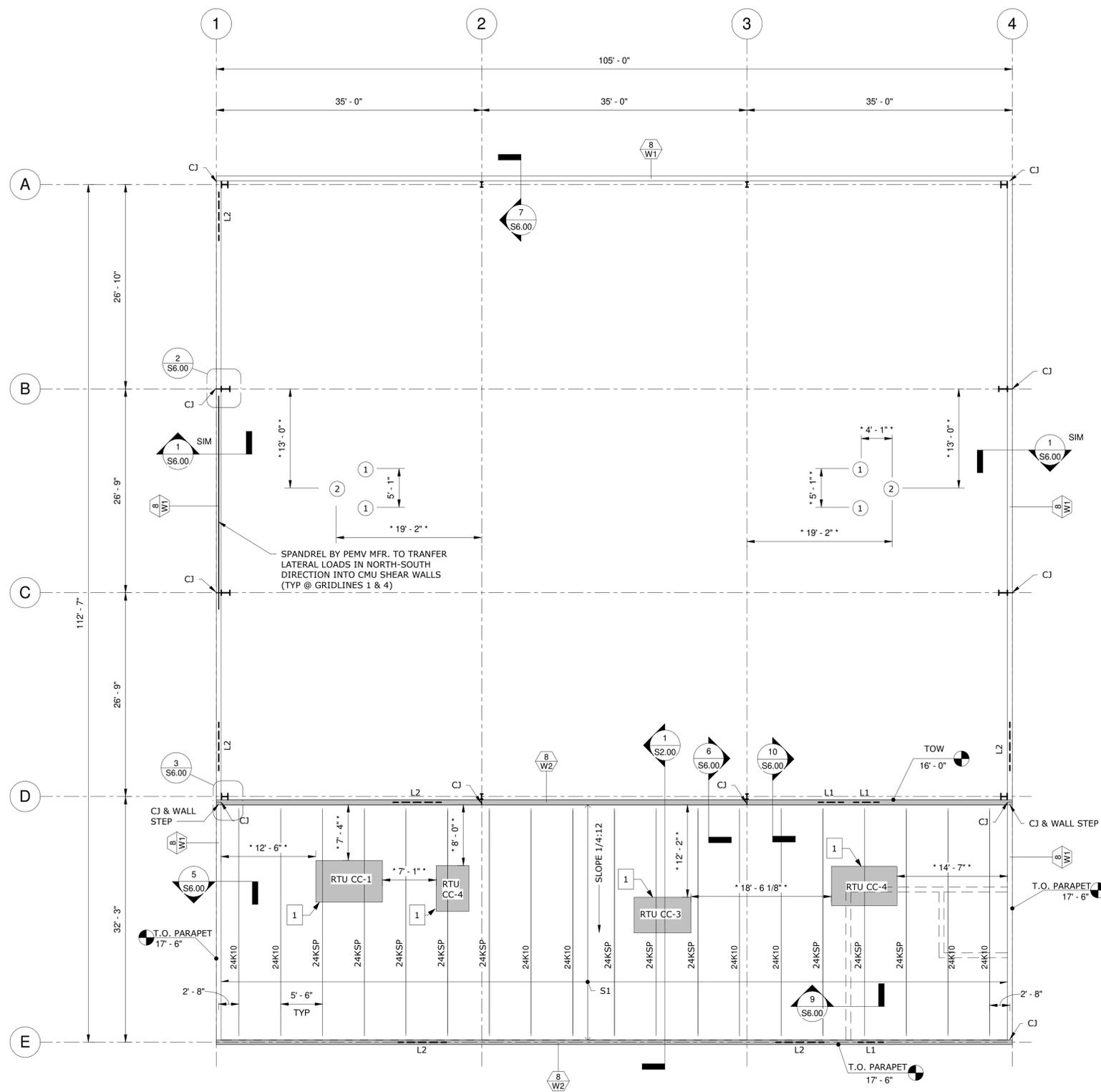
REV	DATE	DESCRIPTION
0	06/25/20	ISSUED FOR PERMIT
A	2/6/20	ISSUED FOR BID



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DESIGNED BY:	MAD
QUALITY CHECK:	MAD
DATE:	2/6/2020
JOB NO:	M19-064
FIELDBOOK:	

NEW MUNICIPAL BUILDINGS - COMMUNITY CENTER
 ASHLAND AVE.
 FOLCROFT, PA 19032

FOUNDATION PLAN



1 FRAMING PLAN
1/8" = 1'-0"

MECHANICAL EQUIPMENT WEIGHTS

VERIFY ALL SIZES, WEIGHTS AND LOCATIONS OF MECHANICAL EQUIPMENT WITH THE MECHANICAL ENGINEER AND ARCHITECT

MARK	EQUIPMENT WEIGHT	REMARKS
RTU CC-1	1700 LBS	MAX WEIGHT
RTU CC-2	1700 LBS	MAX WEIGHT
RTU CC-3	1400 LBS	MAX WEIGHT
RTU CC-4	900 LBS	MAX WEIGHT

CMU WALL REINFORCING SCHEDULE

MARK	VERTICAL	HORIZONTAL
W1	#5 BARS @ 48" O.C.	(2) #4 BARS IN FULLY GROUTED BOND BEAM AT ROOF LEVEL & TOP OF WALL. STANDARD WEIGHT (NO. 9) LADDER OR WIRE JOINT REINFORCEMENT @ 16" O.C.
W2	#5 BARS @ 24" O.C.	

- NOTES:**
- ALL WALLS 8" NOMINAL CMU BLOCK.
 - FULLY GROUT WALLS BELOW GRADE.
 - REINFORCE AT CORNERS AND OPENINGS PER TYPICAL DETAILS ON SHEET S1.4.

GENERAL NOTES

- ALL DIMENSIONS HAVE BEEN FURNISHED BY THE ARCHITECT. IF THERE ARE ANY DISCREPANCIES CONTACT ENGINEER THROUGH ARCHITECT.
- FOR GENERAL STRUCTURAL NOTES SEE S0.00 & S0.01.
- FOR DETAILS SEE S6.00 & S8.00.
- CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS.
- OPEN WEB STEEL JOIST SIZES HAVE BEEN SELECTED BASED ON ROOF DEAD LOADS, MINIMUM ROOF SNOW LOAD, AND SNOW DRIFTS PER TYPICAL DETAIL.
 - JOIST MANUFACTURER CONTRACTOR TO PROVIDE JOIST BRIDGING AS REQUIRED BY SJU SPECIFICATIONS
 - JOISTS SHALL BE DESIGNED FOR NET UPLIFT PER SCHEDULE ON S0.02

FLAG NOTES

- PROVIDE ANGLE PERIMETER FRAMING BELOW RTU CURBS AND OPENING PERIMETERS PER DETAIL 1 & 2 ON SHEET S8.00. CONTRACTOR TO VERIFY SIZE, LOCATION AND PLACEMENT WITH MECH./ARCH. DRAWINGS

LEGEND

- CMU WALL NOMINAL THICKNESS
- WALL REINFORCEMENT PER SCHEDULE
- CMU LINTEL PER SCHEDULE
- BASKETBALL HOOP (EZ-FOLD TB-25 OR EQUAL) CEILING SUSPENDED, REAR-FOLDING, REAR BRACED, TO BE HUNG FROM PEMB STRUCTURE. CONTRACTOR TO VERIFY LOAD DISTRIBUTION AND LOCATION WITH ACTUAL EQUIPMENT INSTALLED. ESTIMATED LOADS AS FOLLOWS

	1	2
PLAYING POSITION	1012 LBS	506 LBS
FOLDING POSITION	445 LBS	890 LBS
- CMU BEARING WALL FOR PEMB
- CONTROL JOINT
- **** INDICATES DIMENSIONS PROPOSED DIMENSION. DIMENSION TO BE VERIFIED BY CONTRACTOR/MANUFACTURER

ROOF FRAMING PLAN NOTES

- TOP OF STEEL ELEVATION AT LOW ROOF = VARIES. ELEVATIONS INDICATED THUS (+/-) ARE RELATIVE TO TOP OF STEEL ELEVATION XXX'-XX"
- DBE INDICATES DECK BEARING ELEVATION. DECK BEARING ELEVATIONS ARE NOTED IN PLAN AND ARE RELATIVE TO SLAB ON GRADE ELEVATION 0'-0"
- S1 INDICATES SPAN OF 1 1/2" 20 GAGE GALVANIZED METAL ROOF DECK. FOR SPAN OF DECK, SEE PLAN. SEE GENERAL NOTES FOR DECK ATTACHMENT.
- THE GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL OBTAIN CERTIFIED MANUFACTURER'S DRAWINGS FOR ALL EQUIPMENT SHOWN. THE GC/CM SHALL COORDINATE DIMENSIONS INDICATED THUS (*) WITH MECHANICAL CONTRACTOR FOR EQUIPMENT PURCHASED, PRIOR TO STEEL FABRICATION.
- THE GENERAL CONTRACTOR/CONSTRUCTION MANAGER SHALL COORDINATE SIZE AND LOCATION OF ALL OPENINGS SHOWN ON THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS AND TYPICAL DETAILS.
- SEE TYPICAL DETAIL DRAWINGS FOR TYPICAL DETAILS NOT REFERENCED IN PLAN.
- FOR ADDITIONAL INFORMATION, SEE GENERAL NOTES

PERMIT ISSUE

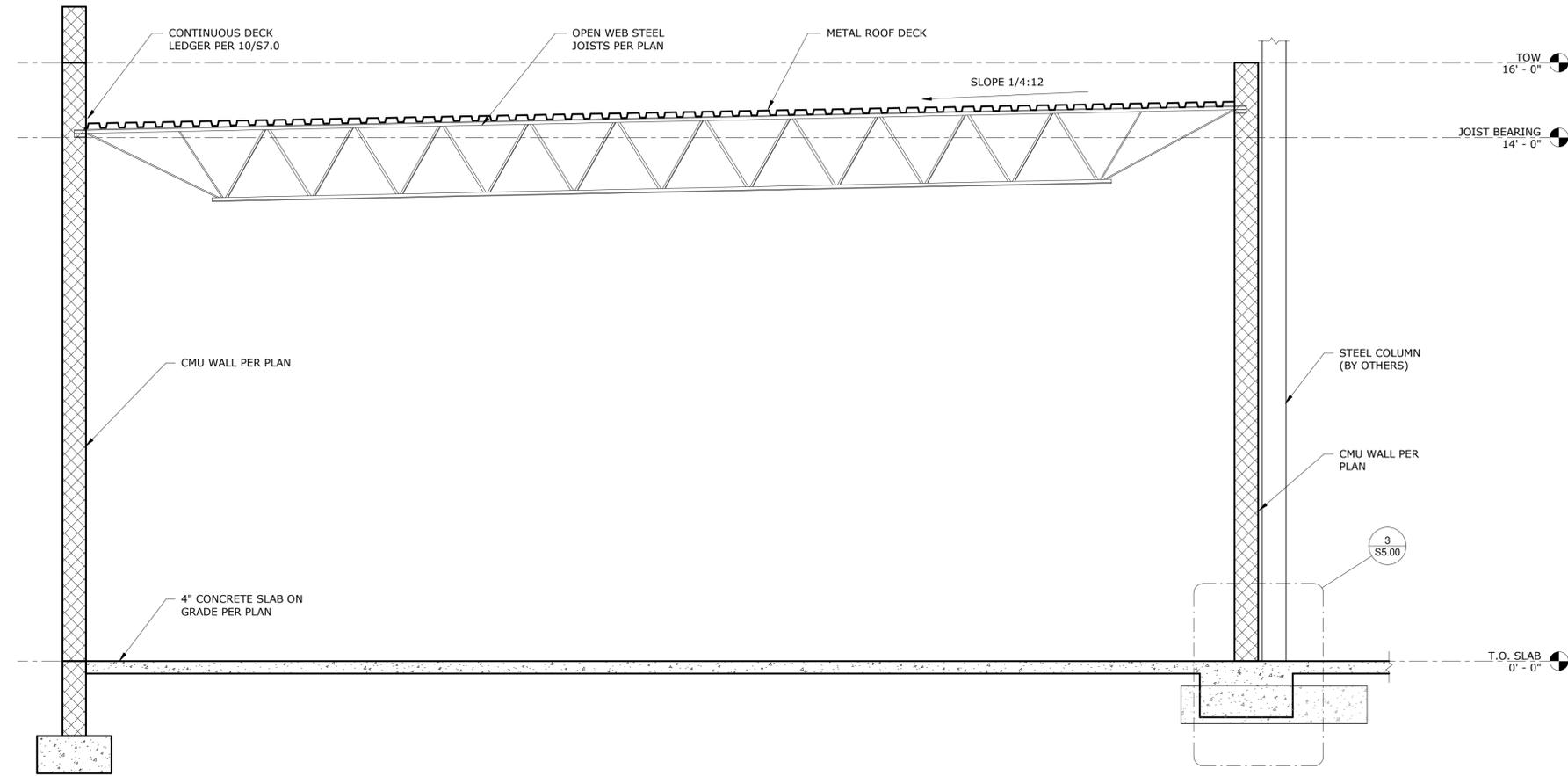


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 FOLCROFT, PA 19032
ROOF FRAMING PLAN



1 FRONT BUILDING SECTION
1/2" = 1'-0"

PERMIT ISSUE



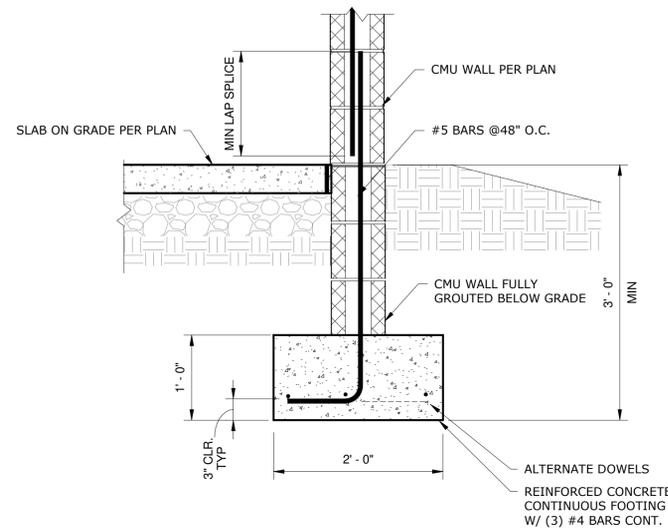
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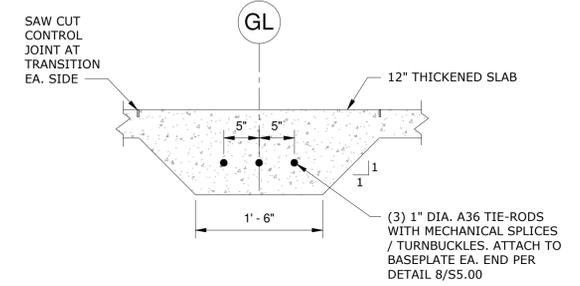
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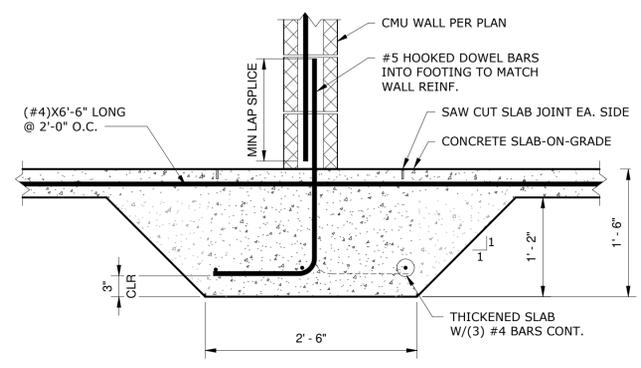
BUILDING SECTIONS



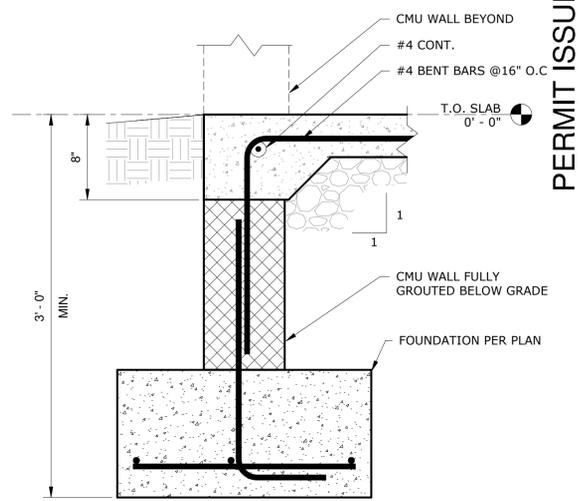
1 EXTERIOR WALL FOOTING
1" = 1'-0"



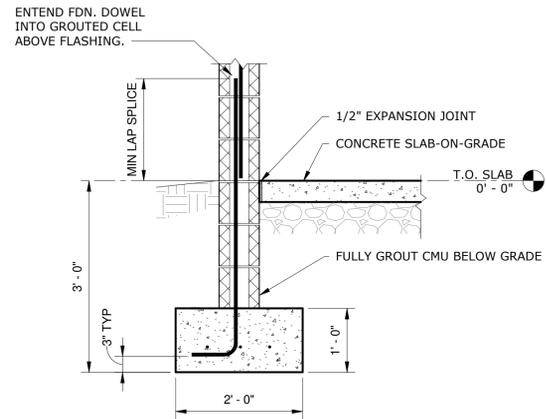
2 TIE ROD DETAIL
1" = 1'-0"



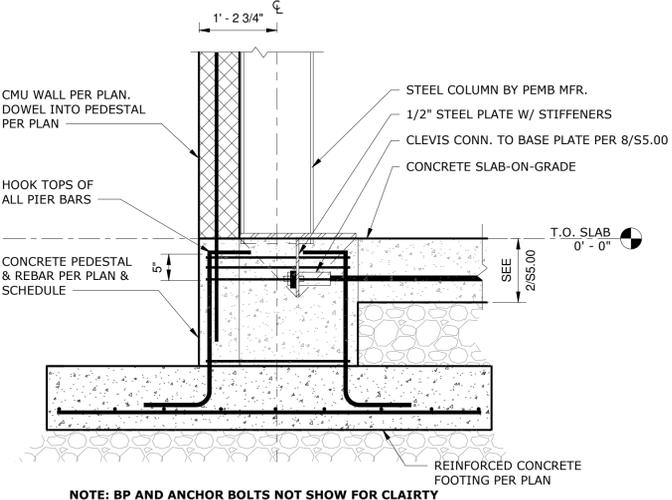
3 INTERIOR WALL FOOTING
1" = 1'-0"



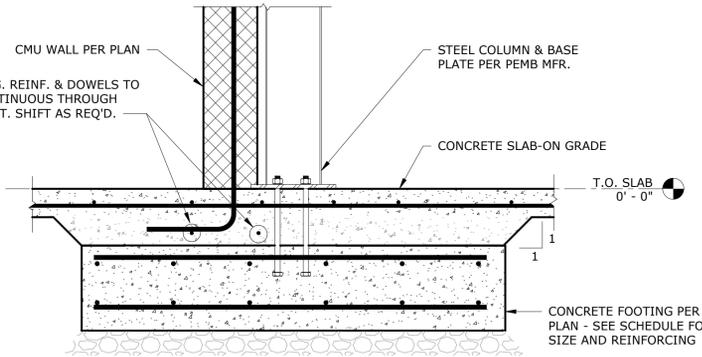
4 WALL AT OPENING
1 1/2" = 1'-0"



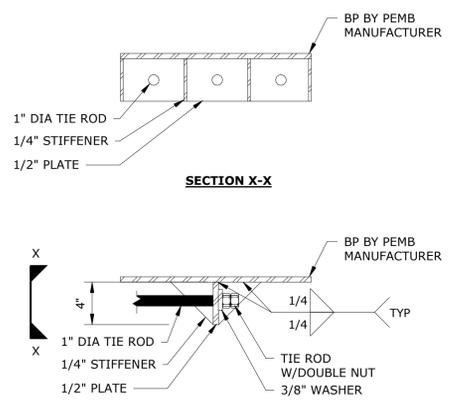
5 CMU WALL FOOTING (GRID A)
3/4" = 1'-0"



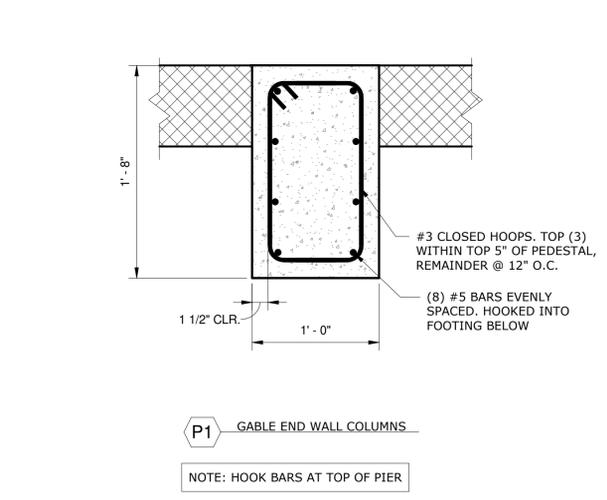
6 SECTION AT PEMB COLUMN
3/4" = 1'-0"



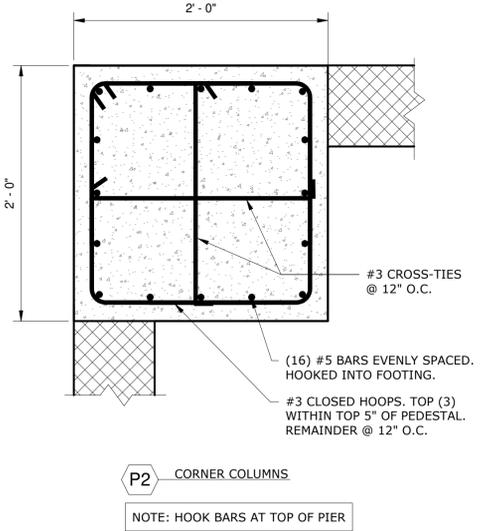
7 INTERIOR COLUMN FOOTING
1" = 1'-0"



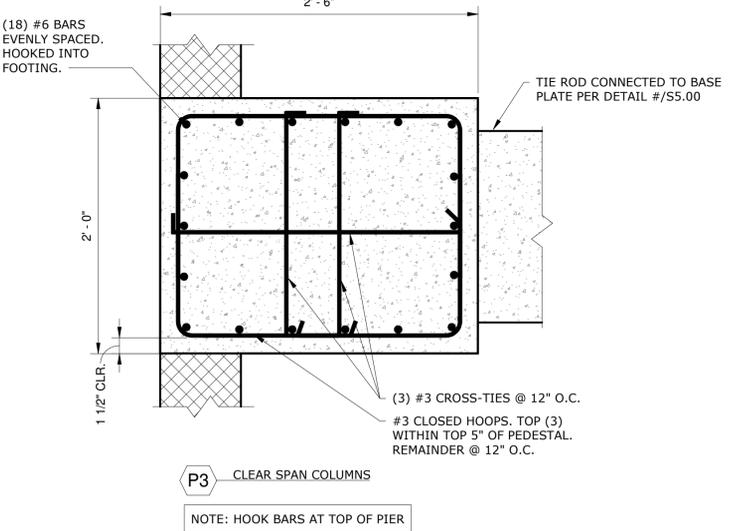
9 TIE ROD CONNECTION
DO NOT SCALE



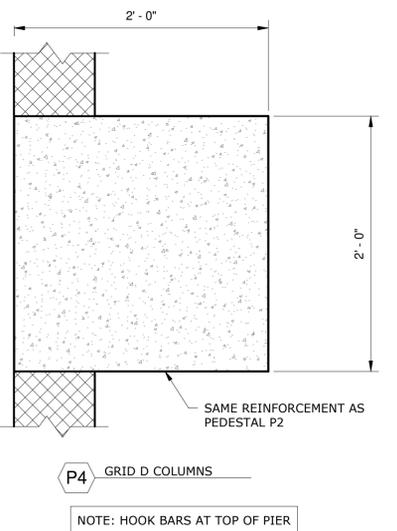
10 PEDESTAL DETAILS
1 1/2" = 1'-0"



P2 CORNER COLUMNS
NOTE: HOOK BARS AT TOP OF PIER



P3 CLEAR SPAN COLUMNS
NOTE: HOOK BARS AT TOP OF PIER



P4 GRID D COLUMNS
NOTE: HOOK BARS AT TOP OF PIER



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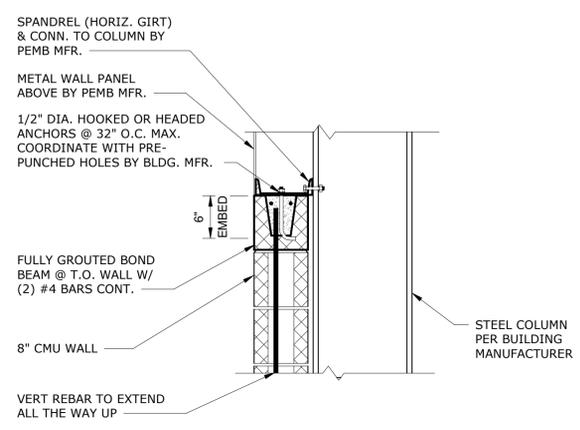
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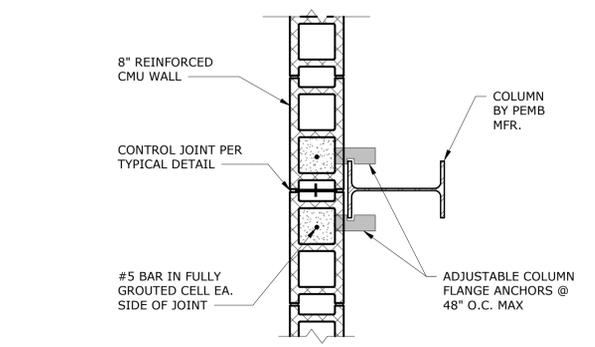
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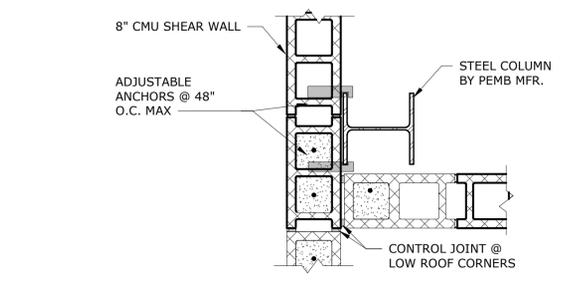
FRAMING DETAILS



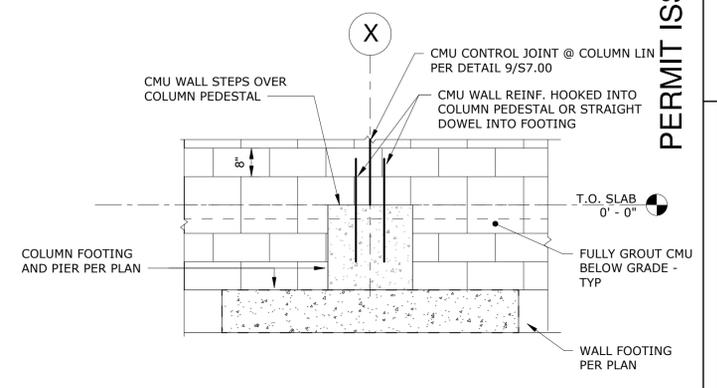
1 TOP OF CMU WALL AT GIRT
1" = 1'-0"



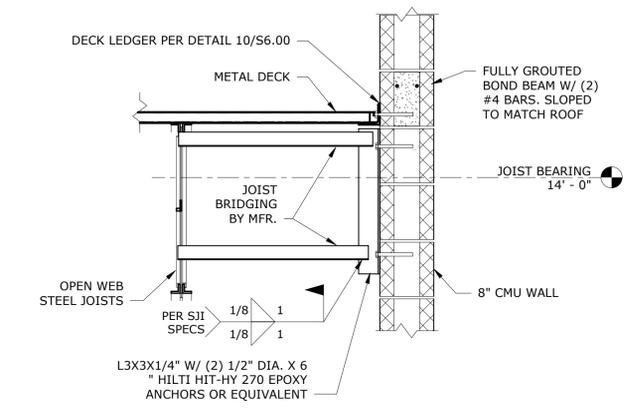
2 CMU WALL AT COLUMN
1" = 1'-0"



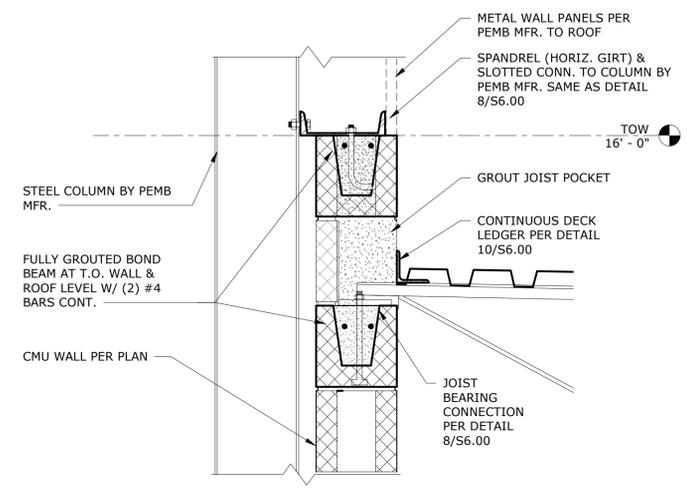
3 CMU WALL AT CORNER
1" = 1'-0"



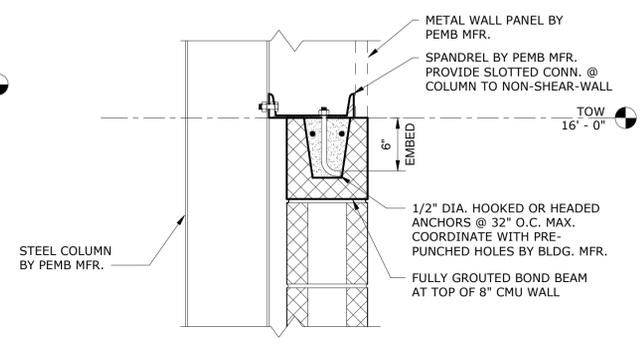
4 CMU WALL AT PEDESTAL
1/2" = 1'-0"



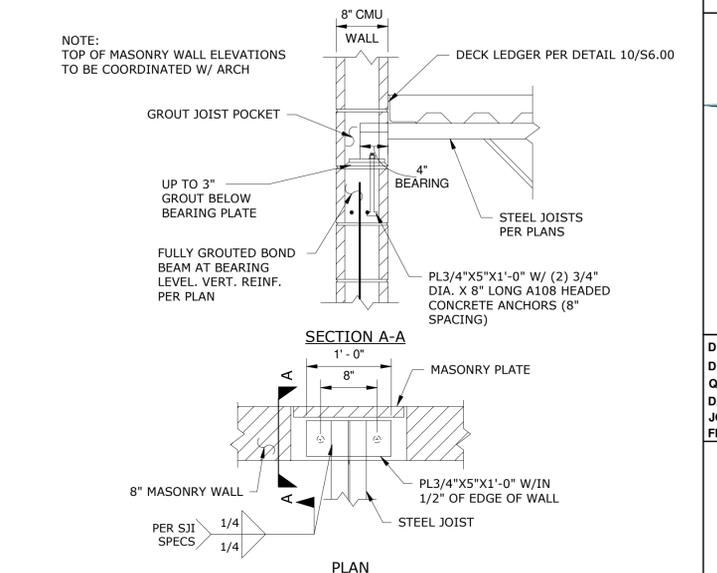
5 JOIST PARALLEL TO CMU WALL
1" = 1'-0"



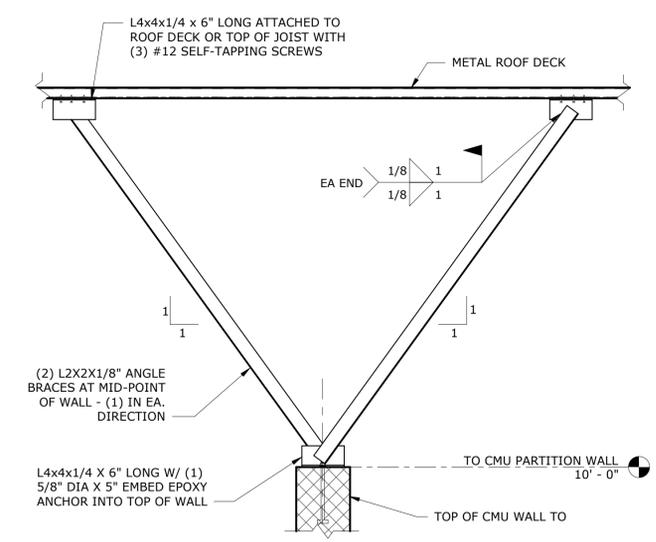
6 TOP OF INTERIOR CMU WALL
1 1/2" = 1'-0"



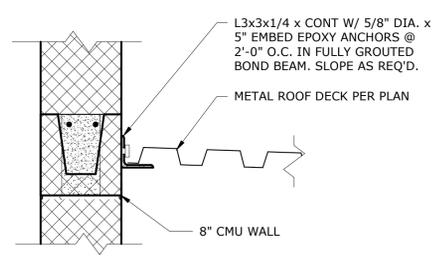
7 TOP OF CMU WALL GRID A
1 1/2" = 1'-0"



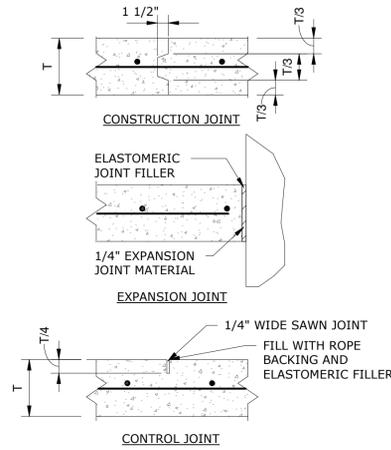
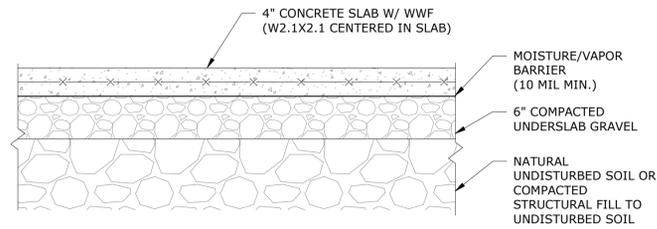
8 JOIST BEARING DETAIL
1" = 1'-0"



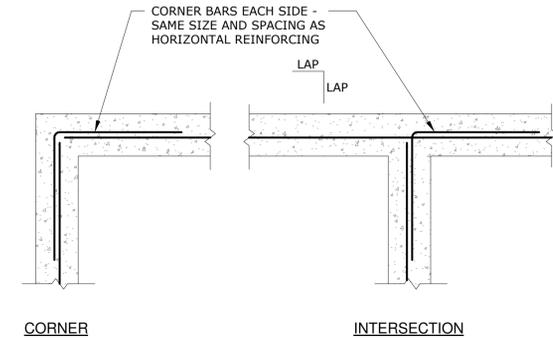
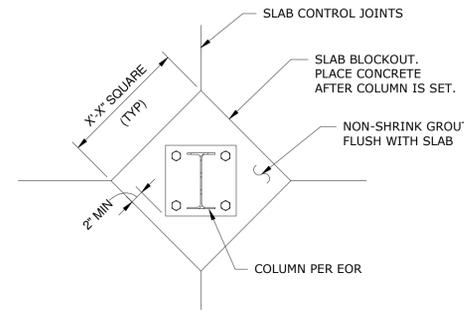
9 NON LOAD BEARING WALL BRACING
1" = 1'-0"



10 TYPICAL DECK LEDGER
1 1/2" = 1'-0"



*SEE NOTES FOR SPACING REQUIREMENTS

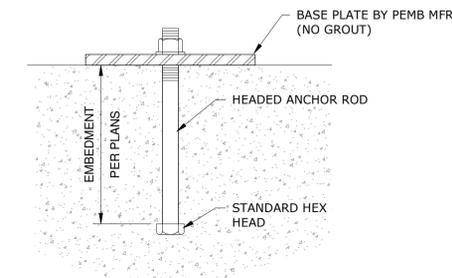
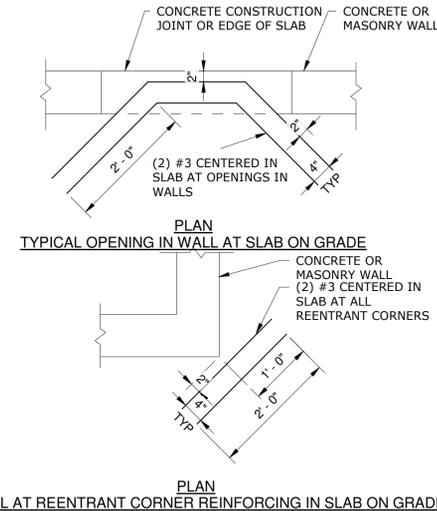
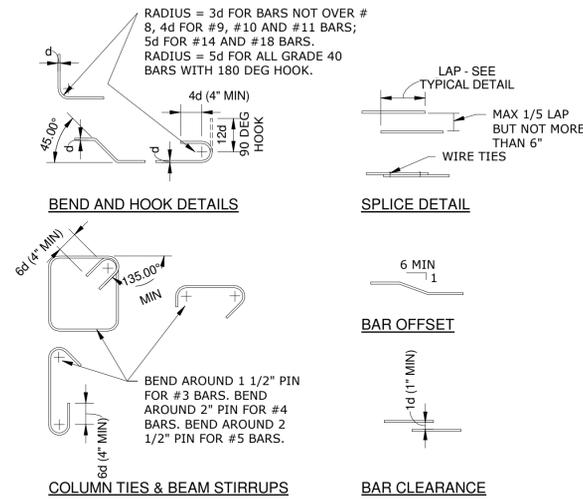
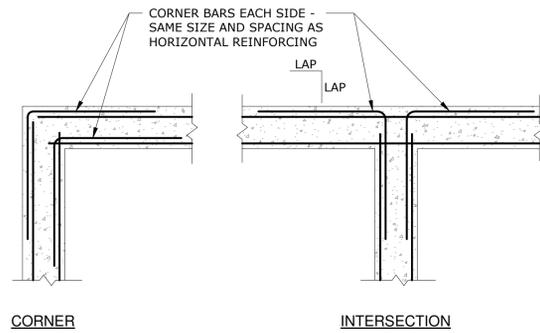


1 TYP. SLAB-ON-GRADE SECTION
DO NOT SCALE

2 TYP. CONCRETE SLAB JOINT DETAILS
DO NOT SCALE

3 TYP. BLOCKOUT DETAIL
DO NOT SCALE

4 TYP. PLAN CORNER REINFORCING
DO NOT SCALE



ANCHOR ROD NOTES

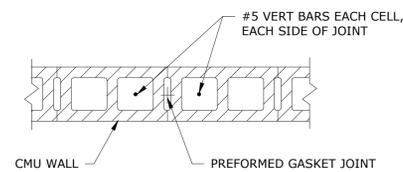
- ANCHOR ROD LOCATIONS AND DIAMETERS ARE PER BUILDING MANUFACTURER
- ALL ANCHOR RODS SHALL BE EITHER HEADED OR DOUBLE NUT WITH 1/4"x2"x2" STEEL WASHER
- MINIMUM EMBEDMENT:
 - 3/4" DIAM. RODS = 12"
 - 1" DIAM. RODS AND PEMB ANCHOR BOLTS = 22"
- ANCHOR RODS SHALL BE ASTM F1554 GR 36 MATERIAL.
- ANCHOR NUTS SHALL BE INSTALLED SNUG TIGHT.

5 TYP. PLAN CORNER REINFORCING
DO NOT SCALE

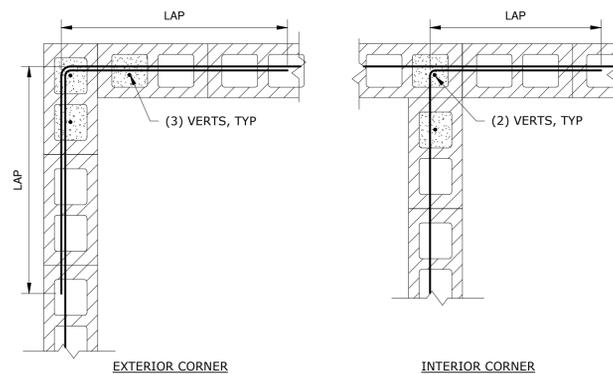
6 TYP. CONC REINF BAR DETAILS
DO NOT SCALE

7 TYP. REENTRANT CORNER REINFORCING IN SLAB-ON-GRADE
DO NOT SCALE

8 TYPICAL HEADED ANCHOR ROD DETAIL
DO NOT SCALE



- NOTES:
- CUT ALL HORIZ REINF @ WALL JOINT EXCEPT @ TOP BOND BEAM.
 - SEE PLANS & ARCH ELEVATIONS FOR LOCATIONS.



- NOTES:
- LAP 48 BAR DIAMETERS OR 2'-8" MIN UNLESS NOTED OTHERWISE.
 - PROVIDE A STANDARD HOOK EACH END ON ANY HORIZONTAL BAR LESS THAN 6'-0" IN LENGTH.

9 TYP. MASONRY CONTROL JOINT DETAIL
DO NOT SCALE

10 TYP. CMU WALL CORNERS
DO NOT SCALE

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TYPICAL FOUNDATION DETAILS



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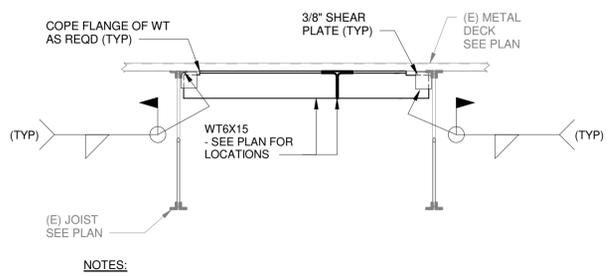
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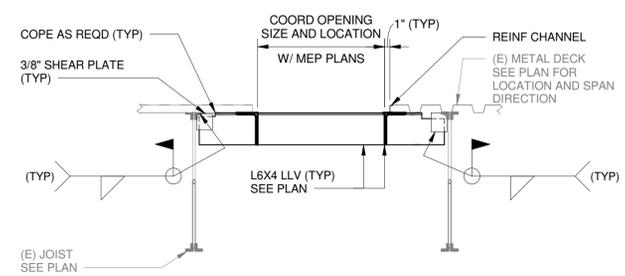
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TYPICAL FRAMING DETAILS



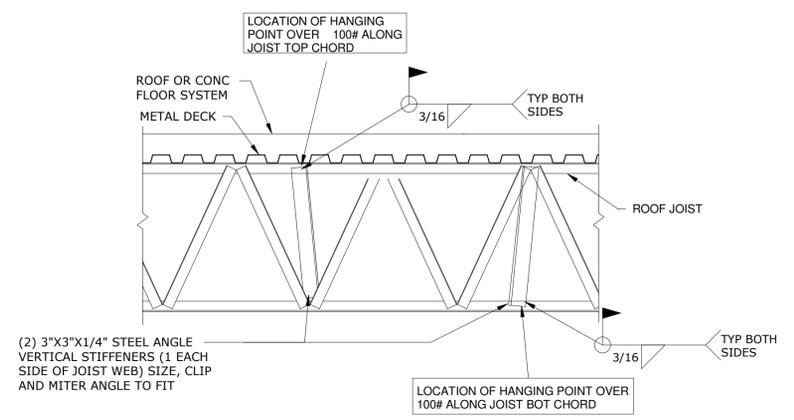
- NOTES:**
1. PROVIDE AT PERIMETER OF ALL RTU'S AND MECHANICAL EQUIPMENT
 2. COORDINATE LOCATIONS WITH ARCH AND MEP DRAWINGS FOR PURCHASED UNITS TO BE INSTALLED

1 TYP MECHANICAL CURB SUPPORT
DO NOT SCALE



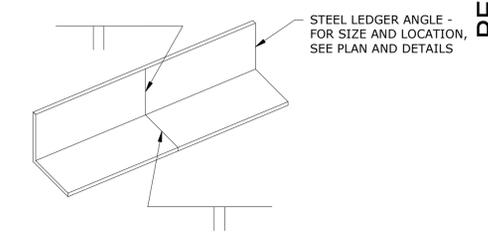
- NOTES:**
1. PROVIDE AT PERIMETER OF ALL ROOF OPENINGS LARGER THAN 12 INCHES SQUARE OR 12 INCHES IN DIAMETER - MAX SPAN BETWEEN JOIST = 6'-0"

2 TYP NEW ROOF OPENING SUPPORT
DO NOT SCALE

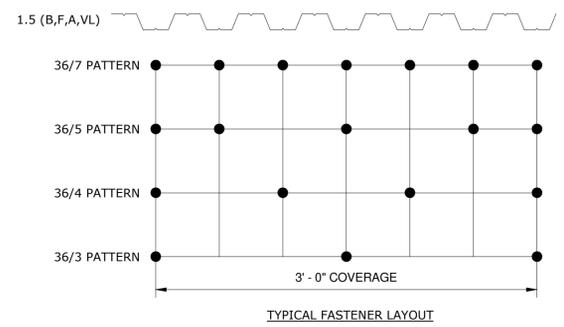


- (2) 3"x3"x1/4" STEEL ANGLE VERTICAL STIFFENERS (1 EACH SIDE OF JOIST WEB) SIZE, CLIP AND MITER ANGLE TO FIT

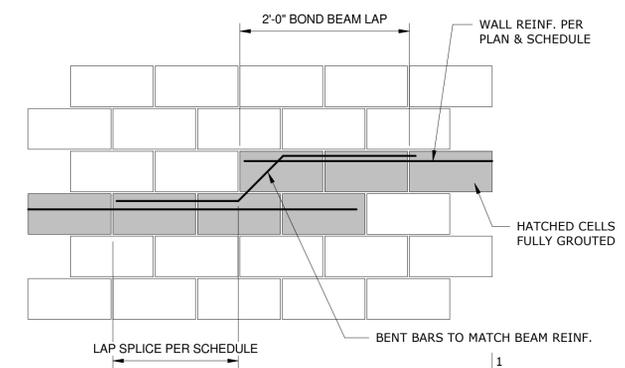
3 TYP JOIST REINFORCEMENT FOR POINT LOADS ON T&B CHORD
DO NOT SCALE



4 TYP SPLICE IN LEDGER ANGLE AT PERIMETER OF ROOF OR FLOOR
DO NOT SCALE

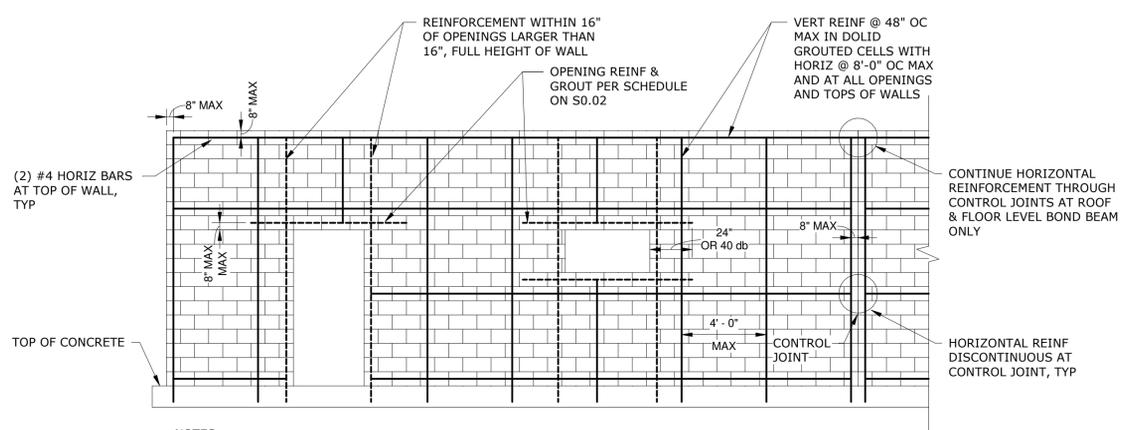


5 TYP METAL DECK FASTENING
DO NOT SCALE



NOTE: SLOPE BOND BEAMS AT ROOF LINE WHERE REQUIRED FOR LEDGER ANGLE ATTACHMENT.

6 TYP BOND BEAM STEP
DO NOT SCALE



- NOTES:**
1. LAP REINFORCING PER SCHEDULE. STAGGER SPLICES IN ADJACENT HORIZ. BARS 2'-0".
 2. PROVIDE DOWEL BARS IN FOUNDATION TO MATCH ALL VERTICAL REINFORCING.
 3. SEE DETAIL 9&10/S7.00 FOR ADDED REINFORCING AT WALL INTERSECTIONS AND CORNERS.

7 TYP CMU WALL DETAIL
DO NOT SCALE